# DESIGN AND TEST EXPERIENCES WITH INSTABILITY

## OF MAJOR AIRFRAME COMPONENTS

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#### **SUMMARY**

**Two** test **incidents** involving **instability** of large scale commercially the first, a fuselage with skin designed to buckle at low stress, and the second, a wing whose surface remains unbuckled to failure. The structure in the region of failure is defined and the failures described and illustrated. Insofar as possible, the stresses in the critical area at the time of failure are reported and compared to strength determined by analysis. Both fuselage and wing surfaces were observed to fail in the mode of a medium range column when adequate support was provided by ribs and frames. Initial failure in the wing example was premature due to a design deficiency in rib strength. A clear illustration is given of the effect of rib stiffness on wing surface stability. Adding stiffness to wing ribs increases the limit surface stability. Adding stiffness to **wing** ribs **increases the** limit **of** surface stability **to the theoretical** flat **panel** value.

#### **INTRODUCTION**

Tests of large, full scale, stiffened shell structures, built by production quality mechanics with production tools, rarely get attention in the technical literature. Individuals in the technical community have limited access to data from these types of tests. Investigators of structural stability have recognized that imperfections exist in all structures and have attempted to evaluate their effects on the limits of stability. The tested structures described in this report are imperfect to the degree which may be expected for commercially built airframes. These imperfections are not specifically defined but one  $m$  ight expect that their effect could be qualitatively evaluated by observing any marked deviation in the behavior of these structures from observing any marked **deviation in** the behavior **of these** structures from **the** theoretical or **experimental behavior of near** perfect specimens.

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## SYMBOLS

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o" Cc  $\sigma_{\rm CT}$ local crushing stress of **effective** section, ksi compressive stress **at** local buckling of skin, ksi

### A FUSELAGE INSTABILITY PROBLEM

During the static test program of the KC-135 jet tanker, one instability type failure occurred in the fuselage monocoque structure. This failure, in the lower aft body under the stabilizer, is shown in Figure 1 at the instant of failure. The critical section was loaded **Figure** 1 **at** the **instant** of failure. The **critical** section was loaded by a combination of bending, vertical shear and **a** small **amount** of torsion.

The structure in the vicinity of the failure may be described as a single cell tapered shell, any section of which is made up of an upper radius and a lower radius separated by a flat segment at the sides. Longitudinal stiffening was provided by hat section stringers which were spaced at about seven inches. Stringers had an area of .23 square inches and a  $\rho = .475$  in. The monocoque skin was .040 - 7075-T6 clad curved to a radius of 35 inches. Frames were connected to the upstanding legs of the hat section stringers and did not interrupt the stringer continuity. The frame forward of the failure area was a partial bulkcontinuity. The frame forward of  $\frac{1}{2}$  area  $\frac{7075-76}{2}$ , section  $2.44$  $h$ ead. The frame aft was a formed  $n = 2$  space. inches deep. The frame spacing was **24** inches.

Loads were applied in small increments up to failure and instru-mentation read at each increment. Compression wrinkles in the lower surface skin were noticed at about 50% of the failure load. At 95% of the failure load, these buckles were very sharp. All load systems of the failure load, these buckles of the final load however, the  $h$ ad stabilized after the application of the final load, he recorded, fuselage collapsed before data could be recorded.

Failure was initiated in the skin-stringer panel midway between<br>the frames described. An inside view of the fuselage after failure is shown in Figure 2. The frames appeared to be in good condition is shown in Figure **2.** The frames **appeared** to be in good condition **after** failure with no **indication** of lateral permanent set.

A group of strain gages were located on the critical stiffener<br>within inches of the point of failure initiation. An analysis of these gage readings indicates that at the time of failure, the average stress in the effective bending structure was  $-43,000$  psi. The stress computed  $\frac{1}{2}$  in the effective body using the elementary  $\frac{1}{2}$  and  $\frac{1}{2}$  =  $\frac{1}{2}$  =  $\frac{1}{2}$  =  $\frac{1}{2}$ bending section is also -43,000 psi.

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#### Allowable Stress Computation

At the time of this design, allowable compressive stresses were computed by the Johnson Parabolic formula

$$
\sigma_{edge} = \sigma_{cc} = \frac{\sigma_{cc}^2 \left(\frac{1}{\rho \sqrt{c}}\right)}{4 \pi^2 E}
$$

A limited amount of test data was used to substantiate this analysis. A fixity coefficient  $c = 1.0$  was assumed and  $\ell$  was taken as the frame spacing. The strengthening effect of the body curvature was neglected. The effective section was calculated using a width of the frame in the frame of the frame in spacing. **The** strengthening effect of the body curvature was neglected.

$$
b_{e} = 1.7 t \sqrt{\frac{E}{\sigma_{edge}}}
$$

The proportions of the stiffener were such that local crippling could edge **The proportions** of the stiffener were such that local crippling could

$$
\sigma_{\text{CC}} = F_{\text{CV}} = 62,000 \text{ psi}.
$$

Computation of the allowable compressive stress at the point of test failure using this method yielded  $\sigma$ edge = -41,200 psi. When compared to the test conditions at failure, this analysis underestimated the surface strength by less than  $\frac{1}{2}$ ,  $\frac{1}{2}$ ,  $\frac{1}{2}$ ,  $\frac{1}{2}$ ,  $\frac{1}{2}$ ,  $\frac{1}{2}$ compared to the test conditions **at** failure, this analysis underestimated

An analysis of the critical section using the techniques described  $\frac{1}{2}$  and  $\frac{1}{2}$  the flat panel portion of another  $\frac{1}{2}$  reference  $\frac{1}{2}$  $\mathcal{L}$   $\mathcal{L}$  and  $\mathcal{L}$  and  $\mathcal{L}$  are centred. In reference 1 was recently completed.

$$
N_{p} = \frac{c \pi^{2} Q}{l^{2}} + \gamma \frac{2}{R} \sqrt{E_{1} D_{2}}
$$

and again assuming the fixity coefficient  $c = 1$ , the edge stress at curvature in this analysis would reise the edge three to the goe panel **buckling** was calculated as \_edge = -43,000 psi. Consideration of

On the strength of these analyses the consideration that the structure behaves as a flat simply supported column appears to be reasonable for structures of these proportions loaded in compression only. The skin, being buckled at a low stress, looses its ability to stabilize the stiffeners by shell action and permits a column failure mode to be predominate. Where the skin between stiffeners is buckled by high shear loads combined with compression, a more sophisticated analysis must be undertaken to account for the interaction of the shear in the skin on the compressive streaght of the superson of the analysis **must** be undertaken to account for the interaction of the shear

### A WING **INSTABILITY** PROBLEM

A wing **instability** failure was **encountered** during the static **des**truction test program of the B-52A airplane. Failure was initiated by **instability** of **the inspar** wing structure **and** resulted in complete **collapse** of the upper surface of the left hand **wing.** Failure occurred approximately 25 feet outboard from **the** side of the body (one-third span) as shown in Figure 3. The critical section of the wing was loaded by a combination of bending, shear and **torsion.**

The **primary** structure is a single-cell, two-spar, box beam of **con**stant **width** and tapered **depth.** Upper and lower surfaces are cambered in the chord direction. **In the** failure area, **the** wing box is approximately ten feet **wide** with an average depth of 33 inches. Upper surface spanwise stiffening is **provided** by extruded "J" section stiffeners spaced at approximately 8.5 inches and supported by chordwise ribs spaced at 30 inches. Ribs are of a stiffened web construction and constitute a beam member between front **and** rear spars. Spanwise stringers are continuous **with their** outstanding flanges attached **to** the chords of the supporting ribs. All **primary** structure is fabricated from 7075-T6 aluminum material **with the** exception of rib **webs which** are 2024-T3 aluminum material. **In** the region of failure, the upper surface skinstringer combination has an area of .5 square inches **per** inch of **width** and a radius of gyration of 1.0 inches.

Loads **were applied** in small increments **and instrumentation data** obtained **at** each **load level.** At 88\_ of failure **load,** the **deflection** indicators used to measure rib crushing began to show a nonlinear load**deflection** relationship. **Incremental** loading **was** continued until a sudden and complete failure occurred. At the final load application, the structure collapsed before instrumentation data could be obtained.

Failure **was initiated** by crushing of the ribs with **the** simultaneous collapse **of** the upper compression material in the **aft portion** of the box. **Internal** damage in the area of **primary** failure is shown in Figure 4. From an analysis of the recorded data **taken** from strain gages located in **the** failure area, **the** average stress in the upper surface material was -45,000 psi at failure. **The** stresses calculated in the failure area by the elementary flexure formula average approximately -50,000 psi. It seems likely that the softness of the supporting ribs in the test structure caused some of the upper surface load to shift to the more lightly loaded structure in the forward **part** of the box, **there**by explaining the discrepancy between **the** measured **test** stresses **and** the calculated stresses in **the** aft portion of the box. Unfortunately, no strain gages were located on the forward part of the **wing to** substantiate **this** argument.

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**Ribs** are generally used as stiffening members in flexible wings to prevent the collapse of the surface material. Rib crushing loads are proportional to the curvature of the wing. Rib loads for the critical **wing** rib were computed according to the formula:

$$
P = \frac{M}{h_{e}h_{x}r} \qquad + \frac{M^{2}l}{h_{e}h_{x}EI_{x}}
$$

which includes **the** effect of initial surface curvature and primary wing bending curvature. **The** computed rib loads were 380 ibs per inch of surface width under **the** failure conditions. A subsequent test of the rib under crushing loads indicated an ultimate strength of 600 Ibs per inch of surface width. **Its** spring rate was found **to** be 4,SO0 ibs per inch deflection per inch of surface width. Since the rib was the primary cause of failure, the foregoing formula obviously did not account for **the** total rib crushing load experienced in **the** test.

**In** attempting **to discover** the reason for this **discrepancy,** it was **observed that the rib adjacent** to **the** critical **rib had a considerably** higher **extensional** stiffness since it formed **a** fuel **tank** bulkhead. As **a consequence,** the **upper** surface **curvature** was **higher than expected at the** critical **rib and** lower at **the adjacent stiffer rib. The opposite effect occurred at** the lower surface. A subsequent **analysis, account**ing for **the** differences in **rib** stiffness **and the** shearing **deformations** in the **ribs caused by the unbalanced crushing** loads, showed **that the upper** surface **crushing** load **at the critical** rib was **actually** 610 lbs **per** inch **of** surface width for the failure **condition. This** is in **close agreement** *with* **the tested ultimate** strength **of this rib and emphasizes the** importance **of** including **all** secondary **effects** in **rib** stress **analysis.**

#### Allowable **Stress** Computation

**Curves** of allowable wing surface stress versus  $\frac{1}{\rho}$  <del>*i*</del> were established on the basis of **a large amount** of experimental data **accumulated** for this **airplane as well as previous Boeing airplanes. Since the** surface skins **were** required to remain **unbuckled at ultimate** load, these **design** curves were modified **in** the **short** column range **according** to the formula

$$
\sigma_{\text{CT}} = \kappa_{\eta} \varepsilon \left(\frac{1}{b}\right)^2
$$

**A** buckling **coefficient of 5.0 was** used **to** account **for the** skin being continuous **over the** stringers, curvature **of the** surface, **and** torsional **restraint provided the** stringers **by the ribs.**

**The** allowable surface stress in **the failure** area **was** computed as -56,000 psi using **these** procedures and assuming a fixity coefficient c **=** 1.0. An analysis using **the** formula

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Np **=** \_2 with a fixity coefficient  $c = 1.0$  gives an allowable edge stress equal **to** -56,000 **psi.** Premature rib failure **restricted the** surface stress **level to** -45,000 psi. As a result of this failure, the rib was reinforced to an **ultimate** strength of 890 Ibs per inch of surface width. **In a** subsequent test of the **wing** incorporating these rib changes, the upper surface material reached **a** stress **level** of -53,000 psi. A failure in **another** part **of** the **wing** prevented further load **application and**

testing was **discontinued.**

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**In an attempt** to further substantiate **the** strength of the upper surface, **a** separate test program was initiated. Several tests were made on portions of the **upper** surface, curved to the shape of **the** deflected wing **and** laterally supported on springs simulating rib flexibility. Destruction tests of **these** panels established **that** the surface allowable of the wing **as** originally tested was -54,000 psi, **and that, after** improving the rib stiffness, the surface **allowable** increased to -55,500 psi. **Further increases** in rib stiffness produced **no** major improvement in strength. Similar tests on initially straight specimens **(without** wing bending curvature) indicated no improvement in strength over specimens that were initially curved.

#### CONCLUDING REMARKS

In the example **of** fuselage instability, the structure did not fail in a mode typical of cylindrical shells but tended to behave in much the same manner as a column. This might be expected in the case where the skin is designed to **buckle** in compression at low applied loads. The curvature **of** the section seemed to have **only** a minor effect on its strength.

**In** both the wing and fuselage examples the fact that **the** ribs **and** frames were uniformly spaced and offered little resistance to rotation of the **surface** structure seemed to **permit** the surfaces to deform to the characteristic single half wave shape between **supports in** the manner of a simply supported column. This is evidenced by the fact that the fixity coefficient for simple **supports** yields an analysis which closely approximates the actual tested **strength.**

The wing program gives definite indication that there is an optimum amount of rib stiffness required to develop the full surface strength. The radius of curvature of the wing  $(\sim$  2000 inches) had no appreciable effect on the surface strength.

### **REFERENCES**

le **Peterson,** James **P.,** Whitley, **Ralph** 0., **and** Deaton, **Jerry** W.: **Structural Behavior and** Compressive **Strength of Circular** Cylinders with **Longitudinal Stiffening.** NASA **TN** D-1251, 1962.

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Figure 1.- Fuselage instability failure.



Figure 2.- Inside view - fuselage instability failure.

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Figure 3.- Wing upper surface instability failure.



Figure 4,- Rear inside view - wing instability failure.

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