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LINEAR AND NONLINEAR ANALYSIS OF SHELLS OF REVOLUTION WITH
ASYMMETRICAL STIFFNESS PROPERTIES

USER'S MANUAL

SPACE TECHNOLOGY REPORT 68 - 65 TO

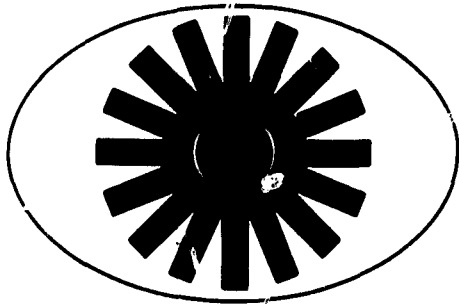
NATIONAL AERONAUTICS AND SPACE ADMINISTRATION

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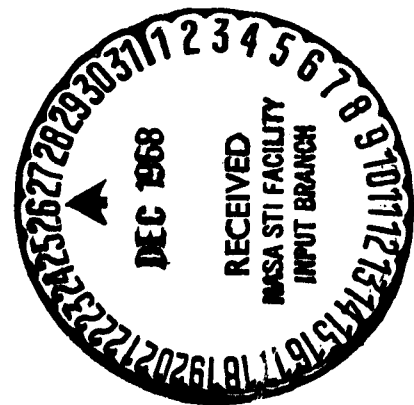
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October 1968

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ABSTRACT

Two computer programs have been developed to perform a linear and nonlinear elastic analysis of shells of revolution under arbitrary loading with asymmetrical stiffness properties in the circumferential direction as well as variable thickness properties in the meridional direction. The method of analysis is the matrix displacement method of structural analysis. Input data define the elastic and geometric properties of the elements, the coordinates and constraints of the nodes, and the direction, magnitude and location of the loads. Output data consist mainly of the displacements of the nodes and the strains and stress resultants at the center of each element. The general purpose of this report is to provide a guideline for the preparation of input data for the computer codes.

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CHAPTER I
INTRODUCTION

I.1 GENERAL

Digital computer programs written in FORTRAN IV have been developed to perform the computations required in the linear and nonlinear elastic analysis of shells of revolution under arbitrary loading with asymmetrical thickness properties in the circumferential direction as well as in the meridional direction^{1,2}. The purpose of this User's Manual is to provide to the engineer readily understood and easily applied information about the computer codes and the preparation of input data.

I.2 BRIEF REVIEW OF THE THEORY

The method of approach is the matrix displacement method of structural analysis.

The shell of revolution is idealized and discretized as an assemblage of curved elements connected together at their nodal circles or nodes (Fig. 1). The element curvature in the meridional direction is described by the slope between the axis of revolution and a tangent to the shell element in the meridional direction and is represented by a second order polynomial function of the meridional distance. The length of the element, measured along the meridian, is obtained by assuming an arc of a circle passing through the nodes with the specified nodal slopes.

The deformations of the element are represented by three linear displacement components in the normal, meridional and circumferential directions (Fig. 2). These displacements are expressed by polynomials in the meridional distance and a Fourier series in the circumferential angle.

The generalized nodal forces are obtained by concentrating the external forces as line loads applied at the nodes. This is accomplished by equating the virtual work performed by these lumped loads moving through the virtual nodal displacements, to the virtual work produced by the external forces undergoing the specified virtual displacements. The components of the external loading are represented by a Fourier series in the circumferential angle. These external load components are assumed to have a stepwise variation in the circumferential direction (Fig. 3). When there exist concentrated ring loads applied at the nodes, these loads are added to those due to the distributed loading to form the total generalized forces.

The strain-displacement relationships used are those given by Novozhilov³ retaining for the nonlinear analysis only the terms suggested by von Karman⁴.

The element stiffness matrix is obtained by applying Castigliano's theorem and observing the coupling of the harmonics. The coupling of the harmonics is due to the fact that the shell element may have variable thickness properties in the circumferential direction.

The thickness of the shell element is assumed to have a stepwise variation and is represented by a Fourier series in the circumferential angle (Fig. 4).

The stiffness matrix for the entire shell is formed by summing up the appropriate stiffness matrices for the various finite elements.

The displacements are obtained by solving the system of linear simultaneous equations using Choleski's method.

To account for the geometric nonlinearities, the total internal energy is separated into a sum of two parts

$$U = U_L + U_{NL}$$

where U_L is the expression for the strain energy based on linear theory and U_{NL} is the contribution due to the nonlinear terms.

The nonlinear terms are regarded as additional generalized forces.

These additional forces are obtained by applying Castigliano's theorem resulting in the following equations of equilibrium

$$[K] \{q\} = \{Q\} - \left\{ \frac{\partial}{\partial q} (U_{NL}) \right\}$$

where $[K]$ is the stiffness of the structure, $\{q\}$ represents the generalized displacements, $\{Q\}$ describes the generalized forces and $\left\{ \frac{\partial}{\partial q} (U_{NL}) \right\}$ expresses the additional generalized forces due to nonlinearities.

The solution of the equations of equilibrium is accomplished

using one of the three following approaches depending on the type of the problem to be solved:

a) In this method the deformation is analyzed by varying the loading in small increments and performing a linear analysis of the deformation due to each load increment. For the first increment the solution is purely linear. For the second increment a linear extrapolation is used and for the third and succeeding increments a quadratic interpolation is employed to generate the deflections to be used in the calculation of the additional forces for the next increment. The process is repeated until the total loading is applied.

b) In this second method the total loading is applied and an iteration is performed. The first approximation of the nonlinear terms is obtained by using a linear solution. The iteration procedure is used until convergence occurs or the number of iterations performed equals the maximum specified number.

c) In this combined method the load is applied by increments. The solution for each increment is obtained by iterating until convergence occurs. The displacements are then extrapolated, in the same fashion as in the method (a) and the nonlinear terms are evaluated for the next increment. The process is repeated until the total loading is reached.

CHAPTER 11
RECOMENDATIONS AND SPECIFICATIONS

11.1 GENERAL

Certain important factors must be taken into account when using this proposed numerical method in order to have a realistic representation of the actual structure. For this matter, it is advisable that the practicing engineer have a reasonable understanding of the structure to be able to discretize the shell in a justifiable manner. In some cases, an inadequate idealization may cause unrealistic results in some areas of the shell, leading to a misunderstanding of the behavior of the structure.

11.2 RECOMENDATIONS

Some recommendations are herein presented to provide an indication of the type of care that the inexperienced user must exercise.

1. Elements should be closely spaced and concentrated in regions where rapid variation in the displacements and stresses may occur. For example, near a clamped boundary where the bending moment may vary rapidly.
2. Elements should be concentrated in regions where there is a rapid change in the material properties.
3. The number of harmonics in the Fourier expansion of the displacement components must be such as to account for all significant modes of deformations. If a shell deforms symmetrically only A harmonics are needed. In the case where it is anticipated that the shell will

deform asymmetrically both A and B harmonics must be used. In most engineering applications satisfactory results may be obtained with only few harmonics.

5. In the solution of the equilibrium equations for the nonlinear analysis use the load increment approach for slightly nonlinear problems and use the combined load increment iteration method to estimate buckling loads.

II.3 SPECIFICATIONS

1. Machine: IBM 7094, IBM 360/65, UNIVAC 1108
2. Language: FORTRAN IV
3. Program capacity (based on machine size):
 - (a) 50 elements (NELEMS)
 - (b) 17 harmonics ($NH = IA + IB$)
 - (c) 29 Simpson's stations for the numerical integrations (NET)
 - (d) 10 angles along the circumference where the displacements, strains and stresses are required (NUNKTH)
 - (e) 10 elements restrained (NEREST)
 - (f) 20 comment cards
 - (g) 50 angles along the circumference where the thickness of the element is specified (NC)
 - (h) 74 angles along the circumference where the external loadings are specified (NDP). The thickness of the element as well as the distributed external loads are assumed to have a stepwise variation in the circumferential direction.

4. Each discrete element may be constituted of any homogeneous and isotropic material.
5. The elements may have constant as well as varying thickness properties in the circumferential direction.
6. Symmetrical as well as asymmetrical loads may be applied to the structure.
7. Loads may be of two types:
 - (a) Uniform constant load between any two angles along the circumference. Concentrated point loads may be considered as pressure loadings distributed over a small area.
 - (b) Concentrated ring loads applied at the nodes.
8. Pressure loadings are described in the shell coordinate system. Concentrated ring loads are described for each harmonic in the cylindrical coordinate system.
9. Displacements are expressed in the cylindrical coordinate system.
10. Restraints:
 - (a) Free - unconstrained in a component direction
 - (b) Fixed - a rigid support
11. Sandwich type shells may also be analyzed.

CHAPTER III

COMPUTER PROGRAMS

III.1 GENERAL

Based on the storage capacity of the IBM 7094, the computer programs are linked into five overlay segments as shown in Fig. 5 and Fig. 6.

The computer codes accept a description of the structure in terms of the coordinates of the nodes (radial and axial), the slopes at the nodes and the elastic and geometric properties (thickness, Young's modulus and Poisson's ratio) of the elements joining them.

For shells of revolution with a geometry composed of a combination of a spherical cap, fillet and a cylinder the programs are capable of generating the shell geometry internally thus reducing considerably the input data.

The flow charts given in Fig. 7 and Fig. 8 indicate the sequence of the principal operations as they are performed in the programs.

III.2 PRINTOUT GENERATED BY THE PROGRAMS

1. General information. This information is read in as comment cards and output as general information giving a description of the problem being analyzed or any other comment.

2. Nomenclature. This printout defines all symbols used.

3. Elastic and geometric properties. The coordinates (axial and radial) and slopes are listed for each node. The elastic constants (Young's modulus and Poisson's ratio) are listed for each element. In this section are also reported the number of harmonics and the number of Simpson's stations used in the analysis.

4. Table of thicknesses. The thickness variation along the circumference is described for each element.

5. Boundary conditions. This describes the way the structure is restrained.

6. Applied loads on the structure. The pressure components in the shell coordinate system (meridional, normal and tangential) are listed for each loaded element. The concentrated ring load components in the cylindrical coordinate system (tangential, radial and angular) are listed for each loaded element and for each harmonic.

7. Displacement components of nodes. The displacement components in the cylindrical coordinate system (axial, tangential, radial and angular) are listed for each node and for each specified angle along the circumference.

8. Strains and changes in curvature. The strains and changes in curvature (ϵ_s , ϵ_θ , $\epsilon_{s\theta}$, K_s , K_θ and $K_{s\theta}$) are listed at the center of each element and for each specified angle along the circumference.

9. Stress resultants. The stress resultants (N_s , N_θ , $N_{s\theta}$, M_s , M_θ and $M_{s\theta}$) are listed at the center of each element and for each specified angle along the circumference.

10. Maximum computed displacement components and stress resultants.

A table summarizing each maximum computed displacement and stress resultant component is presented. This table specifies: (a) the node and the angle along the circumference where each computed displacement component is largest, and (b) the element and the angle along the circumference where each computed stress resultant is largest.

11. For the nonlinear analysis, a printout for each load increment and for each iteration is presented for displacement, strain and stress resultant components.

III.3 INPUT DATA DESCRIPTION

A general description of the input data for the linear as well as for the nonlinear computer programs is presented.

The sequence of information to be provided is as follows:

A. Number of problems to be solved in a single run (NPROBL).

B. Explanatory comments (TITL). As many as 20 cards can be used for general comments as long as punching starts in column 1.

If the number of comment cards is less than 20 input a blank card after all comment cards.

C. One card providing information concerning number of harmonics, tape number assignment, etc. The only difference in the input data for the linear and nonlinear programs is in the information given in this card.

C(linear): for the linear program use this card (IA,IB, ISCRA3,

NELEMS, NET, NEREST, NUNKTH, NAMES, HS, NA, NB, IDEN, PROBLE, ISAVE) and omit the card C (nonlinear).

C(nonlinear): for the nonlinear program use this card (IA, IB, ISCRA3, NELEMS, NET, NEREST, NUNKTH, NA, NB, NDELQ, ITER, ITLAST, ISAVE, NAMES, HS, IDEN, PROBLE, EROR, X, NSTRAI, IPLAST, XSM) and omit the card C(linear).

- D. If the load is applied over a circular area give information concerning the location of the loaded area, the radius of the circular area, etc. (APPO, THELO, RC, RL, PR). Otherwise, input a blank card and loading is described according to cards L1 and/or L2.
- E. Angles (in degrees) measured along the circumference where displacements, strains and stresses are to be evaluated (XTHETA).
- F. Define shell geometry:

F1 - Shell has a geometry as described in Fig. 9.

F1.a - Input RCAP, RFILET and RMAX. If there is not a fillet, i.e., the shell is spherical or the sphere is connected directly to the cylinder, RFILET = 0.

There must be continuity in slope between sphere and fillet.

If there is a cylinder, RMAX is the radius of the cylinder.

If there is not a cylinder RMAX is the maximum radial (cylindrical) coordinate of the shell.

Next, divide the shell into n parts. In the case of Fig. 9, the shell was divided into 8 parts. Define N as NS + NF, where NS represents

the number of parts of the sphere and NF the number of parts of the fillet. If there is not a fillet, NF = 0.

F1.b - Input NPARTS = N if there is not a cylinder; Input NPARTS = N + 1, if there is a cylinder. (In the case of Fig. 9, NPARTS = 6).

F1.c - Input number of elements in part 1, angle ϕ_1 and state if there is a cylinder connected at the end of part 1 (NE1, T1, NCYLIN).

F1.d - Input number of elements in part 2, angle ϕ_2 and coding for the cylinder (NE2, T2, NCYLIN).

F1.e - Repeat for all parts of the sphere and fillet.

F1.f - Input number of elements in part 1 (NE1) of cylinder and distance S1 (NE1, S1).

F1.g - Repeat for all parts of the cylinder.

F2 - If the shell has any surface of revolution shape (Fig. 10), give for each element Z1, Z2, R01, PHY1 and PHY2.

NOTE 1: If the shell has a geometry as described in Fig. 9 input a blank card just after the E cards and the geometry is described by the F1 cards.

NOTE 2: If the shell has any surface of revolution shape, all F1 cards are omitted.

G. - Element thickness. Thickness can be given for one element or for a set of elements if all elements in the set have the same thickness. If an element has a varying thickness, this variation is described by giving the thickness (THI) at various angles (THET) along the circumference. Thicknesses are assumed to be constant from one angle to the next.

- H. Element modulus of elasticity (EM). Here again, this information can be given for one element or for a set of elements.
- I. Element Poisson's ratio (POISSO). This also can be input for one element or for a set of elements.
- J. Elements restrained (IELRES). Specify which elements are restrained.
- K. Describe how each element is restrained (REST).
- L. Loads on the structure other than loads applied over a circular area. There are two types of loading: (a) uniform load on the element and (b) concentrated ring load at each node of the element.
 - L1 - Uniform load: Give each load component (meridional, normal and tangential) followed by the angle (in degrees) along the circumference identifying beginning of applied load. Loads are assumed to be constant from one angle to the next (P,R,S,0).
 - L2 - Ring load: Ring loads are given in the cylindrical coordinate system. Give ring load for each harmonic. Input total load instead of load per unit length.

Card (A)

COLUMNS	VARIABLE NAME	DESCRIPTION
1-5	NPROBL	Number of problems to be solved in a single run. Integer, right justified.

Comment cards (B)

1-80	TITL	Any alphanumeric information can be punched in these cards as long as punching starts in column 1. Up to 20 cards can be used. If the number of comments cards is less than 20, input a blank card after all comment cards.
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C card (linear)

3-4	IA	Number of A (cosine) harmonics
7-8	IB	Number of B (sine) harmonics (Maximum of 17 IA + IB harmonics)
11-12	ISCRA3	This field is used to assign a logical unit number to one binary tape
15-16	NELEMS	Number of elements (50 maximum)
19-20	NET	Number of Simpson's stations to be used in numerical integrations (29 maximum)
23-24	NEREST	Number of restrained elements (10 maximum)

COLUMNS	VARIABLE NAME	DESCRIPTION
27-28	NUNKTH	Number of angles where displacements, strains and stresses are to be evaluated (10 maximum)
29-32	NAMES	If the structure is a sandwich type shell, punch in this field the word "SAND". Otherwise leave blank.
33-42	HS	If the structure is a sandwich type shell punch, anywhere in this field, the distance between face sheets. Use a decimal point.
43-45	NA	Number of cosine terms in the Fourier expansion of the element thickness
46-48	NB	Number of sine terms in the Fourier expansion (maximum of 49 NA + NB)
49-52	IDEN	Punch in this field any alphanumeric information for identification purposes. It can be left blank.
53-56	PROBLE	If the structure to be solved was solved previously and tape ISCRA3 containing shell geometric and elastic properties was saved and is to be used for this problem punch in this field the word

COLUMNS	VARIABLE NAME	DESCRIPTION
57	ISAVE	<p>"RRUN". In the case of a RRUN (rerun), the tape from the previous run must be mounted on logical unit ISCRA3 before execution. Be sure to use as ISCRA3 a logical unit which is not used by the system to avoid destruction of information on the tape. If it is not a RRUN, leave this field blank.</p> <p>If this field is left blank, all geometric and elastic properties of the shell are stored on tape ISCRA3 and it can be used later on in a rerun. A message is printed on line to save tape ISCRA3.</p> <p>In the case of a RRUN, input only comment cards (B), card C, card D, card E and loading. Be sure to have in each run an A card. If one does not want to save tape ISCRA3, punch in this field any number different from zero.</p>

NOTE: Except as noted, all variables must be integers, right justified.

C card (nonlinear)

For the nonlinear program use this card instead of the card C previously described. For the linear program omit this card.

COLUMNS	VARIABLE NAME	DESCRIPTION
2-3	IA	Number of A (cosine) harmonics
5-6	IB	Number of B (sine) harmonics
8-9	ISCRA3	This field is used to assign a logical unit number to one binary tape.
11-12	NELEMS	Number of elements (50 maximum)
14-15	NET	Number of Simpson's stations to be used in numerical integrations (29 maximum)
17-18	NEREST	Number of restrained elements (10 maximum)
20-21	NUNKTH	Number of angles where displacement, strains and stresses are to be evaluated (10 maximum)
23-24	NA	Number of cosine terms in the Fourier expansion of the element thickness.
26-27	NB	Number of sine terms in the Fourier expansion (maximum of $49 NA + NB$).
28-30	NDELQ	Number of load increments.
31-33	ITER	Maximum number of iterations allowed for each load increment. Depending on the approach chosen to solve the equilibrium equations, the variable ITER may be positive, negative or zero. If ITER is positive, iterations will be performed for each load increment

COLUMNS	VARIABLE NAME	DESCRIPTION
		<p data-bbox="950 633 1701 1510">until convergence is satisfied according to the specified EROR. If the solution does not converge after ITER iterations the program stops. If ITER is negative, the program iterates ITER times for each load increment, without checking for convergence. If ITER = 0, no iteration is performed unless the variable ITLAST is greater than zero, in which case there are ITLAST iterations for the last load increment.</p>
34-36	ITLAST	<p data-bbox="950 1553 1716 1899">If ITER is zero or negative and ITLAST is greater than ITER the program will iterate ITLAST times for the last load increment. If ITLAST = 0, the program proceeds according to the value of ITER.</p>
39	ISAVE	<p data-bbox="950 1941 1740 2516">If this field is left blank, all geometric and elastic properties of the shell are stored on tape ISCRA3 and they can be used later on in a RRUN (rerun). A message is printed on line to save tape ISCRA3. In the case of a RRUN input only comment cards (B), card C, card D, card E and loading.</p>

COLUMNS	VARIABLE NAME	DESCRIPTION
40-43	NAMES	<p>Be sure to have in each run an A card. If one does not want to save tape ISCRA3, punch in this field any number different from zero.</p> <p>If the structure is a sandwich type shell, punch in this field the word "SAND". Otherwise leave it blank.</p>
44- 48	HS	<p>If the structure is a sandwich type shell, punch, anywhere in this field, the distance between face sheets. Use a decimal point.</p>
49-52	IDEN	<p>Punch in this field any alphanumeric information for identification purposes. It can be left blank.</p>
53-56	PROBLE	<p>If the structure to be solved was solved previously and tape ISCRA3 containing shell geometric and elastic properties was saved and is to be used for this problem, punch in this field the word "RRUN". In</p>

the case of a RRUN (rerun), the tape from the previous run must be mounted on logical unit ISCRA3 before execution. Be sure to use as ISCRA3 a logical unit which is not used by the system to avoid destruction of information on the tape. If it is not a RRUN, leave this field blank. If ITER is positive, iteration will be performed for each load increment until convergence is satisfied according to the specified EROR, i.e.,

$$\frac{q_i - q_{i-1}}{q_i} \leq \text{EROR}$$

where q_i represents the displacement for the i th iteration and q_{i-1} is the displacement for the $(i-1)$ th iteration. If ITER is zero or negative, leave this field blank.

57-62

EROR

63-68

X

Extrapolation factor to calculate the generalized loads due to nonlinearities. If $X = 0$, $q_i = q_{i-1}$ and the nonlinear terms are evaluated based on the displacements at the previous loading. If $X = 1$ the usual quadratic extrapolation occurs.

COLUMNS	VARIABLE NAME	DESCRIPTION
69	NSTRAI	If this field is left blank information concerning the strains and changes in curvature will be printed. If one does not want to have the strains and changes in curvature printed out, punch in this field any number different from zero.
70	IPLAST	If IPLAST is zero, the strains, changes in curvature and stresses are evaluated only when convergence occurs. Otherwise punch any other number in this field and the strains, changes in curvature and stresses are evaluated for each iteration.
71-77	XSM	<p>It may occur that for some problems the displacements in some region of the shell become very small and the convergence criterion</p> $\frac{q_i - q_{i-1}}{q_i} \leq \text{EROR}$ <p>may require an extra number of iterations. Punch anywhere in this field the absolute value of the smallest displacement regarded as significant. If the displacements are</p>

COLUMNS	VARIABLE NAME	DESCRIPTION
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smaller than this specified value there is no checking for convergence for these displacements. This field can be left blank.

D card

1-4	APPO	If the load is applied over a circular area (Fig. 11), punch the word "APOL".
11-20	THELO	Angle θ in degrees.
21-30	RC	Distance R_c
31-40	RL	Radius R_L
41-50	PR	Pressure P_r

NOTE 1: Variables can be punched anywhere in the field. Use a decimal point.

NOTE 2: The center of the load must be situated inside the region regarded as first and second quadrants.

NOTE 3: If the load is not applied over a circular area, input a blank card and the loading is described using cards L1 and/or L2.

E card

1-10	XTHETA	Angle (in degrees) where displacements, strains and stresses are to be evaluated.
11-20		
21-30		Use two cards if the number of angles is greater than 8. The number can be punched anywhere in the field with a decimal point
31-40		
41-50		

COLUMNS	VARIABLE NAME	DESCRIPTION
51-60		The number of angles must agree with the
61-70		value of NUNKTH given previously on
71-80		card C.

Geometry card (Fl.a)

1-10	RCAP	Radius of the sphere
11-20	RFILET	Radius of the fillet
21-30	RMAX	If there is a cylinder, input the radius of the cylinder, if there is not a cylinder, input the maximum radial (cylindrical) coordinate of the shell. (See Figs. 9 and 12).

NOTE: These variables can be punched anywhere in the field, with a decimal point.

Geometry card (Fl.b)

1-5	NPARTS	(See description given previously Fl.b) Integer, right justified.
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Geometry card (Fl.c, Fl.d, Fl.e)

There will be N of these cards ($N = NS + NF$).
One card for each part of the sphere and

COLUMNS	VARIABLE NAME	DESCRIPTION
		the fillet.
1-5	NE1	Number of elements in the part. Integer right justified.
6-15	T1	Punch anywhere in this field the angle ϕ_1 (in deg), with a decimal point (See Fig. 9).
16	NCYLIN	If there is a cylinder connected at the end of the part punch 1 in this column. Otherwise, leave it blank. NOTE: Repeat for all parts of the sphere and fillet.

Geometry card (Fl.f, Fl.g)

There will be Ncy cards, where Ncy is the number of parts in the cylinder. If there is not a cylinder, these cards are omitted.

1-5	NE1	Number of elements in the part. Integer, right justified.
6-15	S1	Punch anywhere in this field the distance between subdivisions (See Fig. 9) in the cylinder. Use a decimal point. NOTE: Repeat for all parts of the cylinder.

COLUMNS	VARIABLE NAME	DESCRIPTION
Geometry card (F2)		
See Fig. 10. If the shell has any surface of revolution shape, give for each element the information below in E12.5 FORMAT. Consequently, there will be NELEMS of these cards, where NELEMS represents the number of elements in the shell. In this case all F1 cards are omitted.		
1-12	Z1	Axial coordinate of element first node
13-24	Z2	Axial coordinate of element second node
25-36	R01	Radial (cylindrical) coordinate of element first node
37-48	R02	Radial (cylindrical) coordinate of element second node
49-60	PHY1	Angle between meridian and the axis of revolution, at the element first node
61-72	PHY2	Angle between meridian and the axis of revolution, at the element second node
NOTE 1: Repeat for all elements in the shell.		
NOTE 2: If cards F1 are given, cards F2 are omitted and input a blank card after the E cards.		

COLUMNS	VARIABLE NAME	DESCRIPTION
		Element thickness (G)
1-4	NAMET	Punch in this field the word "CONS" if the thickness is constant over one element or a set of elements. Otherwise leave it blank.
9-10	IEL1	First element number of a set of elements where the thickness is described (Fig. 4).
14-15	IEL2	Last element number of the set. IEL1 and IEL2 are integers right justified. If the set contains only one element the fields for IEL1 and IEL2 may be left blank.
16-25	THIK	If the thickness is constant over each element of the set, punch anywhere in the field the thickness. Use a decimal point. If the thickness is not constant leave it blank.
29-30	NC	Number of angles where the thickness is specified. If the thickness is constant this field may be left blank. NC is an integer, right justified. If the thickness is not constant, this additional information must be given to describe it.

COLUMNS	VARIABLE NAME	DESCRIPTION
1-10	THET	Punch anywhere in this field the angle in degrees, where the thickness is specified. The first point to be given is at 0.0 degrees.
11-20	THI	Anywhere in this field the thickness corresponding to the angle THET must be given.
21-30	THET	NC values of THET and
31-40	THI	THI (angle and thickness) must be given in
41-50	THET	the subsequent fields. If NC is larger
51-60	THI	than 4, use additional
61-70	THET	cards. A decimal point
71-80	THI	must be always used.

Modulus of elasticity (H)

1-4	NAM	Punch in this field the word "CONS" if the modulus of elasticity is the same for a set of elements.
9-10	IEL1	First element number of a set of elements with a constant modulus of elasticity.

COLUMNS	VARIABLE NAME	DESCRIPTION
14-15	IEL2	Last element number of the set. If the set contains only one element the fields for NAM, IEL1 and IEL2 may be left blank. IEL1 and IEL2 are integers, right justified.
16-25	EM	The value of the modulus of elasticity for the set. It can be punched anywhere in this field, with a decimal point. There will be one card if the modulus of elasticity is constant for all elements and several cards if the modulus of elasticity varies with the element.

Poisson's ratio (I)

1-4	NAM	Follow the same instructions as
9-10	IEL1	for the modulus of elasticity
14-15	IEL2	
16-25	POISSO	

Boundary conditions (J) - Which nodes are restrained

1-5	IELRES	Punch in these fields the number
6-10		of each element having a restraint.

COLUMNS	VARIABLE NAME	DESCRIPTION
11-15		One can have up to 10 restrained
16-20		elements. Element number must be
21-25		right justified.
26-30		For example. If the structure has
31-35		50 elements and the last
36-40		node is fixed, punch
41-45		the number 50 in columns 4 and 5.
46-50		

Boundary conditions (K) - Specify types of restrains

		Input one card for each restrained element.
1-5	J	Number of the restrained element. Integer, right justified.
10	REST	If axial component of the first node of element J is restrained, punch 1 in this column. Otherwise leave it blank.
15		Same for the tangential component of the first node.
20		Same for the radial component of the first node.
25		Same for the angular component of the first node.

COLUMNS	VARIABLE NAME	DESCRIPTION
30		These fields are for the displacement components of the second node of element J.
35		
40		
45		
Uniform load (L)		
1-5	IELL	Element number where load is applied. Integer, right justified.
9-10	NDPP	Number of point loads. Integer, right justified (Fig. 3).
11-14	NSTOP	If load is uniformly distributed over all subsequent elements starting with element IELL, punch in this field the word "CONS". Other wise leave it blank.
16	IXXX	Punch 1 in this field if there is a ring load applied at any node of this element, for the first harmonic. Otherwise leave it blank.
17		Punch 1, if there is a ring load
18		for the second harmonic,
19		for the third harmonic, and so on, for
20		all harmonics.

COLUMNS	VARIABLE NAME	DESCRIPTION
21		
etc.		
32		

Uniform load (L1)

1-10	P	Meridional load component per unit area
11-20	R	Normal load " " " "
21-30	S	Tangential load " " " "
31-40	O	Angle in degrees where these loads are

applied. The first point to be given is at 0.0 degrees. Variables can be punched anywhere in the field. Use a decimal point. Input as many as NDPP cards. Maximum of 74 point loads
If the load is applied over a circular area and it was described by card D, omit these cards.

Ring load (L2)

If there is a ring load applied at the element, give for each harmonic the corresponding ring load. Ring loads are applied in the cylindrical coordinate system. Input the total ring load rather than load per unit length.

COLUMNS	VARIABLE NAME	DESCRIPTION
1-10	FORCE	Axial component at the element first node
11-20		Tangential " " " " " "
21-30		Radial " " " " " "
31-40		Angular " " " " " "
41-50		Axial " " " " second "
51-60		Tangential " " " " " "
61-70		Radial " " " " " "
71-80		Angular " " " " " "

Variables can be punched anywhere in the field. Use a decimal point.

CHAPTER IV
EXAMPLE PROBLEMS

IV.1 - SAMPLE PROBLEM NO. 1 -

The first example problem is for the nonlinear elastic analysis of the scalloped Apollo aft heat shield. The aft heat shield is of honeycomb construction with scalloped face sheets. The face sheet thicknesses and shell geometry are shown in Fig. 13.

The structure is subjected to a uniform pressure of - 200 psi distributed over a circular area of twenty inches radius and a pitch angle of 15° (Fig. 14). The shell is clamped at the base and 50 elements are used in the analysis. The Poisson's ratio and the Young's modulus are constant over the entire shell. As the loading and the thickness variation are symmetrical about the origin ($\theta = 0^\circ$) only A harmonics are needed. The pressure loading is applied in four increments of fifty psi each and the increment-iteration method is used to solve the equilibrium equations. A maximum of twenty iterations for each load increment is used. The listing of the input data and the corresponding output are given in Appendix A.

IV.2 - SAMPLE PROBLEM NO. 2 -

This sample problem is a hemispherical shell, of five inches radius, under a uniform external pressure of - 100 psi distributed over its entire surface (Fig. 15). The structure is subdivided into thirty elements

and it is assumed to be clamped at the base. The Poisson's ratio, Young's modulus and thickness are constant for all elements. As the shell is anticipated to deform symmetrically only A harmonics are needed.

IV. 3 - SAMPLE PROBLEM NO. 3 -

This problem is a re-run of the sample problem no. 2 with a new loading. The shell is subjected to a uniform internal pressure of 10 psi applied on elements 26 through 30. The elastic and geometric properties of the structure are the same as for the previous example and both problems are solved in a single run.

IV. 4 - SAMPLE PROBLEM NO. 4 -

A circular plate with a circular hole under a total ring load of 10 lbs. is analyzed (Fig. 16). The geometry of the plate is input rather than generated by the computer. The radius of the plate is ten inches and the radius of the circular hole is two inches. The structure is subdivided into eight elements and it is assumed to be simply supported at node nine.

IV. 5 - SAMPLE PROBLEM NO. 5 -

Next consider a spherical cap of twenty inches radius with varying thickness properties in the circumferential direction (Fig. 17.) The shell is subjected to a uniform pressure of - 10 psi distributed over a circular area of three inches radius. The center of the circular area where the loading is applied is located at ten inches from the center of the shell and at 45° from the origin ($\theta = 0^\circ$). The shell is subdivided into twenty

elements and clamped at the base. The structure is anticipated to deflect asymmetrically and both A and B harmonics are required. The displacements, strains and stresses are evaluated at four different angles along the circumference.

IV. 6 - SAMPLE PROBLEM NO. 6 -

The shell of sample problem no. 5 is re-analyzed with a new loading. The center of the circular area where the loading is applied is moved to the origin ($\theta = 0^\circ$). The displacements, strains and stresses are evaluated at six different angles along the circumference. This example is a re-run of problem no. 5 using the tape ISCRA3 saved from the previous run.

IV. 7 - SAMPLE PROBLEM NO. 7 -

This structure is a combination of a spherical cap and a cylinder (Fig. 18). The radius of the spherical cap is ten inches. The radius of the cylinder is also ten inches and its depth is fifty inches. Both cap and cylinder are divided into 25 elements. Therefore, the total number of elements in the structural idealization is fifty.

As the shell is subjected to a uniform pressure and the structure deforms axisymmetrically only the 0A harmonic is needed. The node 26 is restrained in the radial direction and furthermore the structure is clamped at the base.

REFERENCES

1. Stricklin, J.A., Haisler, W.E., MacDougall, H.R. and Stebbins, F.J., Nonlinear Analysis of Shells of Revolution by the Matrix Displacement Method, AIAA Paper No. 68-117, presented at AIAA 6th Aerospace Sciences Meeting, New York, January, 1968.
2. Stricklin, J.A., DeAndrade, J.C., Stebbins, F.J. and Cwiertny, Jr. A.J., Linear and Nonlinear Analysis of Shells of Revolution with Asymmetrical Stiffness Properties, paper to be presented at the Air Force Second Conference on Matrix Methods in Structural Mechanics, Wright Patterson Air Force Base, Ohio, 15-17 October 1968.
3. Novozhilov, V.V., Foundations of the Nonlinear Theory of Elasticity, Graylock Press, Rochester, New York, 1953.
4. Langhaar, H.L., Energy Methods in Applied Mechanics, Chapter V, John Wiley and Sons, 1962.

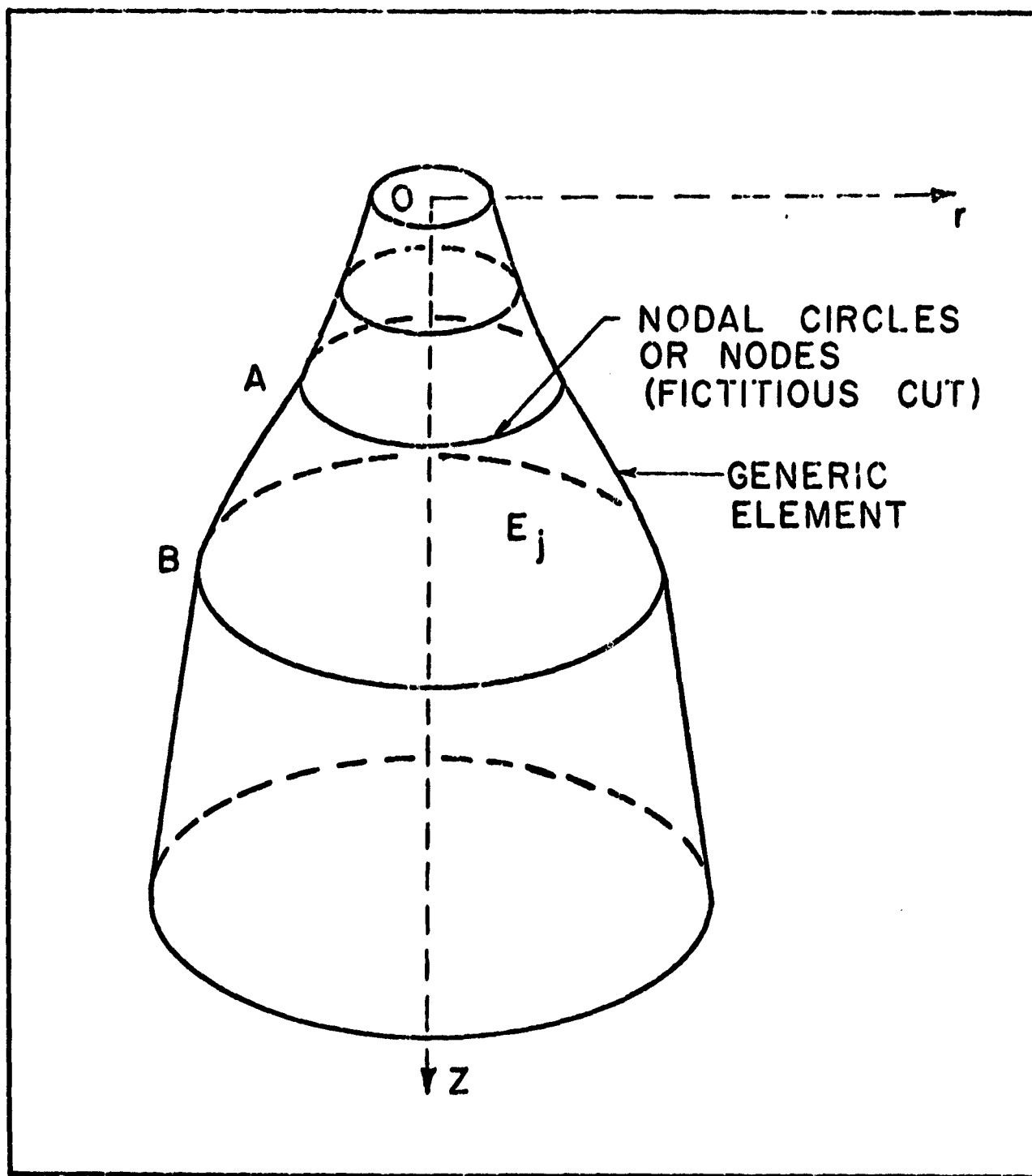


FIG. 1 A SHELL OF REVOLUTION
DIVIDED INTO A NUMBER
OF CURVED ELEMENTS

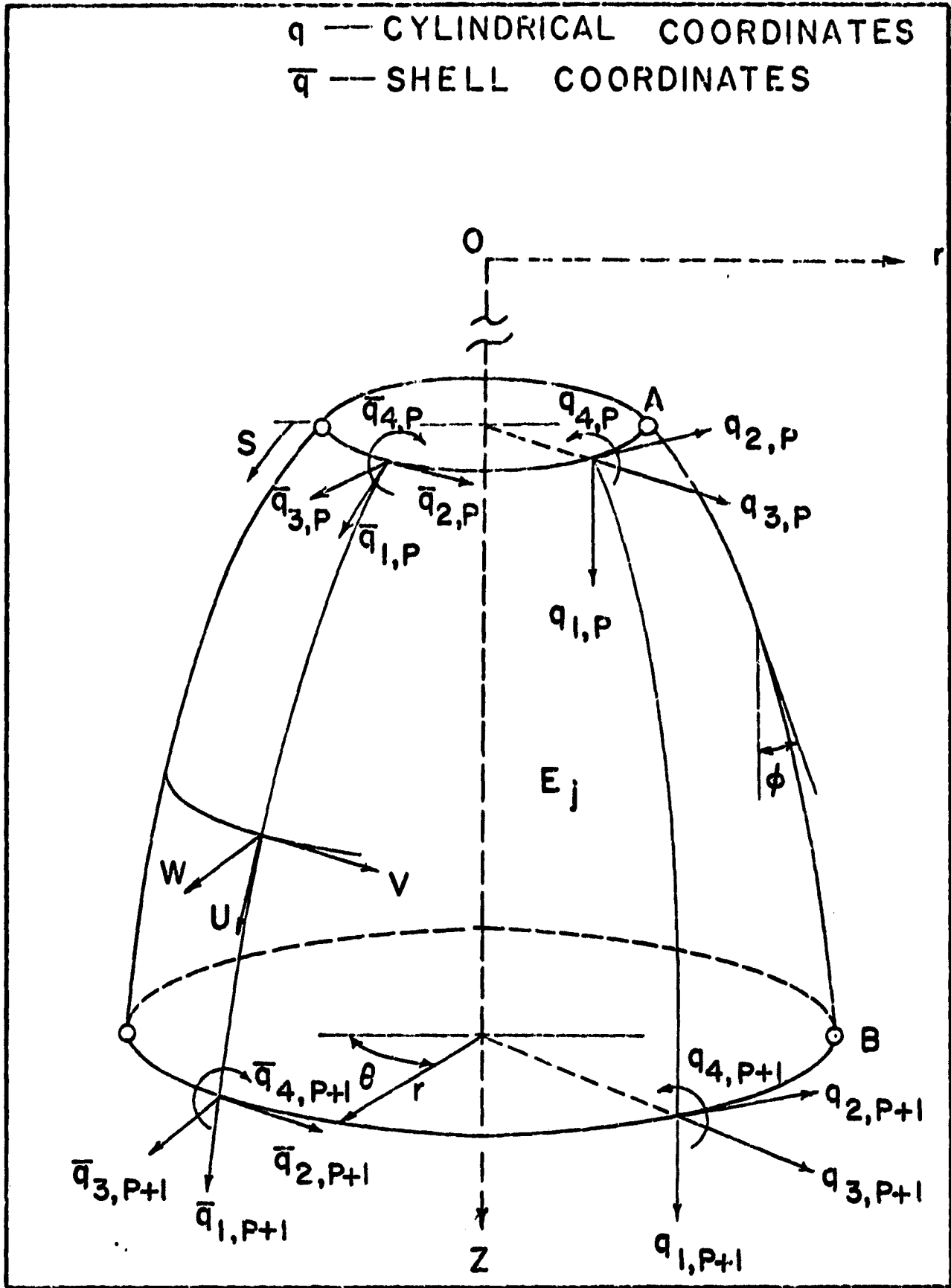


FIG. 2 A GENERIC ELEMENT

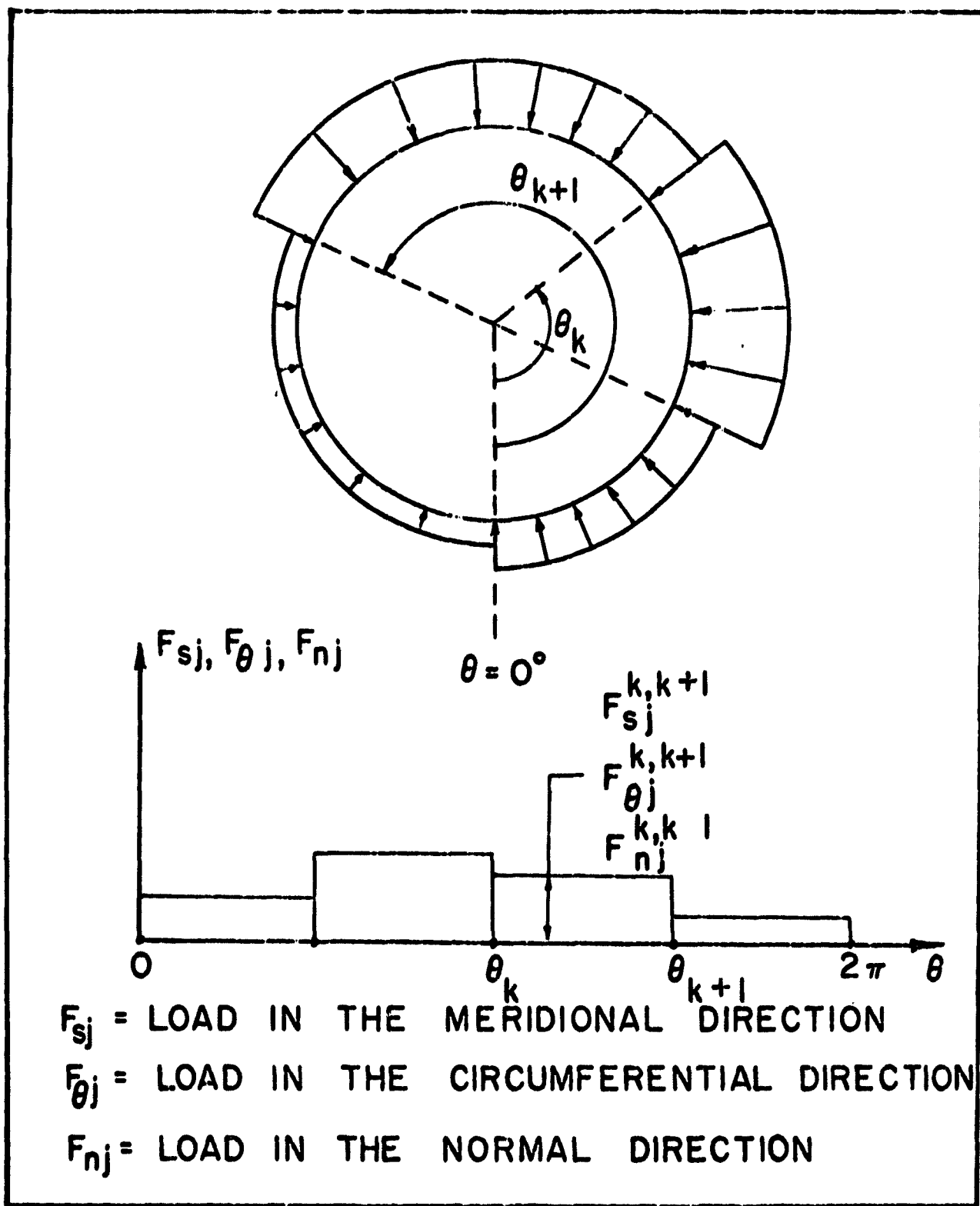


FIG. 3 LOADING VARIATION IN THE CIRCUMFERENTIAL DIRECTION

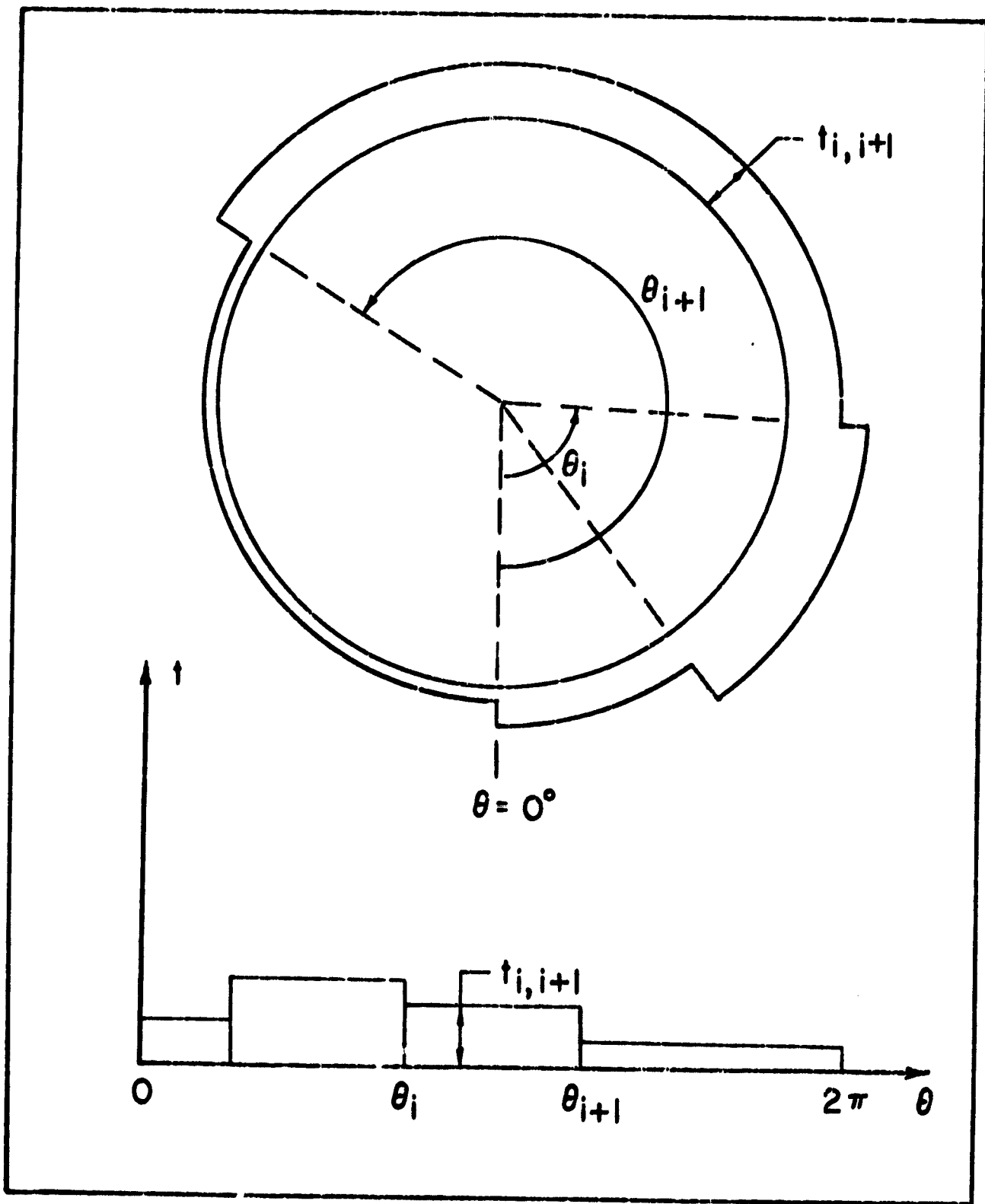


FIG. 4 THICKNESS VARIATION IN THE CIRCUMFERENTIAL DIRECTION

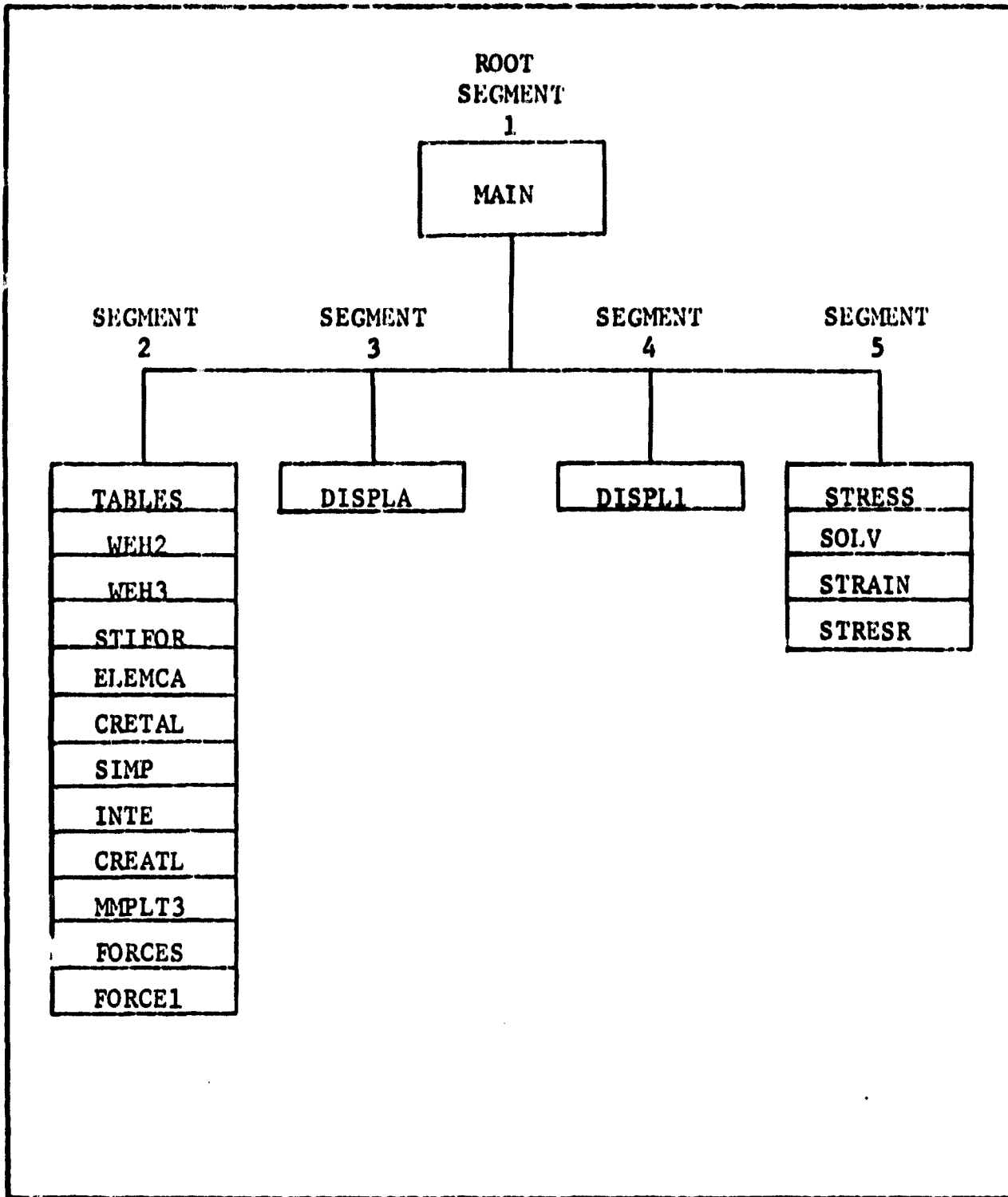


Fig. 5 OVERLAY STRUCTURE
LINEAR PROGRAM

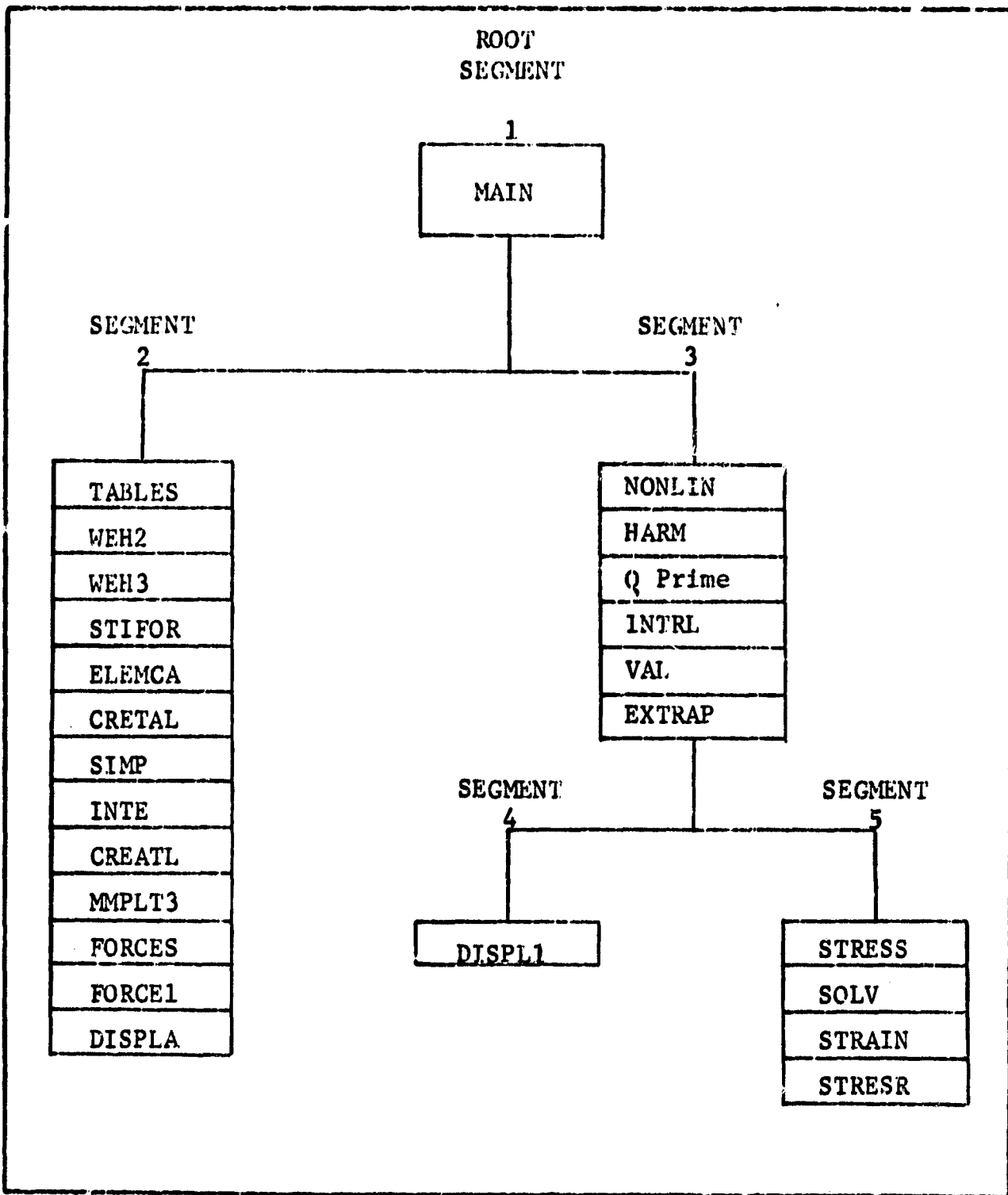


Fig. 6 OVERLAY STRUCTURE
NONLINEAR PROGRAM

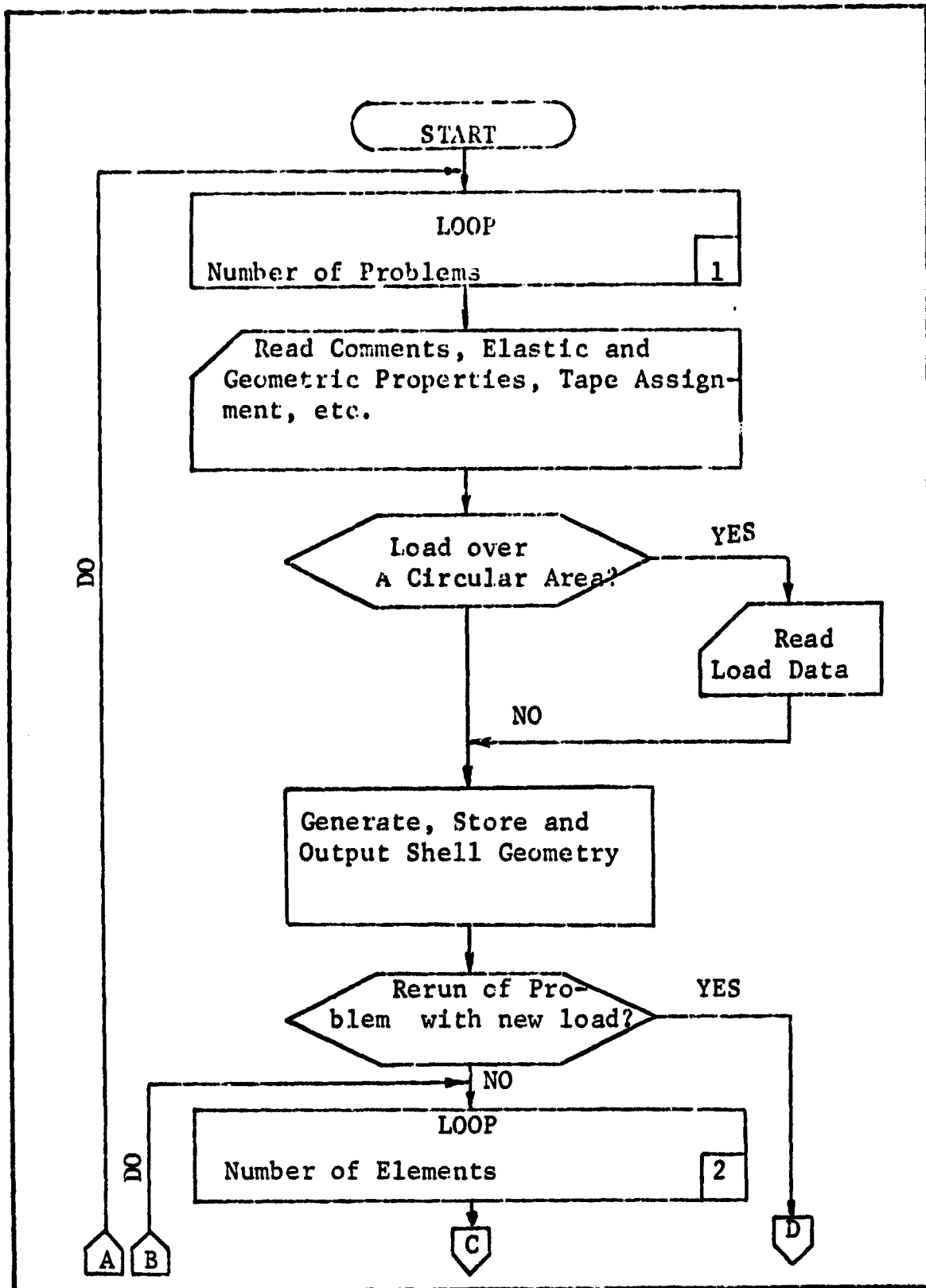


Fig. 7 FLOW CHART FOR THE LINEAR PROGRAM

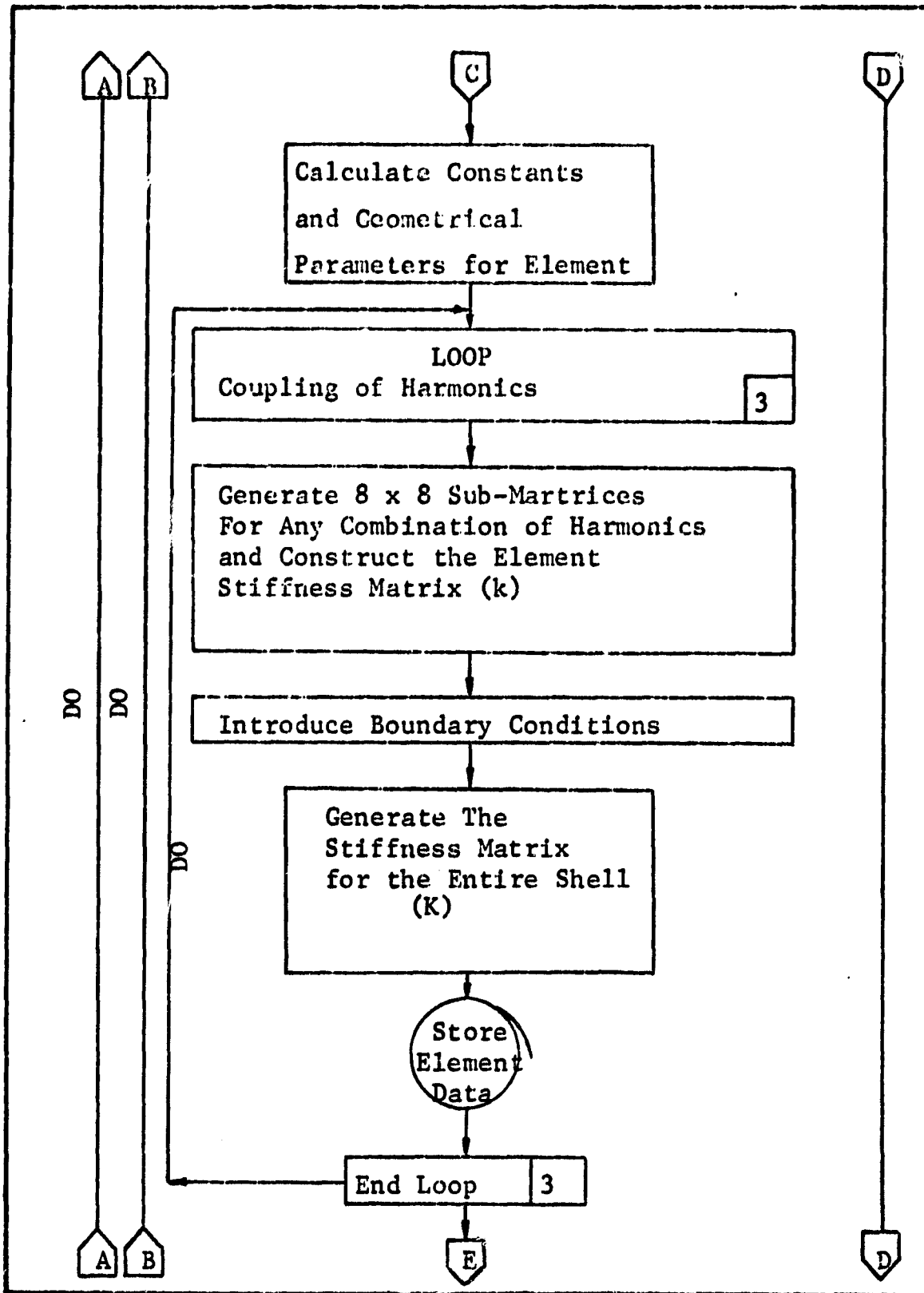


Fig. 7 FLOW CHART FOR THE LINEAR PROGRAM (cont.)

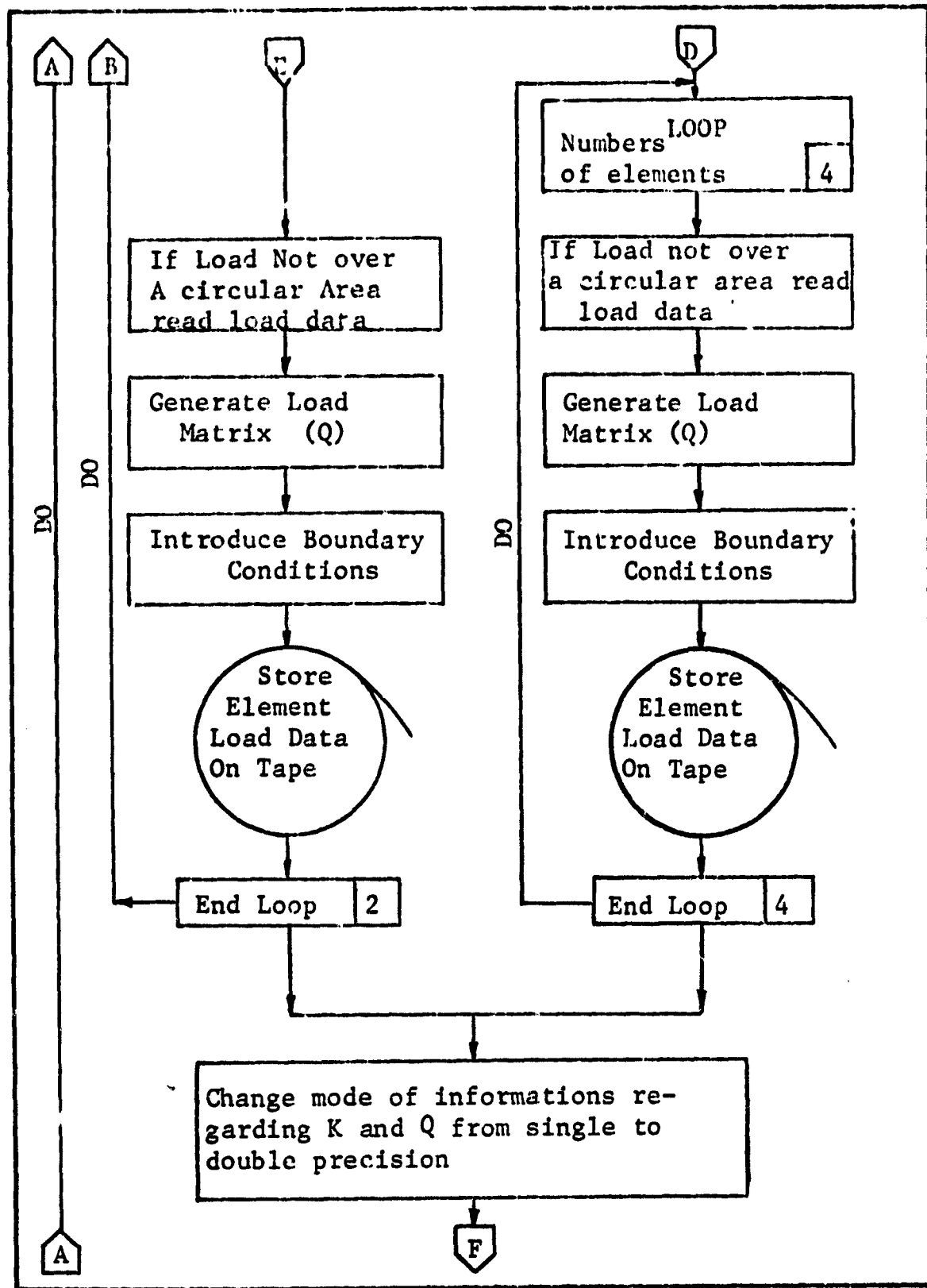


Fig. 7 FLOW CHART FOR THE LINEAR PROGRAM (cont.)

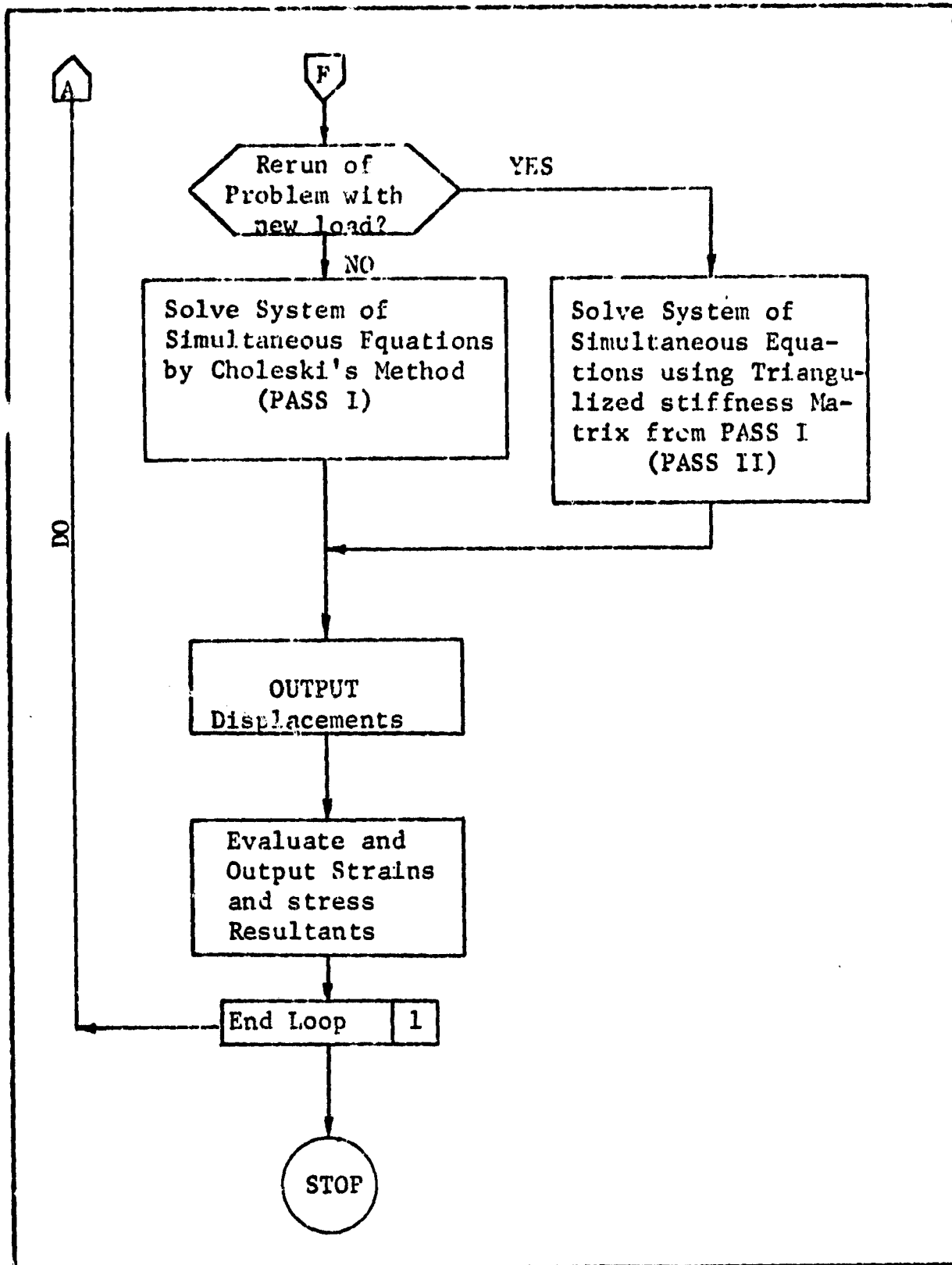


Fig. 7 FLOW CHART FOR THE LINEAR PROGRAM (cont.)

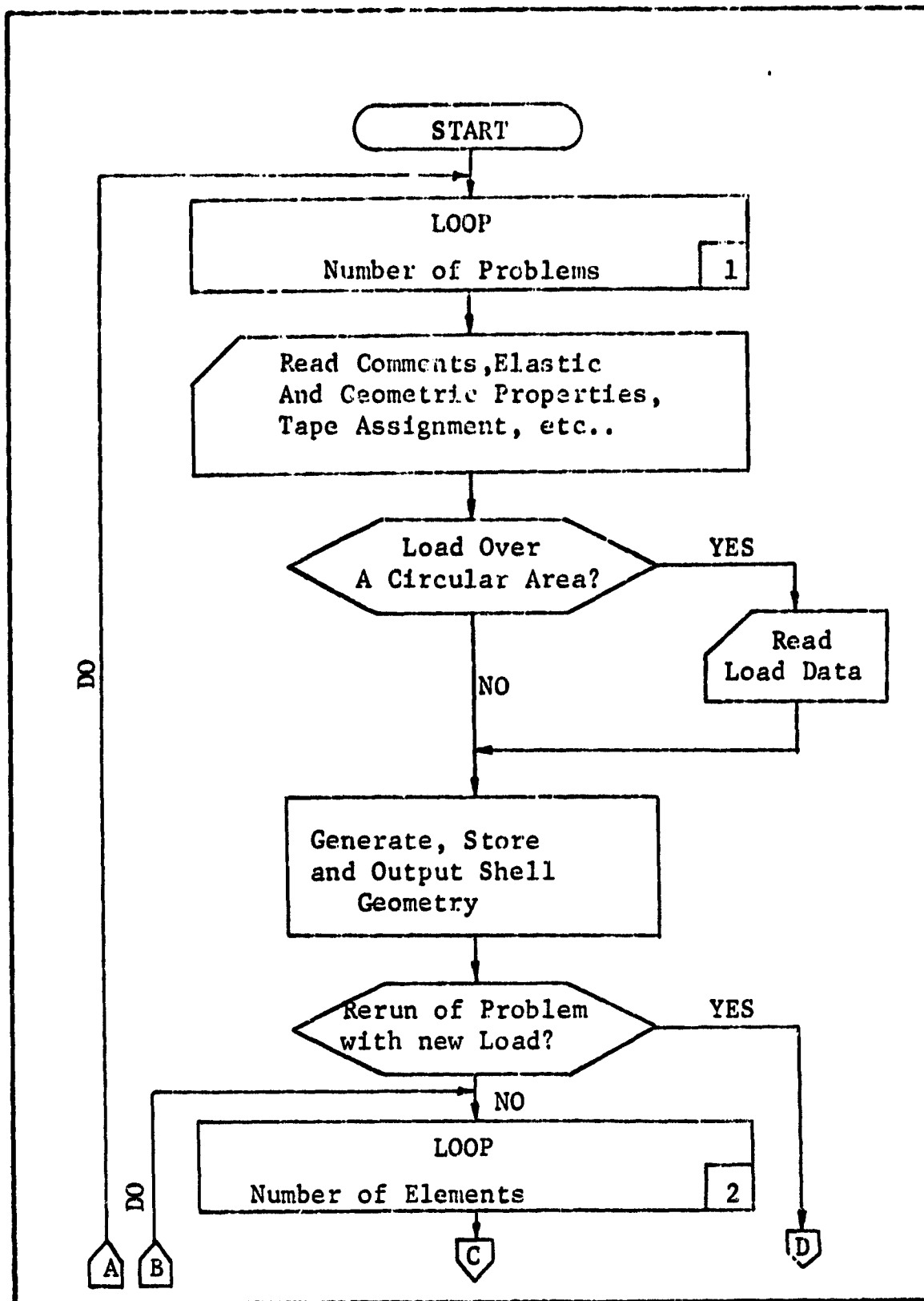


Fig. 8 FLOW CHART FOR THE NONLINEAR PROGRAM

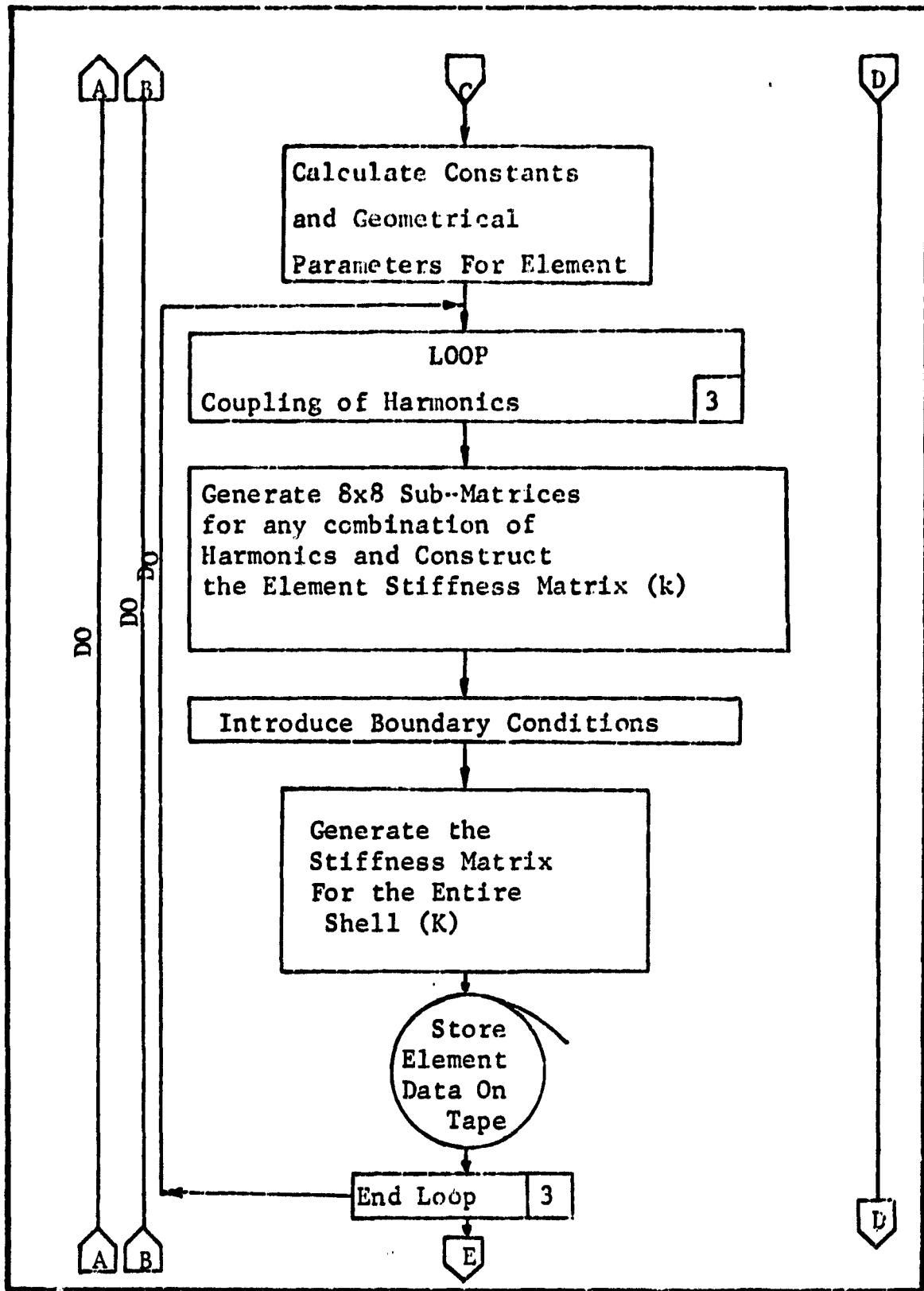


Fig. 8 FLOW CHART FOR THE NONLINEAR PROGRAM (cont.)

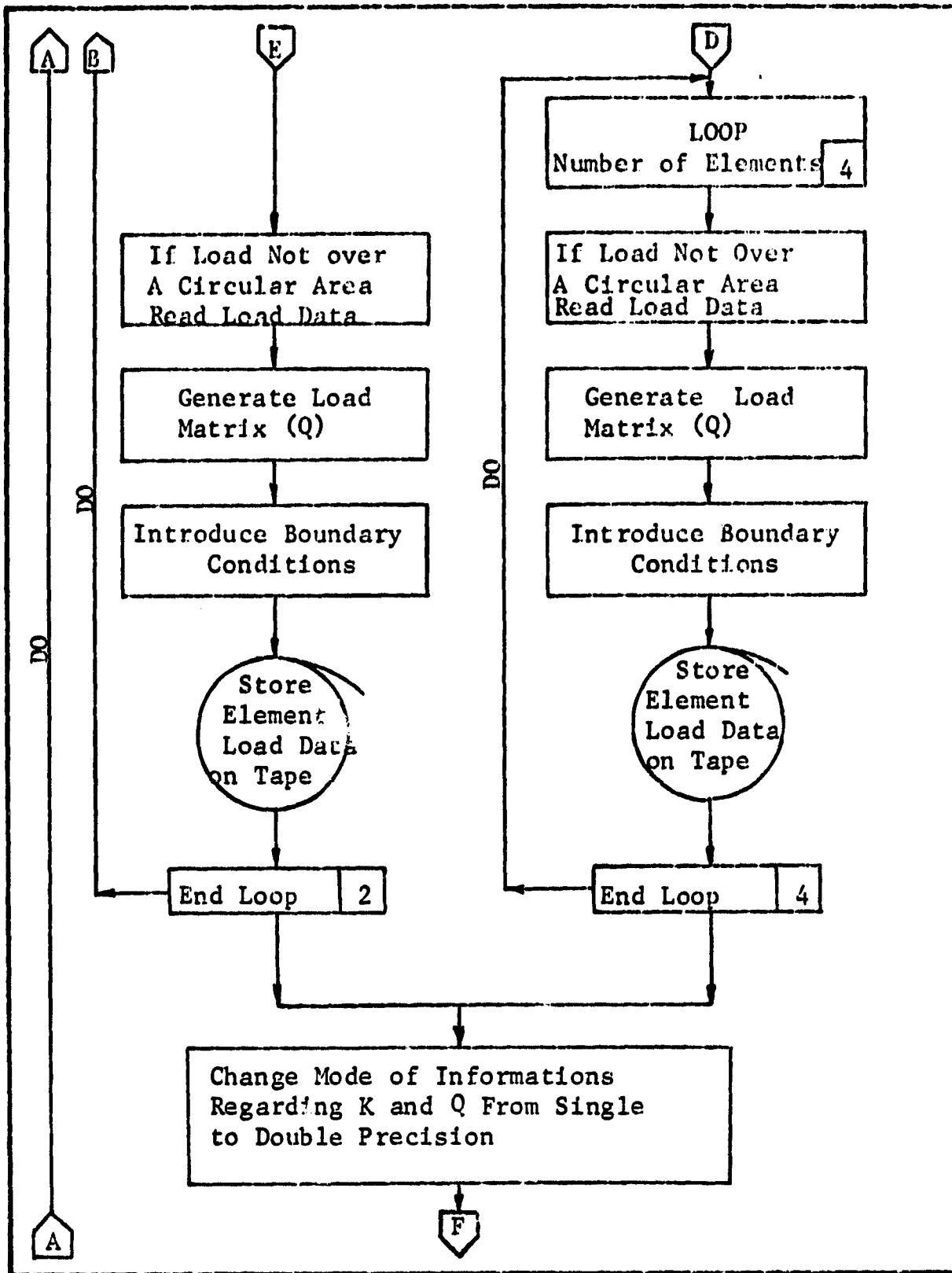


Fig. 8 FLOW CHART FOR THE NONLINEAR PROGRAM (cont.)

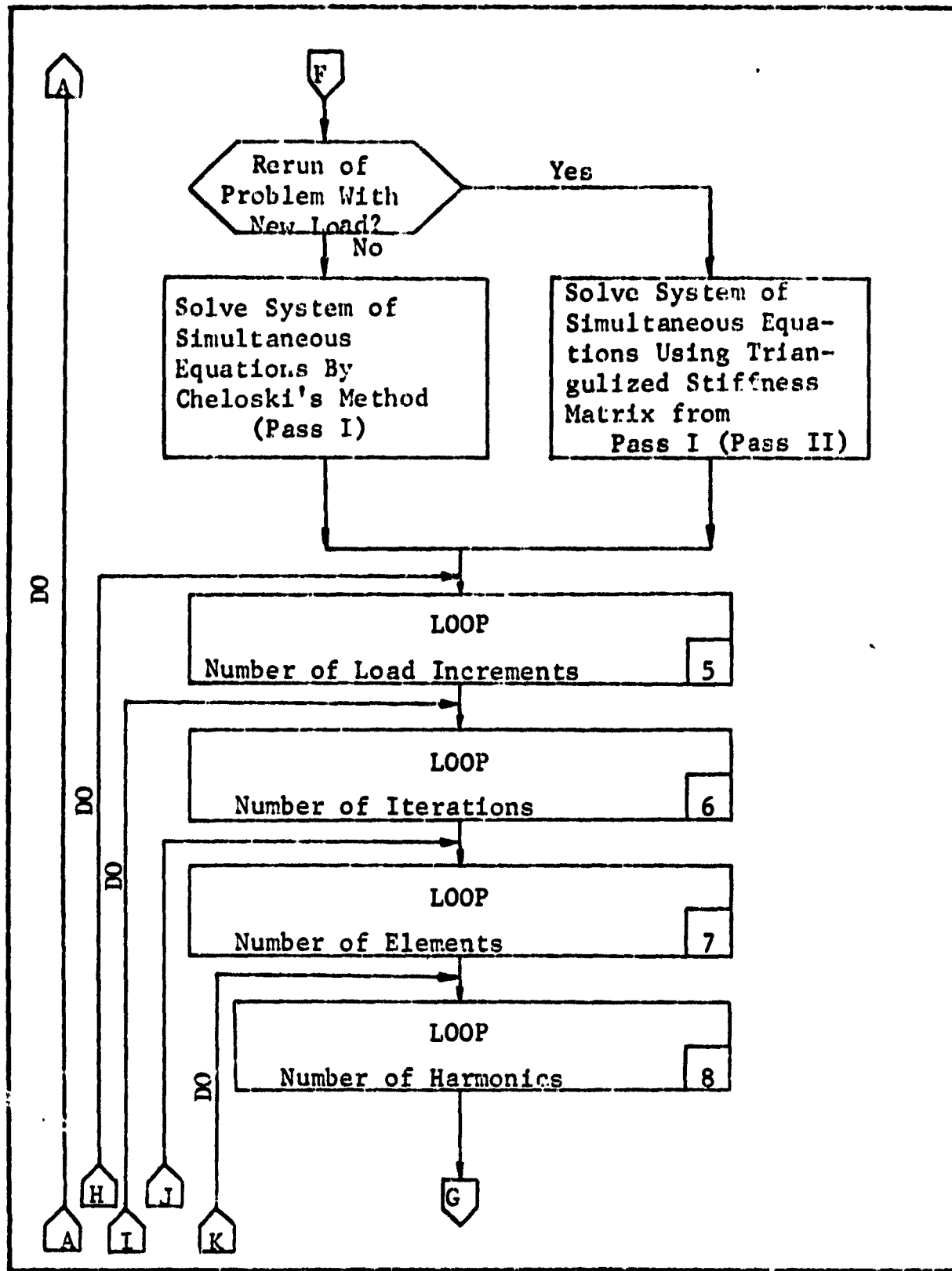


Fig. 8 FLOW CHART FOR THE NONLINEAR PROGRAM (cont.)

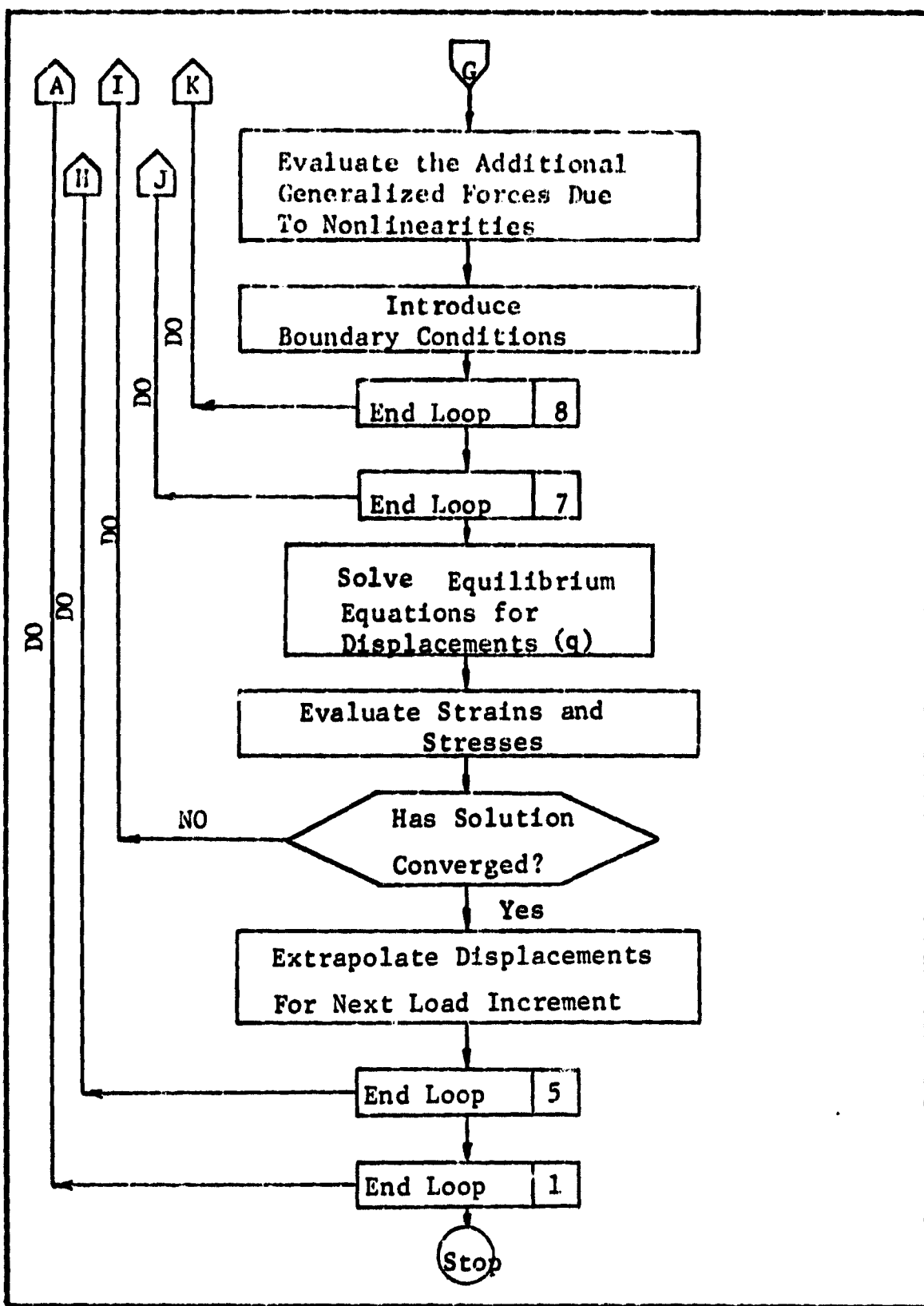


Fig. 8 FLOW CHART FOR THE NONLINEAR PROGRAM (cont.)

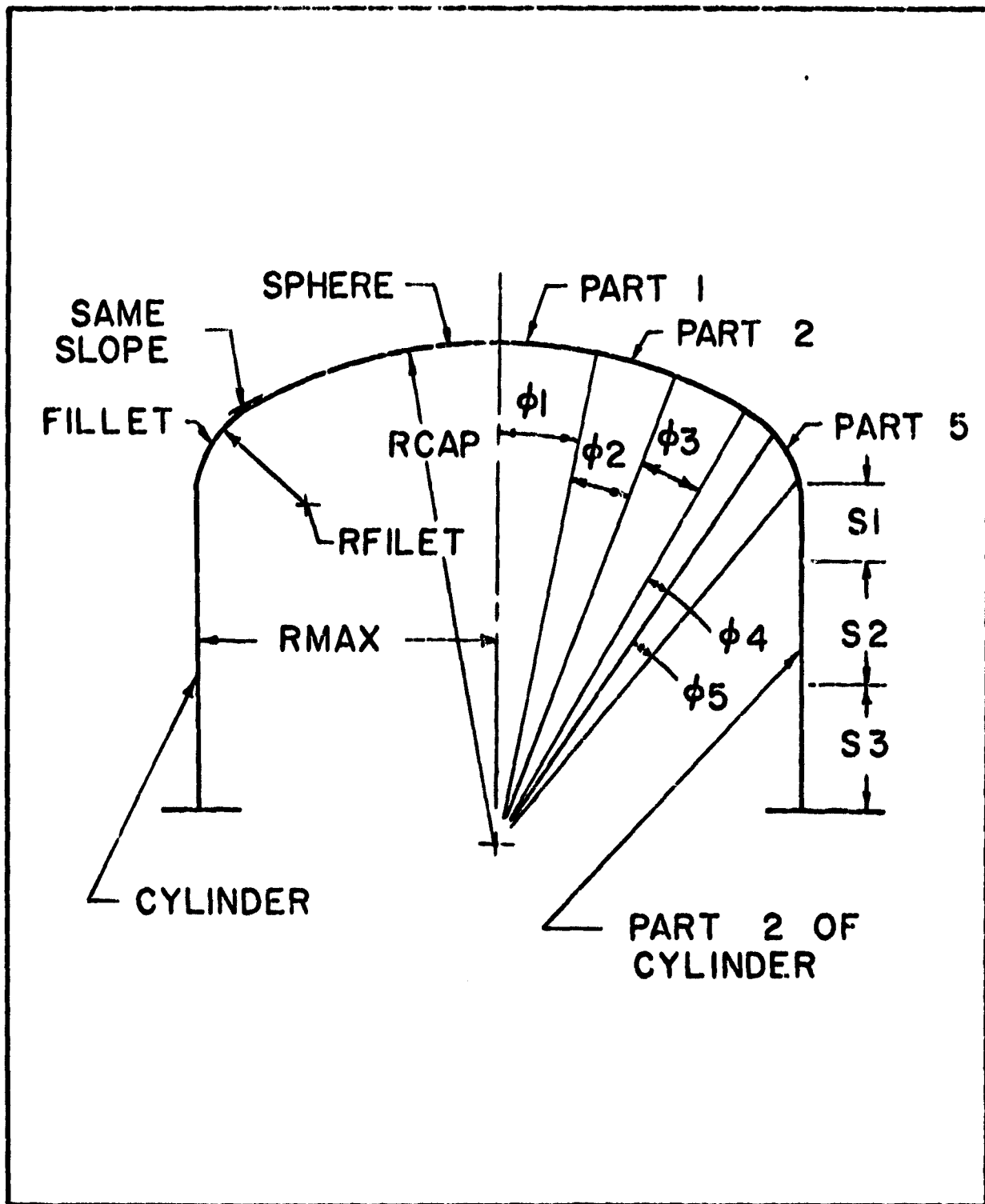


FIG. 9 SHELL IS A COMBINATION OF A CYLINDER, FILLET AND SPHERE

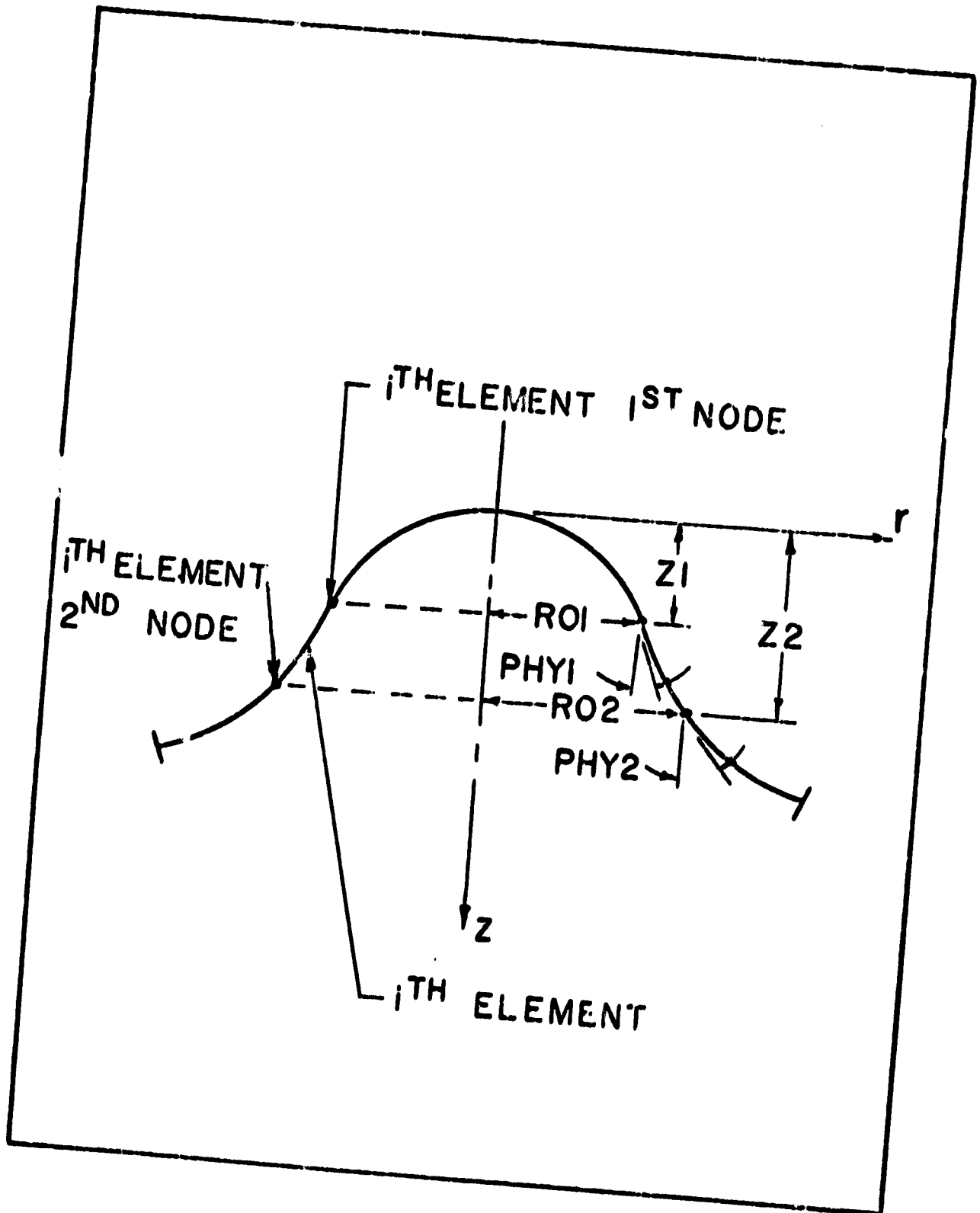


FIG. 10 SHELL WITH GENERAL GEOMETRY

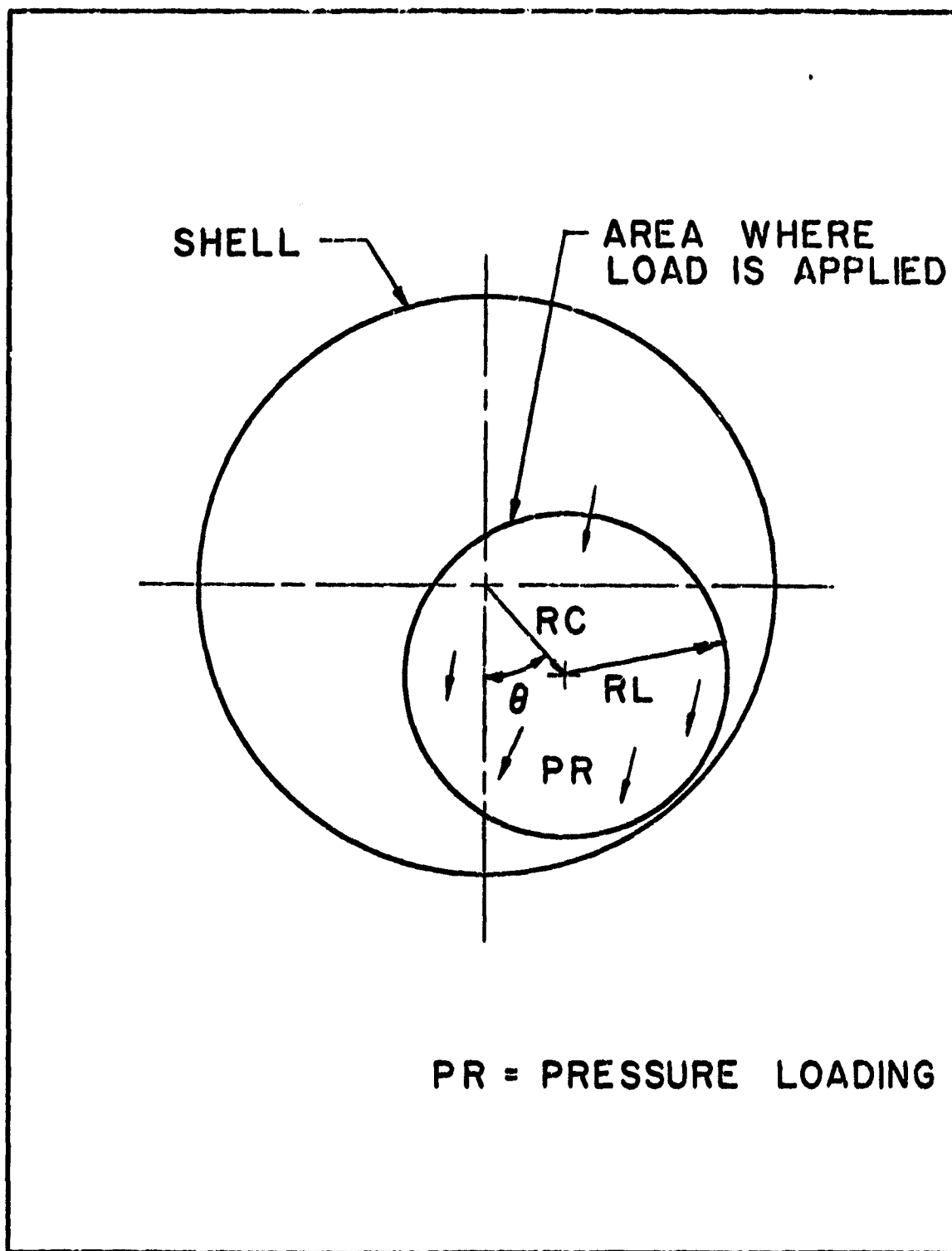


FIG. II LOAD APPLIED OVER A CIRCULAR AREA

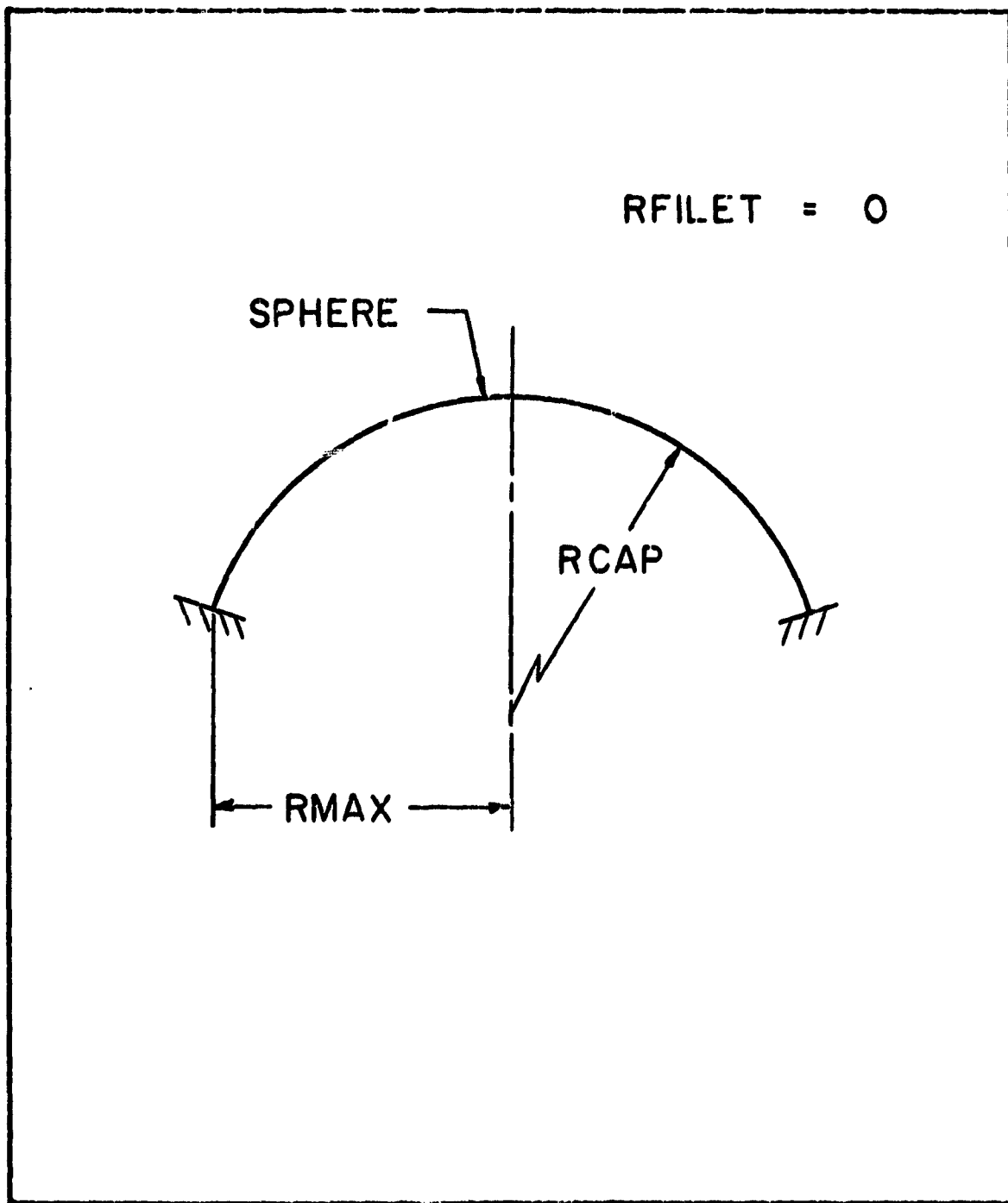


FIG. 12 A SPHERICAL CAP

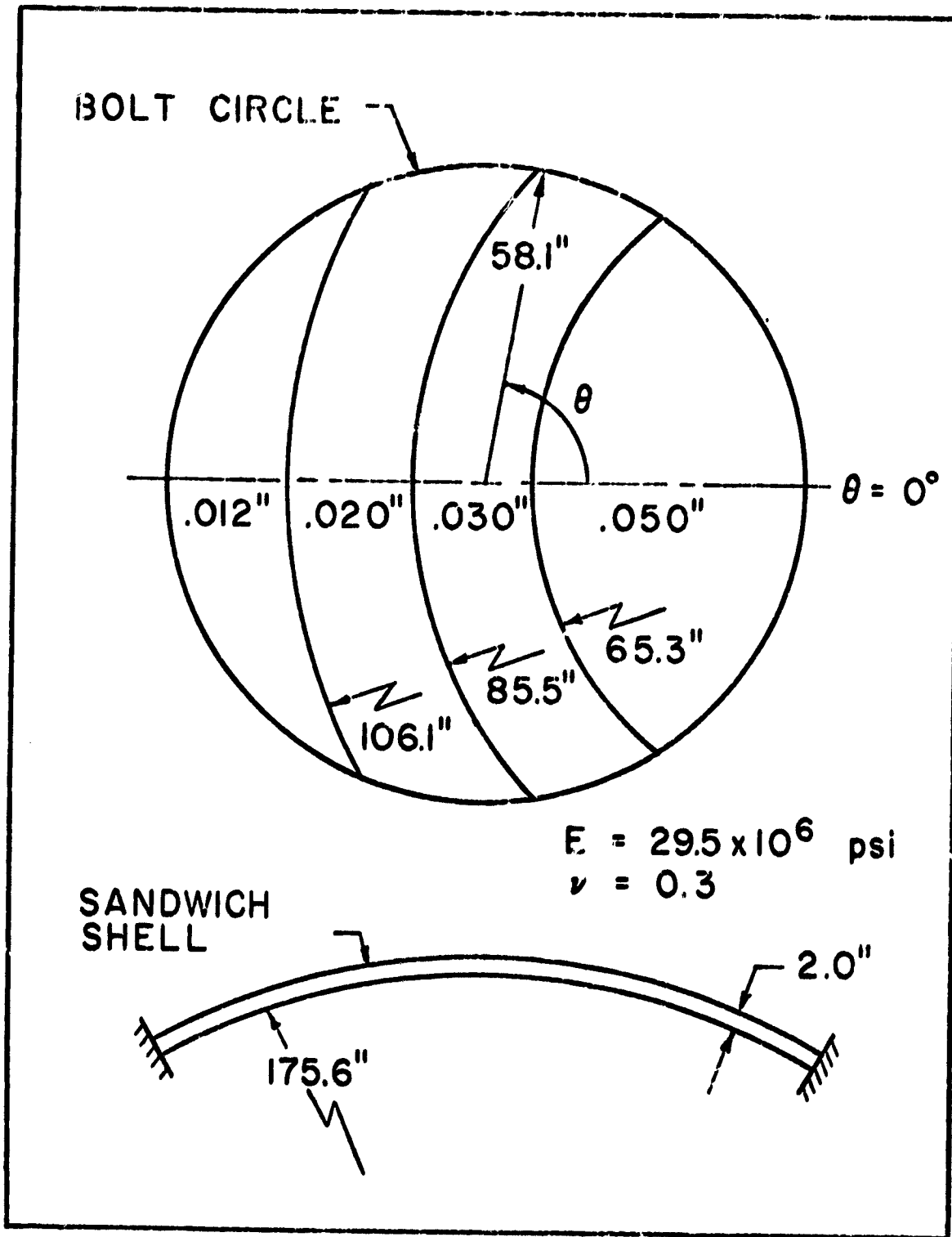


FIG. 13 SCALLOPED APOLLO
AFT HEAT SHIELD

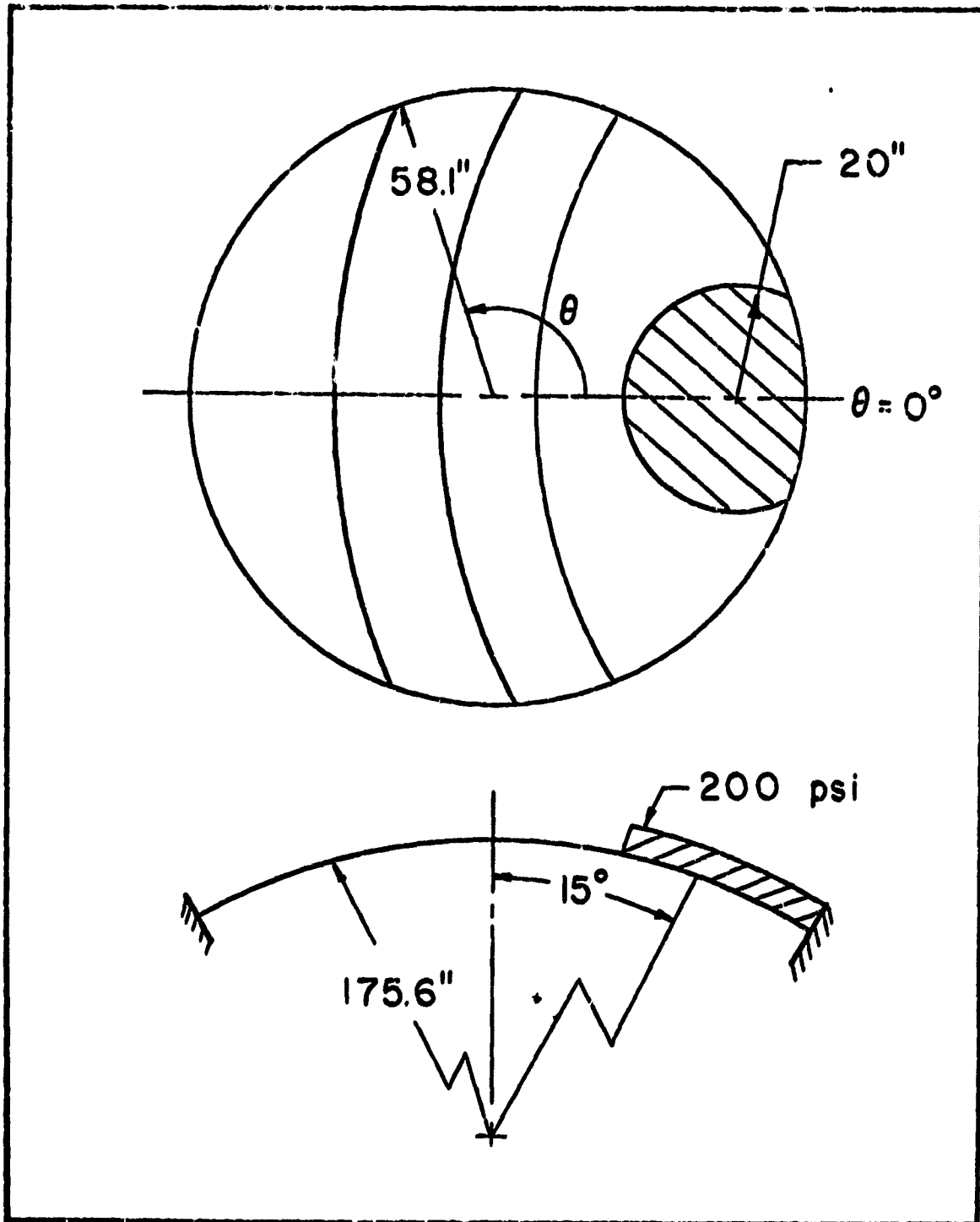


FIG. 14 APOLLO AFT HEAT SHIELD
UNDER A UNIFORM PRESSURE APPLIED
OVER A CIRCULAR AREA

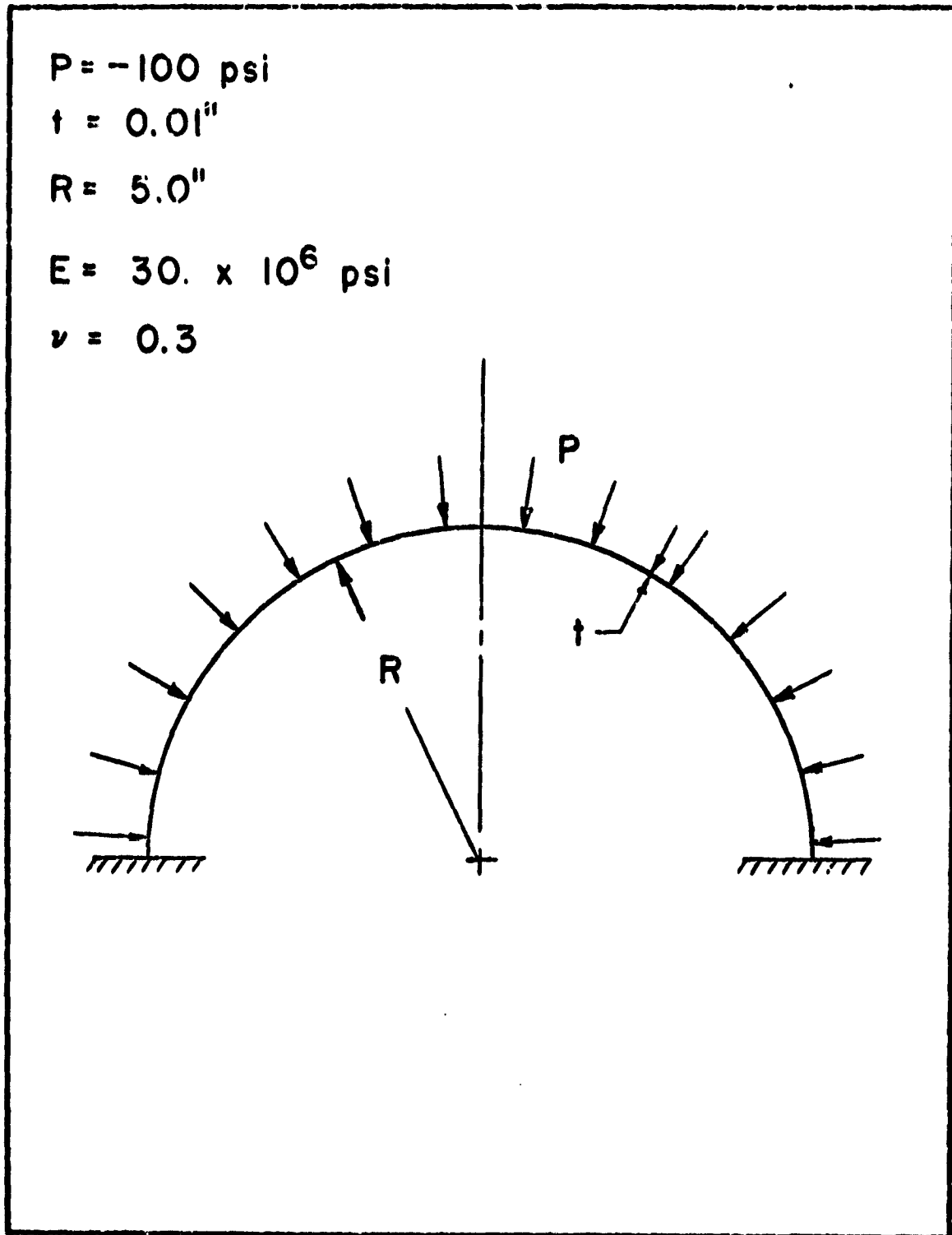


FIG. 15 HEMISPHERICAL SHELL
UNDER UNIFORM PRESSURE

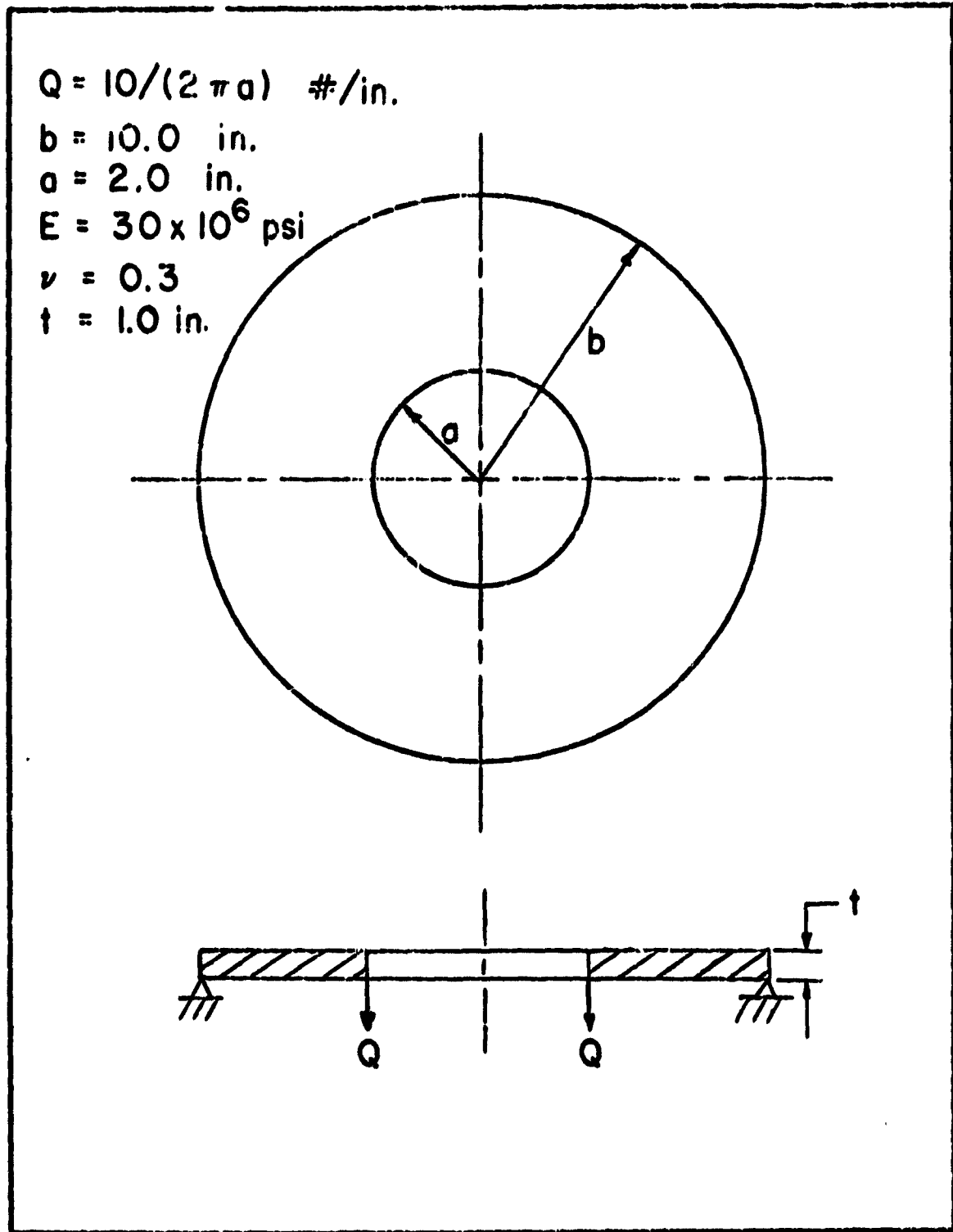


FIG. 16 CIRCULAR PLATE WITH A CIRCULAR HOLE UNDER A UNIFORM RING LOAD

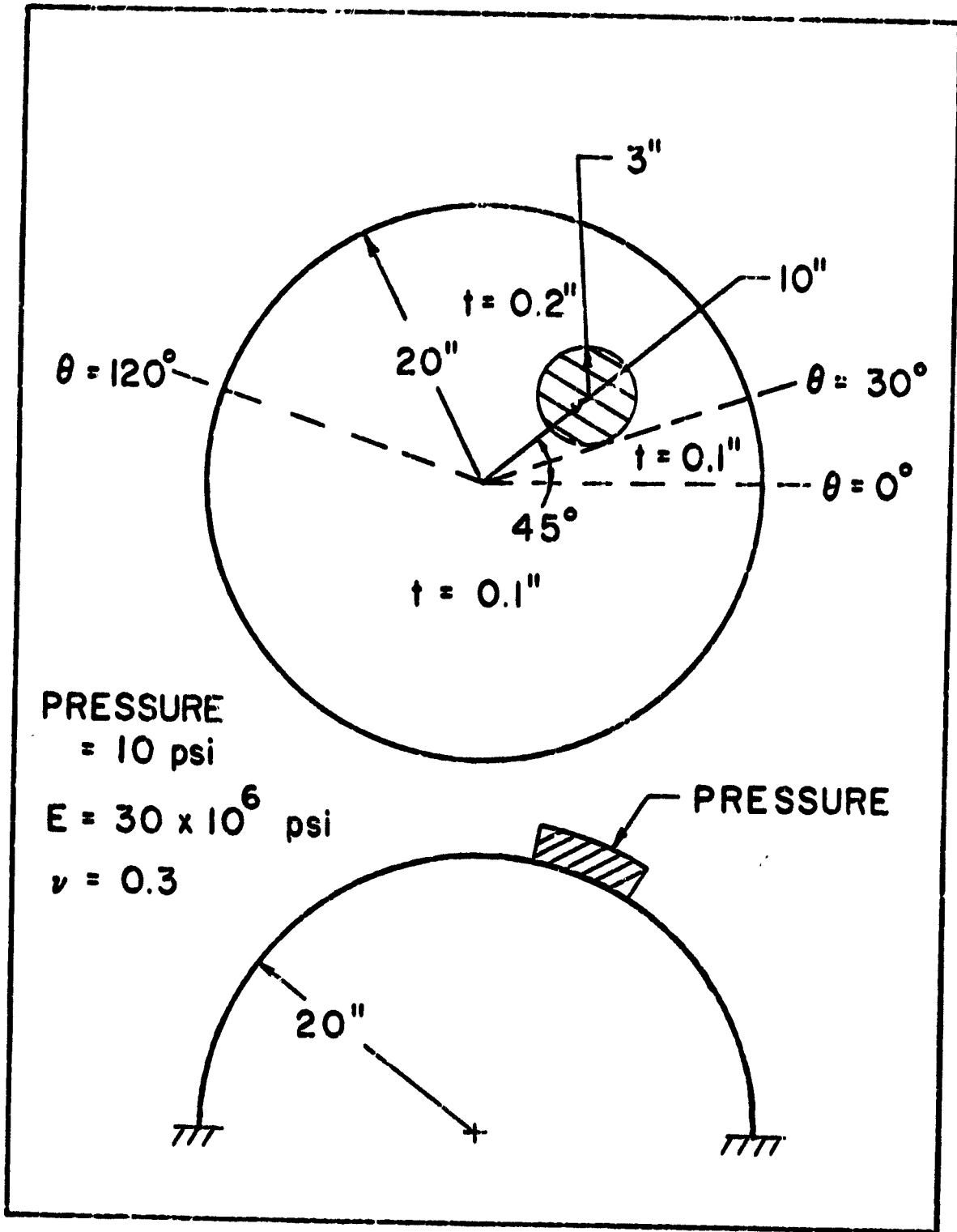


FIG. 17 A SPHERICAL CAP WITH VARYING THICKNESS PROPERTIES

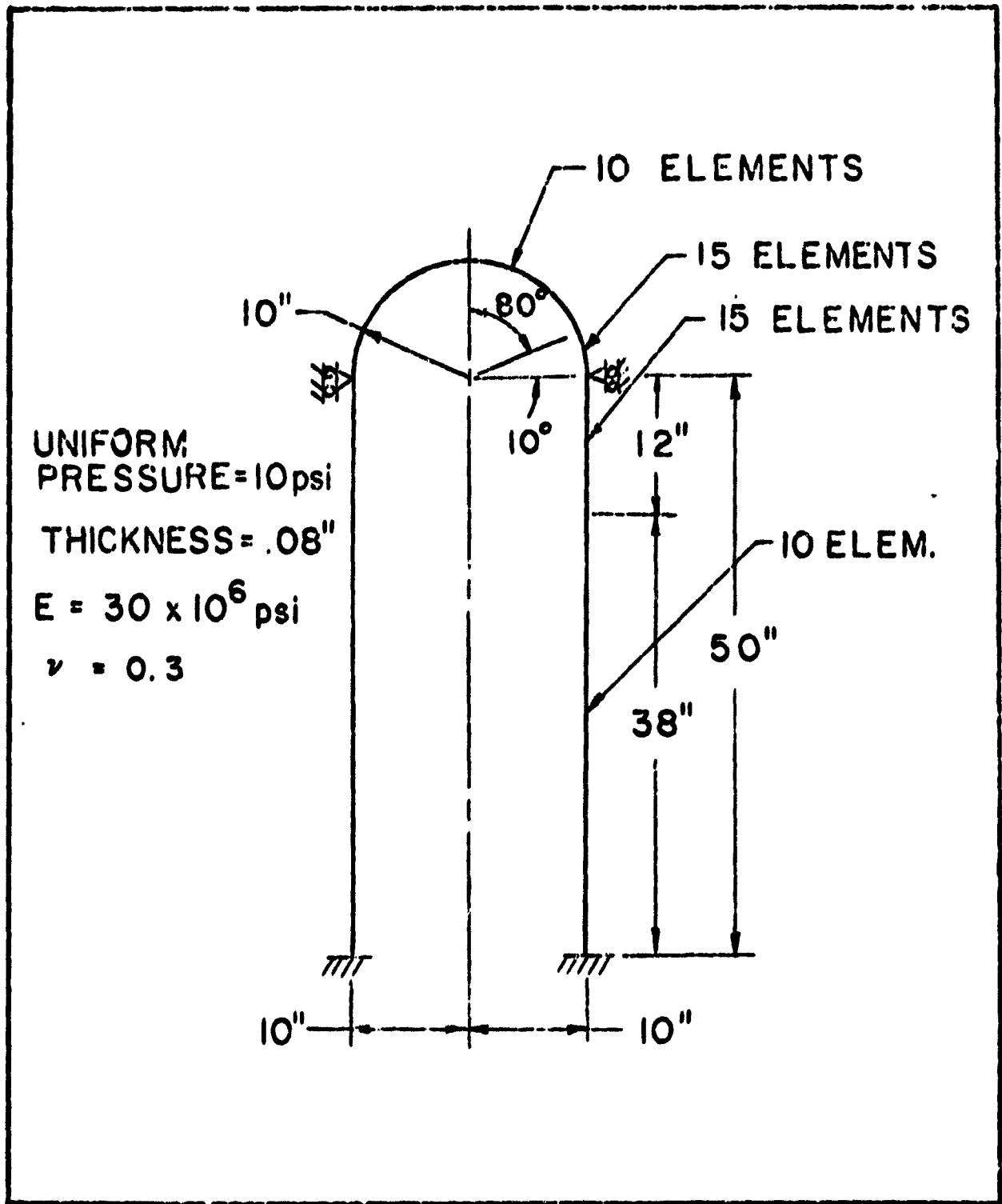


FIG. 18 SPHERICAL CAP AND CYLINDER UNDER A UNIFORM EXTERNAL PRESSURE

APPENDIX A
SAMPLE INPUT DATA AND SAMPLE OUTPUT

PAGE 1

GENERAL INFORMATION

SAMPLE PROBLEM NO. 1

NONLINEAR ANALYSIS OF THE SCALLOPED APOLLO AFT HEAT SHIELD.

THE SHELL IS A STAINLESS SANDWICH STRUCTURE WITH A NOMINAL

THICKNESS OF TWO INCHES.

THE LOAD IS APPLIED OVER A CIRCULAR AREA OF TWENTY INCHES RADIUS.

THE CENTER OF THE CIRCULAR AREA IS LOCATED AT 6.0 DEGREES IN THE

CIRCUMFERENTIAL DIRECTION AND AT 15 DEGREES INCLINATION.

FINE BUCKLING LOAD FOR THE APOLLO AFT HEAT SHIELD

NUMBER OF LOADING INCREMENTS = 4 000 TOTAL LOAD = 200 PSI 000 ERROR = 6 PERCENT

4 4 HARMONICS WERE USED WITH AN = 6 000 NET = 29

P R C B L E P DUCK

N O M E N C L A T U R E

Z ***** SHELL AXIAL COORDINATE
R ***** SHELL RADIAL COORDINATE(CYLINDRICAL)
E ***** MODULUS OF ELASTICITY OF SHELL ELEMENT
NU ***** POISSON'S RATIO OF SHELL ELEMENT
PHI ***** ANGLE BETWEEN MERIDIAN AND AXIS OF REVOLUTION
E(S) ***** STRAIN OF THE MIDDLE SURFACE OF THE SHELL ELEMENT ALONG THE MERIDIONAL DIRECTION
E(THETA) ***** STRAIN OF THE MIDDLE SURFACE OF THE SHELL ELEMENT ALONG THE CIRCUMFERENTIAL DIRECTION
E(S-THETA) ***** STRAIN OF THE MIDDLE SURFACE OF THE SHELL ELEMENT ALONG THE NORMAL DIRECTION
K(S) ***** CURVATURE CHANGE OF THE SHELL ELEMENT SURFACE IN MERIDIONAL PLANE
K(THETA) ***** CURVATURE CHANGE OF THE SHELL ELEMENT SURFACE IN PERIDICAL PLANE
K(S-THETA) ***** TWIST OF THE SHELL ELEMENT SURFACE
N(S) ***** NORMAL FORCE PER UNIT LENGTH OF CIRCUMFERENTIAL SECTION OF A SHELL
N(THETA) ***** NORMAL FORCE PER UNIT LENGTH OF MERIDIONAL SECTION OF A SHELL
N(S-THETA) ***** SHEARING FORCE PER UNIT LENGTH OF CIRCUMFERENTIAL SECTION OR MERIDIONAL SECTION OF A SHELL
M(S) ***** BENDING MOMENT PER UNIT LENGTH OF CIRCUMFERENTIAL SECTION OF A SHELL
M(THETA) ***** BENDING MOMENT PER UNIT LENGTH OF MERIDIONAL SECTION OF A SHELL
M(S-THETA) ***** TWISTING MOMENT PER UNIT LENGTH OF CIRCUMFERENTIAL SECTION OR MERIDIONAL SECTION OF A SHELL
Q(S) ***** SHEARING FORCE NORMAL TO THE SHELL PER UNIT LENGTH OF CIRCUMFERENTIAL SECTION
Q(THETA) ***** SHEARING FORCE NORMAL TO THE SHELL PER UNIT LENGTH OF MERIDIONAL SECTION

ELASTIC AND GEOMETRIC PROPERTIES OF STRUCTURE

ELEMENT NO.	NODE NOS.	COORDINATES			ELASTIC CONSTANTS			PH-I
		Z	R	A	E	NU	NU	
1	C-0	0.10000E-05			0.29500E 0E	C.30000E CC		0.90000E C2
2	0.83923E-02	G.17164E 01			0.29500E 0E	C.30000E CC		0.89440E C2
3	0.83923E-02	G.17164E 01			0.29500E 0E	C.30000E CC		C.89440F C2
4	0.33554E-01	0.34325E 01			0.29500E 0E	C.30000E CC		C.88800E C2
5	0.33554E-01	0.34325E 01			0.29500E 0E	C.30000E CC		0.88800E C2
6	C.75485E-01	0.51482E 01			0.29500E 0E	C.30000E CC		0.88320E C2
7	C.75485E-01	0.51482E 01			0.29500E 0E	C.30000E CC		C.88320E C2
8	C.13419E 00	C.68635E 01			0.29500E 0E	C.30000E CC		0.87760E C2
9	0.13419E 00	C.68635E 01			0.29500E 0E	C.30000E CC		0.87760E C2
10	0.20964E 00	0.85781E 01			0.29500E 0E	C.30000E CC		0.87200E C2
11	0.20964E 00	0.85781E 01			0.29500E 0E	C.30000E CC		C.87200E C2
12	C.30186E 00	C.10292E 02			0.29500E 0E	C.30000E CC		C.86640E C2
13	C.30186E 00	C.10292E 02			0.29500E 0E	C.30000E CC		C.86640E C2
14	0.41083E 00	C.12005E 02			0.29500E 0E	C.30000E CC		0.86080E C2
15	C.41083E 00	C.12005E 02			0.29500E 0E	C.30000E CC		C.86080E C2
16	0.53653E 00	C.13716E 02			0.29500E 0E	C.30000E CC		0.85520E C2
17	0.53653E 00	C.13716E 02			0.29500E 0E	C.30000E CC		C.85520E C2
18	0.67896E 00	C.15427E 02			0.29500E 0E	C.30000E CC		C.84960E C2
19	0.67896E 00	C.15427E 02			0.29500E 0E	C.30000E CC		C.84960E C2
20	0.83807E 00	C.17136E 02			0.29500E 0E	C.30000E CC		0.84400E C2
21	0.83807E 00	C.17136E 02			0.29500E 0E	C.30000E CC		C.84400E C2
22	0.10139E 01	C.18043F 02			0.29500E 0E	C.30000E CC		C.83840E C2
23	0.10139E 01	C.18043F 02			0.29500E 0E	C.30000E CC		C.83840E C2
24	0.10139E 01	C.18043E 02			0.29500E 0E	C.30000E CC		C.83280E C2
25	0.10139E 01	C.18043E 02			0.29500E 0E	C.30000E CC		C.83280E C2
26	0.12064E 01	C.20548E 02			0.29500E 0E	C.30000E CC		0.82720E C2
27	0.12064E 01	C.20548E 02			0.29500E 0E	C.30000E CC		0.82720E C2
28	0.14156E 01	C.22252E 02			0.29500E 0E	C.30000E CC		C.82160E C2
29	0.14156E 01	C.22252E 02			0.29500E 0E	C.30000E CC		C.82160E C2
30	0.16414E 01	C.23953E 02			0.29500E 0E	C.30000E CC		0.81600E C2
31	0.16414E 01	C.23953E 02			0.29500E 0E	C.30000E CC		0.81600E C2
32	0.16414E 01	0.25652E 02			0.29500E 0E	C.30000E CC		0.81040E C2
33	0.16414E 01	0.25652E 02			0.29500E 0E	C.30000E CC		0.81040E C2
34	0.18038E 01	C.27349E 02			0.29500E 0E	C.30000E CC		C.80480E C2
35	0.18038E 01	C.27349E 02			0.29500E 0E	C.30000E CC		C.80480E C2
36	0.21428E 01	C.29043E 02			0.29500E 0E	C.30000E CC		C.79920E C2
37	0.21428E 01	C.29043E 02			0.29500E 0E	C.30000E CC		C.79920E C2
38	C.24184E 01	0.30734E 02			0.29500E 0E	C.30000E CC		0.79360E C2
39	C.24184E 01	0.30734E 02			0.29500E 0E	C.30000E CC		0.79360E C2
40	0.27105E 01	C.32422E 02			0.29500E 0E	C.30000E CC		C.78800E C2
41	0.27105E 01	C.32422E 02			0.29500E 0E	C.30000E CC		C.78800E C2
42	0.30192E 01	0.34108E 02			0.29500E 0E	C.30000E CC		C.78475E C2
43	0.30192E 01	0.34108E 02			0.29500E 0E	C.30000E CC		C.78475E C2
44	0.33443E 01	C.35083E 02			0.29500E 0E	C.30000E CC		0.78151E C2
45	0.33443E 01	C.35083E 02			0.29500E 0E	C.30000E CC		0.78151E C2
46	0.35403E 01	C.36058E 02			0.29500E 0E	C.30000E CC		0.77826E C2
47	0.35403E 01	C.36058E 02			0.29500E 0E	C.30000E CC		0.77826E C2
48	0.37419E 01	C.37031E 02			0.29500E 0E	C.30000E CC		C.77501E C2
49	0.37419E 01	C.37031E 02			0.29500E 0E	C.30000E CC		C.77501E C2
50	0.39490E 01	0.38003E 02			0.29500E 0E	C.30000E CC		
51	0.39490E 01	0.38003E 02			0.29500E 0E	C.30000E CC		
52	0.41616E 01				0.29500E 0E	C.30000E CC		
53	0.41616E 01				0.29500E 0E	C.30000E CC		

ELASTIC AND GEOMETRIC PROPERTIES OF STRUCTURE

ELEMENT NO.	MODE NOS.	COORDINATES			ELASTIC CONSTANTS			P
		Z	R	N	E	NU	N	
25		0.41616E 01	0.38003E 02	0.25500E 0E	0.30000E 00	0.77501E 02		
26		0.43797E 01	0.38974E 02	0.25500E 0E	0.30000E 00	0.77177E 02		
27		0.46033E 01	0.39943E 02	0.25500E 0E	0.30000E 00	0.76852E 02		
28		0.48323E 01	0.40912E 02	0.25500E 0E	0.30000E 00	0.76527E 02		
29		0.50669E 01	0.41879E 02	0.25500E 0E	0.30000E 00	0.76203E 02		
30		0.53069E 01	0.42844E 02	0.25500E 0E	0.30000E 00	0.75878E 02		
31		0.55524E 01	0.43809E 02	0.25500E 0E	0.30000E 00	0.75553E 02		
32		0.58034E 01	0.44771E 02	0.25500E 0E	0.30000E 00	0.75228E 02		
33		0.60598E 01	0.45733E 02	0.25500E 0E	0.30000E 00	0.74904E 02		
34		0.63217E 01	0.46693E 02	0.25500E 0E	0.30000E 00	0.74579E 02		
35		0.65890E 01	0.47651E 02	0.25500E 0E	0.30000E 00	0.74255E 02		
36		0.68617E 01	0.48608E 02	0.25500E 0E	0.30000E 00	0.73930E 02		
37		0.70479E 01	0.49250E 02	0.25500E 0E	0.30000E 00	0.73712E 02		
38		0.72365E 01	0.49891E 02	0.25500E 0E	0.30000E 00	0.73454E 02		
39		0.74276E 01	0.50531E 02	0.25500E 0E	0.30000E 00	0.73276E 02		
40		0.76210E 01	0.51171E 02	0.25500E 0E	0.30000E 00	0.73058E 02		
41		0.78170E 01	0.51809E 02	0.25500E 0E	0.30000E 00	0.72840E 02		
42		0.80153E 01	0.52447E 02	0.25500E 0E	0.30000E 00	0.72622E 02		
43		0.82161E 01	0.53085E 02	0.25500E 0E	0.30000E 00	0.72404E 02		
44		0.84193E 01	0.53721E 02	0.25500E 0E	0.30000E 00	0.72186E 02		
45		0.86248E 01	0.54357E 02	0.25500E 0E	0.30000E 00	0.71968E 02		
46		0.88329E 01	0.54992E 02	0.25500E 0E	0.30000E 00	0.71750E 02		
47		0.90434E 01	0.55626E 02	0.25500E 0E	0.30000E 00	0.71532E 02		
48		0.92562E 01	0.56259E 02	0.25500E 0E	0.30000E 00	0.71314E 02		
49		0.94714E 01	0.56892E 02	0.25500E 0E	0.30000E 00	0.71096E 02		

ELASTIC AND GEOMETRIC PROPERTIES OF STRUCTURE

ELEMENT NO.	NODE NOS.	COORDINATES			ELASTIC CONSTANTS			PHI
		Z	R		E	NU		
49	49	0.94714E 01	0.56892E 02	0.29500E 0E	0.30000E 00	0.71056E 02		
50	50	0.96891E 01	0.57523E 02	0.29500E 0E	0.30000E 00	0.70878E 02		
50	50	0.96891E 01	0.57523E 02	0.29500E 0E	0.30000E 00	0.70878E 02		
51	51	0.98902E 01	0.58100E 02	0.29500E 0E	0.30000E 00	0.70660E 02		

NUMBER OF A HARMONICS = 4 NUMBER OF SIMPSON STATIONS = 25 NUMBER OF B HARMONICS = C
RADIUS OF SPHERICAL SHELL = 175.6CC

TABLE OF THICKNESSES
 SANDWICH TYPE SHELL WITH FACE SHEETS SPACED 2.000 IN FROM EACH OTHER
 THICKNESSES OF EACH FACE SHEET ARE GIVEN BELOW

ELEMENT NO. 1	CONSTANT THICKNESS =	0.03000
ELEMENT NO. 2	CONSTANT THICKNESS =	0.03000
ELEMENT NO. 3	CONSTANT THICKNESS =	0.03000
ELEMENT NO. 4	CONSTANT THICKNESS =	0.03000
ELEMENT NO. 5	CONSTANT THICKNESS =	0.03000
ELEMENT NO. 6	CONSTANT THICKNESS =	0.03000
ELEMENT NO. 7		
THETA (DEGREES)	0.0	TC 30.0000
THICKNESS	0.05000	30.0000 TO 145.0000 0.03000
THETA (DEGREES)	330.0000	TC 360.0000
THICKNESS	0.05000	360.0000 TO 215.0000 0.03000
ELEMENT NO. 8		
THETA (DEGREES)	0.0	TC 40.0000
THICKNESS	0.05000	40.0000 TO 145.0000 0.03000
THETA (DEGREES)	320.0000	TC 360.0000
THICKNESS	0.05000	360.0000 TO 215.0000 0.03000
ELEMENT NO. 9		
THETA (DEGREES)	0.0	TC 40.0000
THICKNESS	0.05000	40.0000 TO 127.5000 0.03000
THETA (DEGREES)	232.5000	TC 320.0000
THICKNESS	0.05000	320.0000 TO 232.5000 0.03000

TABLE OF THICKNESSES
 SANDWICH TYPE SHELL WITH FACE SHEETS SPACED 2.00' IN FROM EACH OTHER
 THICKNESSES OF EACH FACE SHEET ARE GIVEN BELOW

		ELEMENT NO. 9		ELEMENT NO. 10		ELEMENT NO. 11		ELEMENT NO. 12		ELEMENT NO. 13		ELEMENT NO. 14	
THETA(DEGREES)	320.0000	TC	360.0000	50.0000	TC	127.5000	50.0000	TC	115.0000	50.0000	TC	105.0000	60.0000
THICKNESS			0.0500			0.0300			0.0300			0.0300	
THETA(DEGREES)	0.0	TC	50.0000	50.0000	TC	127.5000	50.0000	TC	115.0000	50.0000	TC	105.0000	60.0000
THICKNESS			0.0500			0.0300			0.0300			0.0300	
THETA(DEGREES)	310.0000	TC	360.0000	310.0000	TC	360.0000	310.0000	TC	360.0000	310.0000	TC	360.0000	310.0000
THICKNESS			0.0500			0.0500			0.0500			0.0500	
THETA(DEGREES)	0.0	TC	50.0000	50.0000	TC	50.0000	50.0000	TC	50.0000	50.0000	TC	60.0000	60.0000
THICKNESS			0.0500			0.0300			0.0300			0.0300	
THETA(DEGREES)	310.0000	TC	360.0000	310.0000	TC	360.0000	310.0000	TC	360.0000	310.0000	TC	360.0000	310.0000
THICKNESS			0.0500			0.0500			0.0500			0.0500	
THETA(DEGREES)	0.0	TC	60.0000	60.0000	TC	60.0000	60.0000	TC	60.0000	60.0000	TC	60.0000	60.0000
THICKNESS			0.0500			0.0300			0.0300			0.0300	
THETA(DEGREES)	300.0000	TC	360.0000	300.0000	TC	360.0000	300.0000	TC	360.0000	300.0000	TC	360.0000	300.0000
THICKNESS			0.0500			0.0500			0.0500			0.0500	
THETA(DEGREES)	0.0	TC	60.0000	60.0000	TC	60.0000	60.0000	TC	60.0000	60.0000	TC	60.0000	60.0000
THICKNESS			0.0500			0.0300			0.0300			0.0300	

TABLE OF THICKNESSES
 SANDWICH TYPE SHELL WITH FACE SHEETS SPACED 2.000 IN FROM EACH OTHER
 THICKNESSES OF EACH FACE SHEET ARE GIVEN BELOW

		ELEMENT NO. 19			
THETA(DEGREES)	205.0000	TC	265.0000	TC	360.0000
THICKNESS	0.0200		0.0300		0.0500
			ELEMENT NO. 20		
THETA(DEGREES)	0.0	TC	60.0000	TC	155.0000
THICKNESS	0.0500		0.0300		0.0200
THETA(DEGREES)	205.0000	TC	265.0000	TC	360.0000
THICKNESS	0.0200		0.0300		0.0500
			ELEMENT NO. 21		
THETA(DEGREES)	0.0	TC	60.0000	TC	135.0000
THICKNESS	0.0500		0.0300		0.0200
THETA(DEGREES)	225.0000	TC	265.0000	TC	360.0000
THICKNESS	0.0200		0.0300		0.0500
			ELEMENT NO. 22		
THETA(DEGREES)	0.0	TC	60.0000	TC	135.0000
THICKNESS	0.0500		0.0300		0.0200
THETA(DEGREES)	225.0000	TC	265.0000	TC	360.0000
THICKNESS	0.0200		0.0300		0.0500
			ELEMENT NO. 23		
THETA(DEGREES)	0.0	TC	60.0000	TC	135.0000
THICKNESS	0.0500		0.0300		0.0200
THETA(DEGREES)	225.0000	TC	265.0000	TC	360.0000
THICKNESS	0.0200		0.0300		0.0500
			ELEMENT NO. 24		
THETA(DEGREES)	0.0	TC	60.0000	TC	135.0000
THICKNESS	0.0500		0.0300		0.0200
THETA(DEGREES)	225.0000	TC	265.0000	TC	360.0000
THICKNESS	0.0200		0.0300		0.0500

TABLE OF THICKNESSES
 SANDWICH TYPE SHELL WITH FACE SHEETS SPACE 2.000 IN FROM EACH OTHER
 THICKNESSES OF EACH FACE SHEET ARE GIVEN BELOW

		ELEMENT NO. 29				ELEMENT NO. 30				ELEMENT NO. 31				ELEMENT NO. 32				ELEMENT NO. 33				ELEMENT NO. 34	
THETA(DEGREES)		232.5000	TC	270.0000	TC	270.0000	TC	300.0000	TC	300.0000	TC	360.0000	TC	360.0000	TC	127.5000	TC	127.5000	TC	245.0000	TC	245.0000	TC
THICKNESS		0.0200		0.0300		0.0300		0.0300		0.0300		0.0300		0.0300		0.0200		0.0200		0.0300		0.0300	
THETA(DEGREES)		0.0	TC	60.0000	TC	60.0000	TC	90.0000	TC	90.0000	TC	127.5000	TC	127.5000	TC	0.0	TC	60.0000	TC	85.0000	TC	85.0000	TC
THICKNESS		0.0	0.0500	0.0500	0.0300	0.0300	0.0300	0.0300	0.0300	0.0300	0.0300	0.0300	0.0300	0.0300	0.0300	0.0500	0.0500	0.0500	0.0500	0.0300	0.0300	0.0300	0.0300
THETA(DEGREES)		232.5000	TC	270.0000	TC	270.0000	TC	300.0000	TC	300.0000	TC	360.0000	TC	360.0000	TC	0.0	TC	60.0000	TC	85.0000	TC	85.0000	TC
THICKNESS		0.0200		0.0300		0.0300		0.0300		0.0300		0.0300		0.0300		0.0200		0.0200		0.0300		0.0300	
THETA(DEGREES)		0.0	TC	60.0000	TC	60.0000	TC	90.0000	TC	90.0000	TC	115.0000	TC	115.0000	TC	0.0	TC	60.0000	TC	85.0000	TC	85.0000	TC
THICKNESS		0.0	0.0500	0.0500	0.0300	0.0300	0.0300	0.0300	0.0300	0.0300	0.0300	0.0300	0.0300	0.0300	0.0300	0.0500	0.0500	0.0500	0.0500	0.0300	0.0300	0.0300	0.0300
THETA(DEGREES)		245.0000	TC	270.0000	TC	270.0000	TC	300.0000	TC	300.0000	TC	360.0000	TC	360.0000	TC	0.0	TC	60.0000	TC	85.0000	TC	85.0000	TC
THICKNESS		0.0200		0.0300		0.0300		0.0300		0.0300		0.0300		0.0300		0.0200		0.0200		0.0300		0.0300	
THETA(DEGREES)		0.0	TC	60.0000	TC	60.0000	TC	90.0000	TC	90.0000	TC	115.0000	TC	115.0000	TC	0.0	TC	60.0000	TC	85.0000	TC	85.0000	TC
THICKNESS		0.0	0.0500	0.0500	0.0300	0.0300	0.0300	0.0300	0.0300	0.0300	0.0300	0.0300	0.0300	0.0300	0.0300	0.0500	0.0500	0.0500	0.0500	0.0300	0.0300	0.0300	0.0300
THETA(DEGREES)		245.0000	TC	270.0000	TC	270.0000	TC	300.0000	TC	300.0000	TC	360.0000	TC	360.0000	TC	0.0	TC	60.0000	TC	85.0000	TC	85.0000	TC
THICKNESS		0.0200		0.0300		0.0300		0.0300		0.0300		0.0300		0.0300		0.0200		0.0200		0.0300		0.0300	
THETA(DEGREES)		0.0	TC	60.0000	TC	60.0000	TC	85.0000	TC	85.0000	TC	115.0000	TC	115.0000	TC	0.0	TC	60.0000	TC	85.0000	TC	85.0000	TC
THICKNESS		0.0	0.0500	0.0500	0.0300	0.0300	0.0300	0.0300	0.0300	0.0300	0.0300	0.0300	0.0300	0.0300	0.0300	0.0500	0.0500	0.0500	0.0500	0.0300	0.0300	0.0300	0.0300
THETA(DEGREES)		0.0	TC	60.0000	TC	60.0000	TC	85.0000	TC	85.0000	TC	115.0000	TC	115.0000	TC	0.0	TC	60.0000	TC	85.0000	TC	85.0000	TC
THICKNESS		0.0	0.0500	0.0500	0.0300	0.0300	0.0300	0.0300	0.0300	0.0300	0.0300	0.0300	0.0300	0.0300	0.0300	0.0500	0.0500	0.0500	0.0500	0.0300	0.0300	0.0300	0.0300
THETA(DEGREES)		0.0	TC	60.0000	TC	60.0000	TC	85.0000	TC	85.0000	TC	115.0000	TC	115.0000	TC	0.0	TC	60.0000	TC	85.0000	TC	85.0000	TC
THICKNESS		0.0	0.0500	0.0500	0.0300	0.0300	0.0300	0.0300	0.0300	0.0300	0.0300	0.0300	0.0300	0.0300	0.0300	0.0500	0.0500	0.0500	0.0500	0.0300	0.0300	0.0300	0.0300

TABLE OF THICKNESSES
 SANDWICH TYPE SHELL WITH FACE SHEETS SPACED 2.000 IN FROM EACH OTHER
 THICKNESSES OF EACH FACE SHEET ARE GIVEN BELOW

ELEMENT NO. 34

THETA(DEGREES) THICKNESS	245.0000 TC 0.0200	275.0000 TC 0.0200	300.0000 TC 0.0300	360.0000 TC 0.0500	115.0000 TC 0.0120
THETA(DEGREES) THICKNESS	0.0 0.0500	60.0000 TC 0.0500	85.0000 TC 0.0300	115.0000 TC 0.0200	245.0000 TC 0.0120
THETA(DEGREES) THICKNESS	245.0000 TC 0.0200	275.0000 TC 0.0200	300.0000 TC 0.0300	360.0000 TC 0.0500	115.0000 TC 0.0120

ELEMENT NO. 35

THETA(DEGREES) THICKNESS	0.0 0.0500	60.0000 TC 0.0500	85.0000 TC 0.0300	110.0000 TC 0.0200	250.0000 TC 0.0120
THETA(DEGREES) THICKNESS	250.0000 TC 0.0200	275.0000 TC 0.0200	300.0000 TC 0.0300	360.0000 TC 0.0500	110.0000 TC 0.0120

ELEMENT NO. 36

THETA(DEGREES) THICKNESS	0.0 0.0500	60.0000 TC 0.0500	85.0000 TC 0.0300	110.0000 TC 0.0200	250.0000 TC 0.0120
THETA(DEGREES) THICKNESS	250.0000 TC 0.0200	275.0000 TC 0.0200	300.0000 TC 0.0300	360.0000 TC 0.0500	110.0000 TC 0.0120

ELEMENT NO. 37

THETA(DEGREES) THICKNESS	0.0 0.0500	60.0000 TC 0.0500	85.0000 TC 0.0300	110.0000 TC 0.0200	250.0000 TC 0.0120
THETA(DEGREES) THICKNESS	250.0000 TC 0.0200	275.0000 TC 0.0200	300.0000 TC 0.0300	360.0000 TC 0.0500	110.0000 TC 0.0120

TABLE OF THICKNESSES
 SANDWICH TYPE SHELL WITH FACE SHEETS SPACEE 2.000 IN FROM EACH OTHER
 THICKNESSES OF EACH FACE SHEET ARE GIVEN BELOW

		ELEMENT NO. 49		ELEMENT NO. 50	
THETA(DEGREES)	255.0000 TC	202.0000 TC	300.0000 TC	300.0000 TC	360.0000 TC
THICKNESS	0.02000	0.03000	0.05000	0.05000	0.05000
THETA(DEGREES)	0.0	60.0000 TC	70.0000 TC	70.0000 TC	105.0000 TC
THICKNESS	0.05000	0.03000	0.03000	0.02000	0.01200
THETA(DEGREES)	255.0000 TC	202.0000 TC	300.0000 TC	300.0000 TC	360.0000 TC
THICKNESS	0.02000	0.03000	0.05000	0.05000	0.05000

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BOUNDARY CONDITIONS

DISPLACEMENT COMPONENTS OF NODES

NODE NO.	AXIAL	TANGENTIAL	RADIAL	ANGULAR
51	FIXED	FIXED	FIXED	FIXED

ALL OTHER NODES ARE FREE TO DISPLACE

.....
.....
.....

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APPLIED LOADS ON THE STRUCTURE

MERIDIONAL		PRESSURE COMPONENTS		FROM THE TA TO TETA(DEGREES)	
		NORMAL	TANGENTIAL		
ELEMENT NO. 15					
0.C	-200.CCC	C.C	0.C	8.831	8.831
0.C	C.C	C.C	8.831	351.158	
0.C	-200.CCC	C.C	351.158	360.CCC	
ELEMENT NO. 16					
0.C	-200.CCC	C.C	0.C	10.576	10.576
0.C	C.C	C.C	10.576	345.424	
0.C	-200.CCC	C.C	345.424	360.CCC	
ELEMENT NO. 17					
0.C	-200.CCC	C.C	0.C	16.129	16.129
0.C	C.C	C.C	16.129	343.853	
0.C	-200.CCC	C.C	343.853	360.CCC	
ELEMENT NO. 18					
0.C	-200.CCC	C.C	0.C	19.538	19.538
0.C	C.C	C.C	19.538	340.442	
0.C	-200.CCC	C.C	340.442	360.CCC	
ELEMENT NO. 19					
0.C	-200.CCC	C.C	0.C	21.848	21.848
0.C	C.C	C.C	21.848	338.131	
0.C	-200.CCC	C.C	338.131	360.CCC	
ELEMENT NO. 20					
0.C	-200.CCC	C.C	0.C	23.419	23.419

APPLIED LOADS ON THE STRUCTURE

MERIDIONAL	PRESSURE COMPONENTS		TANGENTIAL	FROM THETA TO THETA(DEGREES)
	NORMAL			
	ELEMENT NO. 20			
0.C	C.C		C.C	23.419 336.544
0.C	-20C.CCC		C.C	336.544 36C.CCC
	ELEMENT NO. 21			
0.C	-20C.CCC		C.C	0.C 24.457
0.C	C.C		C.C	24.457 335.542
0.C	-20C.CCC		C.C	335.542 36C.CCC
	ELEMENT NO. 22			
0.C	-20C.CCC		C.C	0.C 24.852
0.C	C.C		C.C	24.852 335.C77
0.C	-20C.CCC		C.C	335.C77 36C.CCC
	ELEMENT NO. 23			
0.C	-20C.CCC		C.C	0.C 25.383
0.C	C.C		C.C	25.383 334.617
0.C	-20C.CCC		C.C	334.617 36C.CCC
	ELEMENT NO. 24			
0.C	-20C.CCC		C.C	0.C 25.612
0.C	C.C		C.C	25.612 334.359
0.C	-20C.CCC		C.C	334.359 360.CCC
	ELEMENT NO. 25			
0.C	-20C.CCC		C.C	0.C 25.830

APPLIED LOADS ON THE STRUCTURE

ELEMENT NO. 25		ELEMENT NO. 26		ELEMENT NO. 27		ELEMENT NO. 28		ELEMENT NO. 29		ELEMENT NO. 30	
PRESSURE COMPONENTS		PRESSURE COMPONENTS		PRESSURE COMPONENTS		PRESSURE COMPONENTS		PRESSURE COMPONENTS		PRESSURE COMPONENTS	
PERIODICAL	NORMAL	TANGENTIAL	PERIODICAL	NORMAL	TANGENTIAL	PERIODICAL	NORMAL	TANGENTIAL	PERIODICAL	NORMAL	TANGENTIAL
0.C	C.C	C.C	0.C	C.C	C.C	0.C	C.C	C.C	0.C	C.C	C.C
0.C	-200.CCC	C.C	0.C	C.C	C.C	0.C	C.C	C.C	0.C	C.C	C.C
0.C	-200.CCC	C.C	0.C	C.C	C.C	0.C	C.C	C.C	0.C	C.C	C.C
0.C	C.C	C.C	0.C	C.C	C.C	0.C	C.C	C.C	0.C	C.C	C.C
0.C	-200.CCC	C.C	0.C	C.C	C.C	0.C	C.C	C.C	0.C	C.C	C.C
0.C	-200.CCC	C.C	0.C	C.C	C.C	0.C	C.C	C.C	0.C	C.C	C.C
0.C	C.C	C.C	0.C	C.C	C.C	0.C	C.C	C.C	0.C	C.C	C.C
0.C	-200.CCC	C.C	0.C	C.C	C.C	0.C	C.C	C.C	0.C	C.C	C.C
0.C	-200.CCC	C.C	0.C	C.C	C.C	0.C	C.C	C.C	0.C	C.C	C.C

FROM THETA TO THETA(DEGREES)

25.030 334.141

324.141 360.CCC

0.C 26.C4C

26.C40 333.56C

333.560 360.CCC

0.C 26.C15

26.C15 333.552

333.952 360.CCC

0.C 26.C46

26.C46 333.935

333.935 360.CCC

0.C 25.950

25.950 334.C16

334.C16 360.CCC

0.C 25.031

APPLIED LOADS ON THE STRUCTURE

ELEMENT NO. 35		ELEMENT NO. 36		ELEMENT NO. 37		ELEMENT NO. 38		ELEMENT NO. 39		ELEMENT NO. 40	
PRESSURE COMPONENTS		PRESSURE COMPONENTS		PRESSURE COMPONENTS		PRESSURE COMPONENTS		PRESSURE COMPONENTS		PRESSURE COMPONENTS	
MERIDIONAL	NORMAL	TANGENTIAL	FROM THETA	TO THETA	(DEGREES)	MERIDIONAL	NORMAL	TANGENTIAL	FROM THETA	TO THETA	(DEGREES)
0.C	C.C	C.C	24.465	335.535		0.C	C.C	C.C	0.C	23.344	
0.C	-200.CCC	C.C	335.535	360.CCC		0.C	C.C	C.C	23.344	336.619	
0.C	-200.CCC	C.C				0.C	C.C	C.C	336.619	360.CCC	
0.C	C.C	C.C	24.107	335.853		0.C	C.C	C.C	0.C	23.C30	
0.C	-200.CCC	C.C	335.893	360.CCC		0.C	C.C	C.C	23.C30	336.947	
0.C	-200.CCC	C.C				0.C	C.C	C.C	336.947	360.CCC	
0.C	-200.CCC	C.C				0.C	C.C	C.C	0.C	22.653	
0.C	C.C	C.C	23.732			0.C	C.C	C.C			
0.C	-200.CCC	C.C	23.732	336.247		0.C	C.C	C.C			
0.C	-200.CCC	C.C	336.247	360.CCC		0.C	C.C	C.C			
0.C	-200.CCC	C.C				0.C	C.C	C.C			
0.C	C.C	C.C	23.344			0.C	C.C	C.C			
0.C	-200.CCC	C.C	336.619	360.CCC		0.C	C.C	C.C			
0.C	-200.CCC	C.C				0.C	C.C	C.C			
0.C	C.C	C.C	23.C30			0.C	C.C	C.C			
0.C	-200.CCC	C.C	336.947	360.CCC		0.C	C.C	C.C			
0.C	-200.CCC	C.C				0.C	C.C	C.C			

APPLIED LOADS ON THE STRUCTURE

MERIDIONAL	PRESSURE COMPONENTS		FROM THETA TO THETA(DEGREES)
	NORMAL	TANGENTIAL	
ELEMENT NO. 45			
0.0	0.0	C.0	20.460 339.529
0.0	-200.000	C.0	339.529 360.000
ELEMENT NO. 46			
0.0	-200.000	C.0	0.0 19.940
0.0	0.0	0.0	19.940 340.049
0.0	-200.000	C.0	340.049 360.000
ELEMENT NO. 47			
0.0	-200.000	C.0	0.0 19.333
0.0	0.0	C.0	19.333 340.643
0.0	-200.000	C.0	340.643 360.000
ELEMENT NO. 48			
0.0	-200.000	C.0	0.0 18.795
0.0	0.0	C.0	18.795 341.190
0.0	-200.000	C.0	341.190 360.000
ELEMENT NO. 49			
0.0	-200.000	C.0	0.0 18.135
0.0	0.0	C.0	18.135 341.841
0.0	-200.000	C.0	341.841 360.000
ELEMENT NO. 50			
0.0	-200.000	C.0	0.0 17.643

APPLIED LOADS ON THE STRUCTURE

ELEMENT NO. 50

PRESSURE COMPONENTS		TANGENTIAL		FROM THETA TO THETA(DEGREES)	
MERIDIONAL	NORMAL				
0.0	0.0		0.0	17.643	342.356
0.0	-200.000		0.0	342.356	360.000

DISPLACEMENT COMPONENTS OF MODES
 INCREMENT NO. 1 *** 25 PERCENT OF TOTAL LOAD
 I T E R A T I O N N O. 3
 THETA= 0.0 DEGREES

MODE NO.	AXIAL	TANGENTIAL	RADIAL	ANGULAR
1	0.16007554E-01	0.0	-0.12830410E-01	-0.26907007E-02
2	0.20776909E-01	0.0	-0.12989905E-01	-0.2839996E-02
3	0.25905017E-01	0.0	-0.13382431E-01	-0.30971738E-02
4	0.31388484E-01	0.0	-0.13888724E-01	-0.33038992E-02
5	0.37208579E-01	0.0	-0.14495194E-01	-0.34950911E-02
6	0.43329056E-01	0.0	-0.15197795E-01	-0.36617300E-02
7	0.4968632E-01	0.0	-0.15995443E-01	-0.37928619E-02
8	0.56256179E-01	0.0	-0.16709320E-01	-0.38806619E-02
9	0.62932312E-01	0.0	-0.17494772E-01	-0.39400235E-02
10	0.69671929E-01	0.0	-0.18359818E-01	-0.39624460E-02
11	0.76400816E-01	0.0	-0.19291390E-01	-0.39374828E-02
12	0.83031952E-01	0.0	-0.20281382E-01	-0.38559851E-02
13	0.89453459E-01	0.0	-0.2131620E-01	-0.37004838E-02
14	0.9523119E-01	0.0	-0.22356205E-01	-0.34523159E-02
15	0.10107177E 00	0.0	-0.23387404E-01	-0.30956920E-02
16	0.10590094E 00	0.0	-0.24364714E-01	-0.26152420E-02
17	0.10980153E 00	0.0	-0.25241464E-01	-0.20127974E-02
18	0.11257684E 00	0.0	-0.25967654E-01	-0.12976155E-02
19	0.11404997E 00	0.0	-0.26486631E-01	-0.48311031E-03
20	0.11408025E 00	0.0	-0.26752960E-01	0.13300835E-02
21	0.11257654E 00	0.0	-0.26721656E-01	0.18763004E-02
22	0.11098343E 00	0.0	-0.26553281E-01	0.24215288E-02
23	0.10885936E 00	0.0	-0.26265956E-01	0.29604402E-02
24	0.10420838E 00	0.0	-0.25854848E-01	0.34866133E-02
25	0.10304147E 00	0.0	-0.25316361E-01	0.39951354E-02
26	0.99374235E-01	0.0	-0.24649419E-01	0.44783205E-02
27	0.9522268E-01	0.0	-0.2392285E-01	0.49320012E-02
28	0.90635180E-01	0.0	-0.22925850E-01	0.53484850E-02
29	0.85623920E-01	0.0	-0.21872737E-01	0.57229474E-02
30	0.80235898E-01	0.0	-0.20698167E-01	0.60481355E-02
31	0.74517190E-01	0.0	-0.19409161E-01	0.63179900E-02
32	0.68516672E-01	0.0	-0.18017568E-01	0.65240007E-02
33	0.62294137E-01	0.0	-0.16532339E-01	0.66626146E-02
34	0.55913594E-01	0.0	-0.14987572E-01	0.66986196E-02
35	0.49447242E-01	0.0	-0.13341818E-01	0.67238500E-02
36	0.42970914E-01	0.0	-0.11674959E-01	0.663685911E-02
37	0.36659103E-01	0.0	-0.1053395E-01	0.61531961E-02
38	0.34409259E-01	0.0	-0.94164275E-02	0.59944404E-02
39	0.30251045E-01	0.0	-0.82973577E-02	0.52125491E-02
40	0.26213132E-01	0.0	-0.71970299E-02	0.47048290E-02
41	0.22328369E-01	0.0	-0.61264448E-02	0.42932853E-02
42	0.18438993E-01	0.0	-0.50978539E-02	0.37302233E-02
43	0.15154134E-01	0.0	-0.41207224E-02	0.31168230E-02
44	0.11935238E-01	0.0	-0.32108594E-02	0.24286371E-02
45	0.90133844E-02	0.0	-0.23089823E-02	0.16449701E-02
46	0.64265132E-02	0.0	-0.16666454E-02	0.62986857E-03
47	0.42159704E-02	0.0	-0.10245033E-02	0.0
48	0.24243123E-02	0.0	-0.53128917E-03	0.0
49	0.10940070E-02	0.0	-0.18448300E-03	0.0
50	0.27044793E-03	0.0	-0.34923796E-05	0.0
51	0.0	0.0	0.0	0.0

STRAINS AND CURVATURES AT THE CENTER OF EACH SHELL ELEMENT
INCREMENT NO. 1 0.0 25 PERCENT OF TOTAL LOAD
ITERATION NO. 3
THETA= 0.0 DEGREES

ELEMENT NO.	E(S)	E(I-THETA)	K(S)	K(I-THETA)	K(I+THETA)
1	-0.80486148E-04	0.61572224E-04	0.11840591E-03	-0.93019509E-04	-0.0
2	-0.17489561E-03	0.65564658E-04	0.12420709E-03	-0.10117216E-03	-0.0
3	-0.21148201E-03	0.66150504E-04	0.12044060E-03	-0.11339023E-03	-0.0
4	-0.23114825E-03	0.55632729E-04	0.11137576E-03	-0.12626892E-03	-0.0
5	-0.24557998E-03	0.41450796E-04	0.97060270E-04	-0.13946700E-03	-0.0
6	-0.25753560E-03	0.25300644E-04	0.76329845E-04	-0.15287573E-03	-0.0
7	-0.16504907E-03	0.48358024E-05	0.51137773E-04	-0.16647110E-03	-0.0
8	-0.16381200E-03	-0.22125678E-04	0.34442302E-04	-0.17875571E-03	-0.0
9	-0.16856533E-03	-0.53356256E-04	0.13031923E-04	-0.19010376E-03	-0.0
10	-0.16914651E-03	-0.88194545E-04	-0.14671656E-04	-0.20155536E-03	-0.0
11	-0.17045370E-03	-0.12510935E-03	-0.47548776E-04	-0.21290504E-03	-0.0
12	-0.16925333E-03	-0.16266704E-03	-0.90836867E-04	-0.22457769E-03	-0.0
13	-0.16756276E-03	-0.19902596E-03	-0.14473178E-03	-0.23668002E-03	-0.0
14	-0.16528671E-03	-0.23302359E-03	-0.20793224E-03	-0.24901354E-03	-0.0
15	-0.16275822E-03	-0.26453356E-03	-0.27995929E-03	-0.26146742E-03	-0.0
16	-0.16002968E-03	-0.29273378E-03	-0.35113166E-03	-0.27358509E-03	-0.0
17	-0.15745027E-03	-0.31693978E-03	-0.41671725E-03	-0.28465339E-03	-0.0
18	-0.15248013E-03	-0.33514979E-03	-0.47450408E-03	-0.29438582E-03	-0.0
19	-0.15054483E-03	-0.34653582E-03	-0.51802065E-03	-0.30120765E-03	-0.0
20	-0.14882811E-03	-0.35123015E-03	-0.54052239E-03	-0.30546496E-03	-0.0
21	-0.14862075E-03	-0.34997077E-03	-0.54885470E-03	-0.30376076E-03	-0.0
22	-0.14773919E-03	-0.34664874E-03	-0.54808496E-03	-0.30614645E-03	-0.0
23	-0.14708482E-03	-0.34085033E-03	-0.54141739E-03	-0.30456437E-03	-0.0
24	-0.14650174E-03	-0.33280696E-03	-0.52896375E-03	-0.30185957E-03	-0.0
25	-0.14716903E-03	-0.32246718E-03	-0.51087025E-03	-0.29802276E-03	-0.0
26	-0.14678591E-03	-0.30998141E-03	-0.48563210E-03	-0.29306556E-03	-0.0
27	-0.14616175E-03	-0.2952321E-03	-0.45564235E-03	-0.28699334E-03	-0.0
28	-0.14573007E-03	-0.27841656E-03	-0.41885588E-03	-0.27984334E-03	-0.0
29	-0.14562339E-03	-0.25984924E-03	-0.37609972E-03	-0.27160905E-03	-0.0
30	-0.14556815E-03	-0.23970773E-03	-0.32655010E-03	-0.26233587E-03	-0.0
31	-0.14872401E-03	-0.21824845E-03	-0.27087690E-03	-0.25203428E-03	-0.0
32	-0.14870295E-03	-0.19581134E-03	-0.20777379E-03	-0.24071393E-03	-0.0
33	-0.14881337E-03	-0.17246827E-03	-0.13817863E-03	-0.22840085E-03	-0.0
34	-0.14954536E-03	-0.14857935E-03	-0.60529215E-04	-0.21511837E-03	-0.0
35	-0.15030098E-03	-0.12469589E-03	0.24450535E-04	-0.20087659E-03	-0.0
36	-0.15224240E-03	-0.10494459E-03	0.10158319E-03	-0.18824835E-03	-0.0
37	-0.15286735E-03	-0.89490874E-04	0.16726470E-03	-0.17764280E-03	-0.0
38	-0.15379096E-03	-0.74488722E-04	0.23727318E-03	-0.16663155E-03	-0.0
39	-0.15407783E-03	-0.60166582E-04	0.31027663E-03	-0.15522169E-03	-0.0
40	-0.15466928E-03	-0.46635512E-04	0.38713822E-03	-0.14342528E-03	-0.0
41	-0.15492525E-03	-0.34085941E-04	0.46791416E-03	-0.13125004E-03	-0.0
42	-0.15516173E-03	-0.22705950E-04	0.55268360E-03	-0.11870210E-03	-0.0
43	-0.15511597E-03	-0.12653134E-04	0.64134737E-03	-0.10578844E-03	-0.0
44	-0.15344404E-03	-0.40807236E-05	0.73461980E-03	-0.92517119E-04	-0.0
45	-0.15268385E-03	0.2822867E-03	0.83068991E-03	-0.78901678E-04	-0.0
46	-0.15211343E-03	0.77591458E-05	0.92996680E-03	-0.64961161E-04	-0.0
47	-0.15028697E-03	0.10514939E-04	0.10331923E-02	-0.50716902E-04	-0.0
48	-0.14769404E-03	0.10826237E-04	0.11399018E-02	-0.36181227E-04	-0.0
49	-0.14423361E-03	0.84503663E-05	0.12500952E-02	-0.21371525E-04	-0.0
50	-0.14006924E-03	0.34238956E-05	0.13986767E-02	-0.69599928E-05	-0.0

STRESS RESULTANTS AT THE CENTER OF EACH SHELL ELEMENT
INCREMENT NO. 1 *** 25 PERCENT OF TOTAL LOAD
ITERATION NO. 3
THETA= 0.0 DEGREES

ELEMENT NO.	N(S)	N(THETA)	N(S-THETA)	M(S)	M(THETA)	M(S-THETA)	Q(S)	Q(THETA)
1	-0.126521E 03	0.727963E 02	0.0	0.176028E 03	-0.111836E 03	0.0	0.204159E 02	0.0
2	-0.301923E 03	0.254724E 02	0.0	0.182554E 03	-0.124308E 03	0.0	-0.147115E 02	0.0
3	-0.372744E 03	0.526315E 01	0.0	0.168099E 03	-0.150271E 03	0.0	-0.213583E 02	0.0
4	-0.417133E 03	-0.266700E 02	0.0	0.142952E 03	-0.180610E 03	0.0	-0.277213E 02	0.0
5	-0.453479E 03	-0.626758E 02	0.0	0.107406E 03	-0.214635E 03	0.0	-0.348947E 02	0.0
6	-0.486157E 03	-0.101065E 03	0.0	0.592602E 02	-0.252812E 03	0.0	-0.427834E 02	0.0
7	-0.530345E 03	-0.144838E 03	0.0	0.387860E 01	-0.489226E 03	0.0	-0.591785E 02	0.0
8	-0.552556E 03	-0.231037E 03	0.0	-0.621911E 02	-0.545986E 03	0.0	-0.600387E 02	0.0
9	-0.598337E 03	-0.336902E 03	0.0	-0.142635E 03	-0.603596E 03	0.0	-0.719012E 02	0.0
10	-0.634103E 03	-0.450404E 03	0.0	-0.243580E 03	-0.667662E 03	0.0	-0.790722E 02	0.0
11	-0.674241E 03	-0.571344E 03	0.0	-0.361197E 03	-0.736428E 03	0.0	-0.955494E 02	0.0
12	-0.706875E 03	-0.691930E 03	0.0	-0.512879E 03	-0.816367E 03	0.0	-0.112279E 03	0.0
13	-0.736755E 03	-0.808152E 03	0.0	-0.699363E 03	-0.908014E 03	0.0	-0.131838E 03	0.0
14	-0.762440E 03	-0.916151E 03	0.0	-0.916238E 03	-0.100946E 04	0.0	-0.152955E 03	0.0
15	-0.784888E 03	-0.101584E 04	0.0	-0.116184E 04	-0.111988E 04	0.0	-0.160708E 03	0.0
16	-0.803468E 03	-0.110460E 04	0.0	-0.140435E 04	-0.122838E 04	0.0	-0.152947E 03	0.0
17	-0.818647E 03	-0.118057E 04	0.0	-0.162773E 04	-0.132805E 04	0.0	-0.135155E 03	0.0
18	-0.820245E 03	-0.123476E 04	0.0	-0.182456E 04	-0.141492E 04	0.0	-0.108065E 03	0.0
19	-0.825044E 03	-0.126979E 04	0.0	-0.196504E 04	-0.147807E 04	0.0	-0.739596E 02	0.0
20	-0.824044E 03	-0.128334E 04	0.0	-0.204931E 04	-0.151592E 04	0.0	-0.367171E 02	0.0
21	-0.822148E 03	-0.127906E 04	0.0	-0.207744E 04	-0.152772E 04	0.0	-0.101776E 02	0.0
22	-0.816059E 03	-0.126743E 04	0.0	-0.207449E 04	-0.152548E 04	0.0	0.115353E 02	0.0
23	-0.808299E 03	-0.124800E 04	0.0	-0.205134E 04	-0.151387E 04	0.0	0.354204E 02	0.0
24	-0.798586E 03	-0.122135E 04	0.0	-0.200834E 04	-0.149299E 04	0.0	0.592475E 02	0.0
25	-0.790694E 03	-0.118849E 04	0.0	-0.194595E 04	-0.146295E 04	0.0	0.824601E 02	0.0
26	-0.777309E 03	-0.114764E 04	0.0	-0.185931E 04	-0.142234E 04	0.0	0.105473E 03	0.0
27	-0.760942E 03	-0.109922E 04	0.0	-0.175619E 04	-0.137349E 04	0.0	0.129558E 03	0.0
28	-0.743188E 03	-0.104428E 04	0.0	-0.162998E 04	-0.131453E 04	0.0	0.152579E 03	0.0
29	-0.724785E 03	-0.983990E 03	0.0	-0.148337E 04	-0.124626E 04	0.0	0.178063E 03	0.0
30	-0.705018E 03	-0.918643E 03	0.0	-0.131372E 04	-0.116801E 04	0.0	0.204110E 03	0.0
31	-0.694379E 03	-0.852146E 03	0.0	-0.112323E 04	-0.108047E 04	0.0	0.229683E 03	0.0
32	-0.672490E 03	-0.779390E 03	0.0	-0.907652E 03	-0.982401E 03	0.0	0.254404E 03	0.0
33	-0.650146E 03	-0.703825E 03	0.0	-0.670067E 03	-0.874802E 03	0.0	0.282191E 03	0.0
34	-0.629287E 03	-0.627094E 03	0.0	-0.405429E 03	-0.756227E 03	0.0	0.309873E 03	0.0
35	-0.608509E 03	-0.550405E 03	0.0	-0.116095E 03	-0.627414E 03	0.0	0.338371E 03	0.0
36	-0.595594E 03	-0.488264E 03	0.0	0.146231E 03	-0.511463E 03	0.0	0.359803E 03	0.0
37	-0.582591E 03	-0.438775E 03	0.0	0.369469E 03	-0.413205E 03	0.0	0.379354E 03	0.0
38	-0.570995E 03	-0.391040E 03	0.0	0.607128E 03	-0.309424E 03	0.0	0.400452E 03	0.0
39	-0.557996E 03	-0.344890E 03	0.0	0.854884E 03	-0.201438E 03	0.0	0.419385E 03	0.0
40	-0.546754E 03	-0.301601E 03	0.0	0.111552E 04	-0.884475E 02	0.0	0.438693E 03	0.0
41	-0.535379E 03	-0.261167E 03	0.0	0.138522E 04	0.295784E 02	0.0	0.461232E 03	0.0
42	-0.525078E 03	-0.224504E 03	0.0	0.167622E 04	0.152696E 03	0.0	0.483466E 03	0.0
43	-0.515153E 03	-0.191873E 03	0.0	0.197621E 04	0.280787E 03	0.0	0.501968E 03	0.0
44	-0.501396E 03	-0.162457E 03	0.0	0.229148E 04	0.414520E 03	0.0	0.522412E 03	0.0
45	-0.492219E 03	-0.139340E 03	0.0	0.261616E 04	0.552088E 03	0.0	0.541359E 03	0.0
46	-0.485660E 03	-0.122781E 03	0.0	0.295155E 04	0.693829E 03	0.0	0.560421E 03	0.0
47	-0.476967E 03	-0.112071E 03	0.0	0.334003E 04	0.840395E 03	0.0	0.579195E 03	0.0
48	-0.468259E 03	-0.102540E 03	0.0	0.366010E 04	0.991294E 03	0.0	0.597438E 03	0.0
49	-0.459352E 03	-0.112877E 03	0.0	0.403172E 04	0.114647E 04	0.0	0.615340E 03	0.0
50	-0.450738E 03	-0.125115E 03	0.0	0.439773E 04	0.129879E 04	0.0	0.631946E 03	0.0

DISPLACEMENT COMPONENTS OF NODES
 INCREMENT NO. 2 *** 50 PERCENT OF TOTAL LOAD
 I T E R A T I O N N O. 2
 THETA= 0.0 DEGREES

NODE NO.	AXIAL	TANGENTIAL	RADIAL	ANGULAR
1	0.32795738E-01	0.0	-0.25789775E-01	-0.54688938E-02
2	0.42527579E-01	0.0	-0.26139542E-01	-0.58845691E-02
3	0.52989278E-01	0.0	-0.26925713E-01	-0.63172244E-02
4	0.64170599E-01	0.0	-0.27962402E-01	-0.67350268E-02
5	0.76030254E-01	0.0	-0.29203884E-01	-0.71192719E-02
6	0.88490963E-01	0.0	-0.30641615E-01	-0.74513592E-02
7	0.10144442E 00	0.0	-0.32272704E-01	-0.77087581E-02
8	0.11476642E 00	0.0	-0.33730377E-01	-0.78808218E-02
9	0.12831807E 00	0.0	-0.35332743E-01	-0.79944544E-02
10	0.14198595E 00	0.0	-0.37095137E-01	-0.80322661E-02
11	0.1561825E 00	0.0	-0.38990565E-01	-0.79732463E-02
12	0.16903758E 00	0.0	-0.41001644E-01	-0.77990331E-02
13	0.18201631E 00	0.0	-0.43088760E-01	-0.7474932E-02
14	0.19426620E 00	0.0	-0.45205042E-01	-0.69621615E-02
15	0.20544505E 00	0.0	-0.47287293E-01	-0.62306300E-02
16	0.21515203E 00	0.0	-0.49255088E-01	-0.5249397E-02
17	0.22296637E 00	0.0	-0.51013969E-01	-0.40225498E-02
18	0.22645238E 00	0.0	-0.52463267E-01	-0.25691567E-02
19	0.23137480E 00	0.0	-0.5348947E-01	-0.91659348E-03
20	0.23133206E 00	0.0	-0.54602266E-01	0.87711564E-03
21	0.22818279E 00	0.0	-0.53912684E-01	0.2752391E-02
22	0.22489774E 00	0.0	-0.53557385E-01	0.38597789E-02
23	0.22053903E 00	0.0	-0.52962519E-01	0.49615987E-02
24	0.21511561E 00	0.0	-0.5218726E-01	0.60499236E-02
25	0.20865023E 00	0.0	-0.51019132E-01	0.71117580E-02
26	0.20117545E 00	0.0	-0.49661841E-01	0.81371740E-02
27	0.19273722E 00	0.0	-0.48043735E-01	0.91106407E-02
28	0.18339425E 00	0.0	-0.46166871E-01	0.10023747E-01
29	0.17321169E 00	0.0	-0.44036772E-01	0.10861009E-01
30	0.15227174E 00	0.0	-0.41664109E-01	0.11612680E-01
31	0.15066814E 00	0.0	-0.39063096E-01	0.12244196E-01
32	0.13850039E 00	0.0	-0.36257394E-01	0.12803238E-01
33	0.12588966E 00	0.0	-0.33264954E-01	0.13214823E-01
34	0.11296576E 00	0.0	-0.30114599E-01	0.13486519E-01
35	0.99874914E-01	0.0	-0.26842866E-01	0.15601765E-01
36	0.86770356E-01	0.0	-0.23489714E-01	0.13545580E-01
37	0.78049183E-01	0.0	-0.21217883E-01	0.13404202E-01
38	0.69456160E-01	0.0	-0.18947538E-01	0.13173923E-01
39	0.61051115E-01	0.0	-0.16697329E-01	0.12849651E-01
40	0.52891787E-01	0.0	-0.1448484E-01	0.12427084E-01
41	0.45044196E-01	0.0	-0.12332194E-01	0.11900987E-01
42	0.37577900E-01	0.0	-0.10262272E-01	0.11266615E-01
43	0.30558933E-01	0.0	-0.82987361E-02	0.10518178E-01
44	0.24042764E-01	0.0	-0.64685866E-02	0.96508041E-02
45	0.18167961E-01	0.0	-0.47986929E-02	0.96584538E-02
46	0.12950659E-01	0.0	-0.3320934E-02	0.75370185E-02
47	0.84938444E-02	0.0	-0.20681876E-02	0.62825084E-02
48	0.48827939E-02	0.0	-0.10742373E-02	0.48900594E-02
49	0.22027511E-02	0.0	-0.37484081E-03	0.33543531E-02
50	0.54449751E-03	0.0	-0.87382750E-05	0.16714833E-02
51	0.0	0.0	0.0	0.0

STRAINS AND CURVATURES AT THE CENTER OF EACH SHELL ELEMENT
 INCREMENT NO. 2 *** 50 PERCENT OF TOTAL LOAD
 I T E R A T I O N N O. 2
 THETA= 0.0 DEGREES

ELEMENT NO.	E(S)	E(THETA)	E(S-THETA)	K(S)	K(META)	K(S-META)
1	-0.15949531E-03	0.12196298E-03	0.0	0.24209607E-03	-0.19081550E-03	-0.0
2	-0.34949393E-03	0.12955705E-03	0.0	0.25208015E-03	-0.20812177E-03	-0.0
3	-0.42287307E-03	0.13032627E-03	0.0	0.24339647E-03	-0.23304809E-03	-0.0
4	-0.46707826E-03	0.10872698E-03	0.0	0.22386618E-03	-0.25931187E-03	-0.0
5	-0.45069454E-03	0.79783189E-04	0.0	0.19346891E-03	-0.28622127E-03	-0.0
6	-0.51431987E-03	0.46947506E-04	0.0	0.14991664E-03	-0.31357375E-03	-0.0
7	-0.32414612E-03	0.55743203E-05	0.0	0.10026926E-03	-0.34111761E-03	-0.0
8	-0.32114750E-02	-0.48797054E-04	0.0	0.66078792E-04	-0.3658847E-03	-0.0
9	-0.33022114E-03	-0.11183576E-03	0.0	0.21980290E-04	-0.3888252E-03	-0.0
10	-0.33111963E-03	-0.18213103E-03	0.0	-0.34659912E-04	-0.41199033E-03	-0.0
11	-0.33357530E-03	-0.25649485E-03	0.0	-0.10154088E-03	-0.43493719E-03	-0.0
12	-0.33126376E-03	-0.33203838E-03	0.0	-0.18933264E-03	-0.45852014E-03	-0.0
13	-0.32845023E-03	-0.40503615E-03	0.0	-0.29869797E-03	-0.48296293E-03	-0.0
14	-0.32467861E-03	-0.47315052E-03	0.0	-0.42635365E-03	-0.50782808E-03	-0.0
15	-0.32072794E-03	-0.53613633E-03	0.0	-0.57162857E-03	-0.5328966E-03	-0.0
16	-0.31656376E-03	-0.59236656E-03	0.0	-0.71488041E-03	-0.55723358E-03	-0.0
17	-0.31272834E-03	-0.64049871E-03	0.0	-0.84684673E-03	-0.57941116E-03	-0.0
18	-0.30382513E-03	-0.67653949E-03	0.0	-0.95286833E-03	-0.59823599E-03	-0.0
19	-0.30059414E-03	-0.69886143E-03	0.0	-0.10449679E-02	-0.61235554E-03	-0.0
20	-0.29702182E-03	-0.70776930E-03	0.0	-0.10937536E-02	-0.62065222E-03	-0.0
21	-0.29568421E-03	-0.70482818E-03	0.0	-0.11094764E-02	-0.62269857E-03	-0.0
22	-0.29297988E-03	-0.69787283E-03	0.0	-0.11079726E-02	-0.62157656E-03	-0.0
23	-0.29046275E-03	-0.68596983E-03	0.0	-0.10935124E-02	-0.61817118E-03	-0.0
24	-0.28772396E-03	-0.66958996E-03	0.0	-0.10681725E-02	-0.61249617E-03	-0.0
25	-0.28723036E-03	-0.64862403E-03	0.0	-0.10304358E-02	-0.60451869E-03	-0.0
26	-0.2844394E-03	-0.62337681E-03	0.0	-0.97866729E-03	-0.59428299E-03	-0.0
27	-0.28102007E-03	-0.59361705E-03	0.0	-0.91676135E-03	-0.58178487E-03	-0.0
28	-0.27792552E-03	-0.55973511E-03	0.0	-0.84210816E-03	-0.56711163E-03	-0.0
29	-0.27548196E-03	-0.52236812E-03	0.0	-0.75475359E-03	-0.55025006E-03	-0.0
30	-0.27320837E-03	-0.48186816E-03	0.0	-0.65421313E-03	-0.53129112E-03	-0.0
31	-0.27392807E-03	-0.43874513E-03	0.0	-0.54099294E-03	-0.51025837E-03	-0.0
32	-0.27565775E-03	-0.39367471E-03	0.0	-0.41329535E-03	-0.48717609E-03	-0.0
33	-0.27452805E-03	-0.34679845E-03	0.0	-0.27298182E-03	-0.46210201E-03	-0.0
34	-0.27515576E-03	-0.29883557E-03	0.0	-0.11555591E-03	-0.43507433E-03	-0.0
35	-0.27640210E-03	-0.2508829E-03	0.0	0.56195742E-04	-0.40612463E-03	-0.0
36	-0.28045336E-03	-0.21123816E-03	0.0	0.21145810E-03	-0.38048276E-03	-0.0
37	-0.2821821E-03	-0.18021086E-03	0.0	0.34459634E-03	-0.35895407E-03	-0.0
38	-0.28492906E-03	-0.15008613E-03	0.0	0.48563816E-03	-0.33661956E-03	-0.0
39	-0.28665946E-03	-0.12132137E-03	0.0	0.63240249E-03	-0.31349296E-03	-0.0
40	-0.28927973E-03	-0.94138217E-04	0.0	0.78748772E-03	-0.28959473E-03	-0.0
41	-0.29149163E-03	-0.69918467E-04	0.0	0.94969687E-03	-0.2649434E-03	-0.0
42	-0.29392866E-03	-0.44038913E-04	0.0	0.11202367E-02	-0.23955207E-03	-0.0
43	-0.29599620E-03	-0.25815621E-04	0.0	0.12979747E-02	-0.21343908E-03	-0.0
44	-0.29488746E-03	-0.8554061E-05	0.0	0.14854458E-02	-0.18661388E-03	-0.0
45	-0.29571750E-03	0.53582235E-05	0.0	0.16783539E-02	-0.15910863E-03	-0.0
46	-0.29686000E-03	0.15330457E-04	0.0	0.18774604E-02	-0.13096326E-03	-0.0
47	-0.29537617E-03	0.20926716E-04	0.0	0.20844252E-02	-0.10222045E-03	-0.0
48	-0.29206160E-03	0.21614555E-04	0.0	0.22983165E-02	-0.72904804E-04	-0.0
49	-0.28655003E-03	0.16902675E-04	0.0	0.25191198E-02	-0.43052525E-04	-0.0
50	-0.27896441E-03	0.68611762E-05	0.0	0.27366115E-02	-0.14017352E-04	-0.0

STRESS RESULTANTS AT THE CENTER OF EACH SHELL ELEMENT
 INCREMENT NO. 2 *** 50 PERCENT OF TOTAL LOAD
 I T E R A T I O N N O. 2
 THETA= 0.0 DEGREES

ELEMENT NO.	N(S)	M(THETA)	MIS-THETA)	M(S)	M(THETA)	MIS-THETA)	Q(S)	Q(META)
1	-0.239059E 03	0.144156E 03	0.0	0.359546E 03	-0.229879E 03	0.0	0.350487E 02	0.0
2	-0.604186E 03	0.480601E 02	0.0	0.368867E 03	-0.257715E 03	0.0	-0.310902E 02	0.0
3	-0.746463E 03	0.673840E 01	0.0	0.337432E 03	-0.311265E 03	0.0	-0.446489E 02	0.0
4	-0.833323E 03	-0.581502E 02	0.0	0.284119E 03	-0.373746E 03	0.0	-0.576399E 02	0.0
5	-0.907872E 03	-0.131145E 03	0.0	0.209293E 03	-0.443823E 03	0.0	-0.725254E 02	0.0
6	-0.972989E 03	-0.208812E 03	0.0	0.108620E 03	-0.522439E 03	0.0	-0.885794E 02	0.0
7	-0.104538E 04	-0.297170E 03	0.0	-0.669751E 01	-0.100831E 04	0.0	-0.120594E 03	0.0
8	-0.108854E 04	-0.470512E 03	0.0	-0.141621E 03	-0.112189E 04	0.0	-0.122526E 03	0.0
9	-0.117926E 04	-0.683693E 03	0.0	-0.306885E 03	-0.123909E 04	0.0	-0.146852E 03	0.0
10	-0.125054E 04	-0.912447E 03	0.0	-0.513030E 03	-0.136928E 04	0.0	-0.160930E 03	0.0
11	-0.133082E 04	-0.115590E 04	0.0	-0.752158E 03	-0.150871E 04	0.0	-0.193825E 03	0.0
12	-0.139679E 04	-0.139855E 04	0.0	-0.105965E 04	-0.167054E 04	0.0	-0.227603E 03	0.0
13	-0.145866E 04	-0.163245E 04	0.0	-0.143800E 04	-0.185614E 04	0.0	-0.266548E 03	0.0
14	-0.151268E 04	-0.184960E 04	0.0	-0.187601E 04	-0.206089E 04	0.0	-0.308543E 03	0.0
15	-0.156113E 04	-0.204994E 04	0.0	-0.237134E 04	-0.228345E 04	0.0	-0.323256E 03	0.0
16	-0.160231E 04	-0.222817E 04	0.0	-0.285939E 04	-0.250166E 04	0.0	-0.307339E 03	0.0
17	-0.163669E 04	-0.238048E 04	0.0	-0.330878E 04	-0.270189E 04	0.0	-0.271204E 03	0.0
18	-0.164288E 04	-0.248865E 04	0.0	-0.370318E 04	-0.287575E 04	0.0	-0.216047E 03	0.0
19	-0.165411E 04	-0.255787E 04	0.0	-0.398306E 04	-0.300137E 04	0.0	-0.146112E 03	0.0
20	-0.165120E 04	-0.258328E 04	0.0	-0.420224E 04	-0.309763E 04	0.0	-0.710034E 02	0.0
21	-0.164400E 04	-0.257244E 04	0.0	-0.419627E 04	-0.307571E 04	0.0	-0.190542E 02	0.0
22	-0.16287E 04	-0.254726E 04	0.0	-0.41609E 04	-0.309253E 04	0.0	0.246102E 02	0.0
23	-0.160873E 04	-0.250623E 04	0.0	-0.414609E 04	-0.306743E 04	0.0	0.752460E 02	0.0
24	-0.158392E 04	-0.245047E 04	0.0	-0.405842E 04	-0.302439E 04	0.0	0.119944E 03	0.0
25	-0.156193E 04	-0.238202E 04	0.0	-0.392833E 04	-0.296183E 04	0.0	0.165830E 03	0.0
26	-0.152835E 04	-0.229746E 04	0.0	-0.375056E 04	-0.287830E 04	0.0	0.215935E 03	0.0
27	-0.148831E 04	-0.219766E 04	0.0	-0.353772E 04	-0.277750E 04	0.0	0.268322E 03	0.0
28	-0.144532E 04	-0.208481E 04	0.0	-0.328144E 04	-0.265741E 04	0.0	0.308441E 03	0.0
29	-0.140106E 04	-0.196130E 04	0.0	-0.298186E 04	-0.251779E 04	0.0	0.365049E 03	0.0
30	-0.135430E 04	-0.182780E 04	0.0	-0.263749E 04	-0.235856E 04	0.0	0.414425E 03	0.0
31	-0.132593E 04	-0.169208E 04	0.0	-0.225001E 04	-0.218028E 04	0.0	0.466978E 03	0.0
32	-0.127647E 04	-0.154428E 04	0.0	-0.181359E 04	-0.198125E 04	0.0	0.513846E 03	0.0
33	-0.122722E 04	-0.139122E 04	0.0	-0.133435E 04	-0.176350E 04	0.0	0.567911E 03	0.0
34	-0.118261E 04	-0.123635E 04	0.0	-0.797725E 03	-0.152279E 04	0.0	0.625767E 03	0.0
35	-0.114002E 04	-0.108213E 04	0.0	-0.212794E 03	-0.126191E 04	0.0	0.681679E 03	0.0
36	-0.111460E 04	-0.957530E 03	0.0	0.315466E 03	-0.102778E 04	0.0	0.728644E 03	0.0
37	-0.109014E 04	-0.858664E 03	0.0	0.768005E 03	-0.828512E 03	0.0	0.766301E 03	0.0
38	-0.106963E 04	-0.763643E 03	0.0	0.124695E 04	-0.618942E 03	0.0	0.804440E 03	0.0
39	-0.104727E 04	-0.672078E 03	0.0	0.174521E 04	-0.401240E 03	0.0	0.843639E 03	0.0
40	-0.102933E 04	-0.586505E 03	0.0	0.227120E 04	-0.172943E 03	0.0	0.887100E 03	0.0
41	-0.101197E 04	-0.506900E 03	0.0	0.282102E 04	0.647239E 02	0.0	0.925836E 03	0.0
42	-0.997618E 03	-0.435100E 03	0.0	0.339856E 04	0.312891E 03	0.0	0.970766E 03	0.0
43	-0.984653E 03	-0.37152E 03	0.0	0.400014E 04	0.570398E 03	0.0	0.100886E 04	0.0
44	-0.964274E 03	-0.314523E 03	0.0	0.463396E 04	0.839679E 03	0.0	0.104959E 04	0.0
45	-0.953432E 03	-0.27023E 03	0.0	0.528607E 04	0.111645E 04	0.0	0.108699E 04	0.0
46	-0.947438E 03	-0.23907E 03	0.0	0.595890E 04	0.140133E 04	0.0	0.112470E 04	0.0
47	-0.937185E 03	-0.219422E 03	0.0	0.665778E 04	0.169579E 04	0.0	0.116230E 04	0.0
48	-0.925771E 03	-0.213948E 03	0.0	0.737968E 04	0.199883E 04	0.0	0.119838E 04	0.0
49	-0.912486E 03	-0.223883E 03	0.0	0.812450E 04	0.231034E 04	0.0	0.123344E 04	0.0
50	-0.897661E 03	-0.249058E 03	0.0	0.885779E 04	0.261599E 04	0.0	0.126642E 04	0.0

DISPLACEMENT COMPONENTS OF MODES
 INCREMENT NO. 3 *** 75 PERCENT OF TOTAL LOAD
 I T E R A T I O N N O. 1
 THETA= 0.0 DEGREES

NODE NO.	AXIAL	TANGENTIAL	RADIAL	ANGULAR
1	0.50447226E-01	0.0	-0.38904525E-01	-0.83791204E-02
2	0.65360725E-01	0.0	-0.39450098E-01	-0.90175346E-02
3	0.81389427E-01	0.0	-0.40660761E-01	-0.96768513E-02
4	0.98512113E-01	0.0	-0.42255294E-01	-0.10310665E-01
5	0.11666071E 00	0.0	-0.44164225E-01	-0.10889955E-01
6	0.13571090E 00	0.0	-0.46373907E-01	-0.11385944E-01
7	0.15549094E 00	0.0	-0.48878763E-01	-0.11763897E-01
8	0.17581153E 00	0.0	-0.51113829E-01	-0.12015983E-01
9	0.19646335E 00	0.0	-0.5368307E-01	-0.12177862E-01
10	0.21727192E 00	0.0	-0.56264069E-01	-0.1222826E-01
11	0.23800373E 00	0.0	-0.59159204E-01	-0.12119170E-01
12	0.25838691E 00	0.0	-0.62225703E-01	-0.11839293E-01
13	0.27807426E 00	0.0	-0.65402150E-01	-0.11330195E-01
14	0.29662687E 00	0.0	-0.68616509E-01	-0.10535505E-01
15	0.31352538E 00	0.0	-0.71771264E-01	-0.94083697E-02
16	0.32816303E 00	0.0	-0.74743509E-01	-0.79034865E-02
17	0.33990377E 00	0.0	-0.77389956E-01	-0.60276501E-02
18	0.34815073E 00	0.0	-0.79558432E-01	-0.38103901E-02
19	0.35236907E 00	0.0	-0.81078410E-01	0.12936164E-02
20	0.35213631E 00	0.0	-0.8181541E-01	0.14343557E-02
21	0.34718180E 00	0.0	-0.81637681E-01	0.42872466E-02
22	0.34209347E 00	0.0	-0.81074953E-01	0.59635490E-02
23	0.33537614E 00	0.0	-0.80150127E-01	0.76346211E-02
24	0.32704413E 00	0.0	-0.78849673E-01	0.92840828E-02
25	0.31713289E 00	0.0	-0.77163815E-01	0.10892194E-01
26	0.30569303E 00	0.0	-0.75090110E-01	0.12443904E-01
27	0.29279530E 00	0.0	-0.72624385E-01	0.13915658E-01
28	0.27852988E 00	0.0	-0.69770157E-01	0.1524731E-01
29	0.26299632E 00	0.0	-0.66536129E-01	0.16557645E-01
30	0.24632061E 00	0.0	-0.62938631E-01	0.17689716E-01
31	0.22864555E 00	0.0	-0.58999415E-01	0.18668905E-01
32	0.21012348E 00	0.0	-0.54753751E-01	0.19476689E-01
33	0.19093859E 00	0.0	-0.50229039E-01	0.20090487E-01
34	0.17128873E 00	0.0	-0.45468554E-01	0.20491656E-01
35	0.15139586E 00	0.0	-0.40527254E-01	0.20655304E-01
36	0.13149285E 00	0.0	-0.35465039E-01	0.20559013E-01
37	0.11925317E 00	0.0	-0.32036033E-01	0.20337347E-01
38	0.10521245E 00	0.0	-0.28609794E-01	0.19981150E-01
39	0.92461407E-01	0.0	-0.25214270E-01	0.19482799E-01
40	0.80087423E-01	0.0	-0.21875903E-01	0.18835932E-01
41	0.68190634E-01	0.0	-0.18627759E-01	0.18032700E-01
42	0.56875095E-01	0.0	-0.15504260E-01	0.17066095E-01
43	0.46241425E-01	0.0	-0.12541097E-01	0.15927467E-01
44	0.36403168E-01	0.0	-0.97787082E-02	0.14609575E-01
45	0.27478728E-01	0.0	-0.72578788E-02	0.13103426E-01
46	0.19582752E-01	0.0	-0.50258040E-02	0.11402946E-01
47	0.12840115E-01	0.0	-0.31329633E-02	0.95022656E-02
48	0.73796289E-02	0.0	-0.16301698E-02	0.73941374E-02
49	0.33275264E-02	0.0	-0.5715173E-03	0.50706677E-02
50	0.82189776E-03	0.0	-0.15846716E-04	0.25260667E-02
51	0.0	0.0	0.0	0.0

STRAINS AND CURVATURES AT THE CENTER OF EACH SHELL ELEMENT
INCREMENT NO. 3 *** 75 PERCENT OF TOTAL LOAD
ITERATION NO. 1
THETA= 0.0 DEGREES

ELEMENT NO.	E(S)	E(THETA)	E(S-THETA)	K(S)	K(THETA)	K(S-THETA)
1	-0.23688677E-03	0.18113964E-03	0.0	0.37183356E-03	-0.29392587E-03	-0.0
2	-0.52387430E-03	0.19191243E-03	0.0	0.36412260E-03	-0.32150280E-03	-0.0
3	-0.63430564E-03	0.19242706E-03	0.0	0.36922959E-03	-0.35971915E-03	-0.0
4	-0.69292891E-03	0.15911488E-03	0.0	0.33751340E-03	-0.39993948E-03	-0.0
5	-0.73546870E-03	0.11476291E-03	0.0	0.28882199E-03	-0.44118194E-03	-0.0
6	-0.77043637E-03	0.64627849E-04	0.0	0.2032179E-03	-0.48306189E-03	-0.0
7	-0.47679269E-03	0.18662377E-05	0.0	0.14686525E-03	-0.52496674E-03	-0.0
8	-0.47150976E-03	-0.80427766E-04	0.0	0.94029325E-04	-0.56247157E-03	-0.0
9	-0.48544330E-03	-0.17593526E-03	0.0	0.26264112E-04	-0.59724343E-03	-0.0
10	-0.48544398E-03	-0.28239191E-03	0.0	-0.60673221E-04	-0.63241879E-03	-0.0
11	-0.4890850E-03	-0.39481674E-03	0.0	-0.16301757E-03	-0.66725235E-03	-0.0
12	-0.4853373E-03	-0.50883763E-03	0.0	-0.2971500E-03	-0.70303027E-03	-0.0
13	-0.48235455E-03	-0.61880215E-03	0.0	-0.46328129E-03	-0.74005313E-03	-0.0
14	-0.47794729E-03	-0.72118267E-03	0.0	-0.65727136E-03	-0.77768415E-03	-0.0
15	-0.47376053E-03	-0.81562740E-03	0.0	-0.8767711E-03	-0.81551657E-03	-0.0
16	-0.46961636E-03	-0.89971954E-03	0.0	-0.10930940E-02	-0.85218437E-03	-0.0
17	-0.46593323E-03	-0.97149052E-03	0.0	-0.12920974E-02	-0.88549894E-03	-0.0
18	-0.45414362E-03	-0.10249636E-02	0.0	-0.14666095E-02	-0.91366214E-03	-0.0
19	-0.45019388E-03	-0.10577475E-02	0.0	-0.15899586E-02	-0.93463529E-03	-0.0
20	-0.44454914E-03	-0.10703593E-02	0.0	-0.16614634E-02	-0.94671478E-03	-0.0
21	-0.44103782E-03	-0.10652866E-02	0.0	-0.16845919E-02	-0.94940839E-03	-0.0
22	-0.4345570E-03	-0.10543638E-02	0.0	-0.16801732E-02	-0.94737392E-03	-0.0
23	-0.42965729E-03	-0.10360258E-02	0.0	-0.16578340E-02	-0.94188028E-03	-0.0
24	-0.42313896E-03	-0.1010902E-02	0.0	-0.16173844E-02	-0.93293120E-03	-0.0
25	-0.41941274E-03	-0.97908452E-03	0.0	-0.1587094E-02	-0.92048338E-03	-0.0
26	-0.41206391E-03	-0.94076712E-03	0.0	-0.14743776E-02	-0.90450898E-03	-0.0
27	-0.40346524E-03	-0.89570042E-03	0.0	-0.13849789E-02	-0.88529428E-03	-0.0
28	-0.39531942E-03	-0.84446976E-03	0.0	-0.12702520E-02	-0.86268922E-03	-0.0
29	-0.38815886E-03	-0.78803604E-03	0.0	-0.11370189E-02	-0.83675934E-03	-0.0
30	-0.38142828E-03	-0.72692358E-03	0.0	-0.98368060E-03	-0.80766273E-03	-0.0
31	-0.36432609E-03	-0.66188932E-03	0.0	-0.81212563E-03	-0.77542267E-03	-0.0
32	-0.37914026E-03	-0.59394841E-03	0.0	-0.6166333E-03	-0.74007874E-03	-0.0
33	-0.37536235E-03	-0.52330573E-03	0.0	-0.4025152E-03	-0.70172874E-03	-0.0
34	-0.37509413E-03	-0.45103976E-03	0.0	-0.16475396E-03	-0.66044834E-03	-0.0
35	-0.37662899E-03	-0.37880708E-03	0.0	0.96771779E-04	-0.61626174E-03	-0.0
36	-0.38299547E-03	-0.31907484E-03	0.0	0.33180485E-03	-0.57716668E-03	-0.0
37	-0.38655475E-03	-0.27237101E-03	0.0	0.53335098E-03	-0.54436528E-03	-0.0
38	-0.39196899E-03	-0.22693262E-03	0.0	0.74604107E-03	-0.51036174E-03	-0.0
39	-0.39642304E-03	-0.18358031E-03	0.0	0.96813170E-03	-0.47516753E-03	-0.0
40	-0.40264754E-03	-0.14260151E-03	0.0	0.12021970E-02	-0.43682430E-03	-0.0
41	-0.40869392E-03	-0.10456903E-03	0.0	0.14468811E-02	-0.40136115E-03	-0.0
42	-0.41541271E-03	-0.70059970E-04	0.0	0.17040162E-02	-0.36279508E-03	-0.0
43	-0.42189891E-03	-0.39522682E-04	0.0	0.15724285E-02	-0.32315403E-03	-0.0
44	-0.42867661E-03	0.75990838E-05	0.0	0.25449740E-02	-0.28246199E-03	-0.0
45	-0.43394021E-03	0.22714506E-04	0.0	0.2844527E-02	-0.24076240E-03	-0.0
46	-0.43508410E-03	0.31241929E-04	0.0	0.31557810E-02	-0.19811718E-03	-0.0
47	-0.43301587E-03	0.32374315E-04	0.0	0.34773191E-02	-0.15459285E-03	-0.0
48	-0.42692828E-03	0.25365225E-04	0.0	0.38090807E-02	-0.11022697E-03	-0.0
49	-0.41669677E-03	0.10309473E-04	0.0	0.41357577E-02	-0.65074448E-04	-0.0
50						

STRESS RESULTANTS AT THE CENTER OF EACH SHELL ELEMENT
 INCREMENT NO. 3 *** 75 PERCENT OF TOTAL LOAD
 I T E R A T I O N N O. 1
 THETA= 0.0 DEGREES

ELEMENT NO.	N(S)	M(THETA)	N(S-THETA)	M(S)	M(THETA)	M(S-THETA)	Q(S)	Q(THETA)
1	-0.355059E 03	0.214099E 03	0.0	0.551726E 03	-0.354731E 03	0.0	0.433905E 02	0.0
2	-0.906979E 04	0.675911E 02	0.0	0.559537E 03	-0.401198E 03	0.0	-0.495350E 02	0.0
3	-0.112147E 04	0.415356E 01	0.0	0.508269E 03	-0.484222E 03	0.0	-0.705076E 02	0.0
4	-0.125494E 04	-0.948479E 02	0.0	0.423110E 03	-0.580955E 03	0.0	-0.904763E 02	0.0
5	-0.136356E 04	-0.205938E 02	0.0	0.304338E 03	-0.689590E 03	0.0	-0.113502E 03	0.0
6	-0.146983E 04	-0.323857E 03	0.0	0.146663E 03	-0.811020E 03	0.0	-0.138010E 03	0.0
7	-0.154383E 04	-0.457643E 03	0.0	-0.344292E 02	-0.155898E 04	0.0	-0.185639E 03	0.0
8	-0.160674E 04	-0.719282E 03	0.0	-0.242198E 03	-0.173195E 04	0.0	-0.187588E 03	0.0
9	-0.174165E 04	-0.104150E 04	0.0	-0.495693E 03	-0.191057E 04	0.0	-0.224550E 03	0.0
10	-0.184832E 04	-0.138755E 04	0.0	-0.811732E 03	-0.210915E 04	0.0	-0.246227E 03	0.0
11	-0.196899E 04	-0.175538E 04	0.0	-0.117738E 04	-0.232161E 04	0.0	-0.295079E 03	0.0
12	-0.206916E 04	-0.212182E 04	0.0	-0.164702E 04	-0.256804E 04	0.0	-0.345817E 03	0.0
13	-0.216548E 04	-0.247511E 04	0.0	-0.222157E 04	-0.284962E 04	0.0	-0.404762E 03	0.0
14	-0.225076E 04	-0.280271E 04	0.0	-0.288703E 04	-0.316028E 04	0.0	-0.467439E 03	0.0
15	-0.232903E 04	-0.310481E 04	0.0	-0.363541E 04	-0.349639E 04	0.0	-0.490375E 03	0.0
16	-0.239738E 04	-0.337338E 04	0.0	-0.437231E 04	-0.382564E 04	0.0	-0.463782E 03	0.0
17	-0.245524E 04	-0.360247E 04	0.0	-0.504983E 04	-0.412171E 04	0.0	-0.409010E 03	0.0
18	-0.246903E 04	-0.376435E 04	0.0	-0.564295E 04	-0.438818E 04	0.0	-0.324615E 02	0.0
19	-0.248011E 04	-0.386678E 04	0.0	-0.606321E 04	-0.457613E 04	0.0	-0.220611E 03	0.0
20	-0.248207E 04	-0.390218E 04	0.0	-0.630676E 04	-0.466883E 04	0.0	-0.103689E 03	0.0
21	-0.246576E 04	-0.388232E 04	0.0	-0.638436E 04	-0.471606E 04	0.0	-0.246415E 02	0.0
22	-0.243704E 04	-0.384148E 04	0.0	-0.636805E 04	-0.470517E 04	0.0	0.445438E 02	0.0
23	-0.240041E 04	-0.377639E 04	0.0	-0.629292E 04	-0.466563E 04	0.0	0.115222E 03	0.0
24	-0.235493E 04	-0.368890E 04	0.0	-0.615046E 04	-0.459728E 04	0.0	0.192788E 03	0.0
25	-0.231182E 04	-0.358184E 04	0.0	-0.594814E 04	-0.449987E 04	0.0	0.264784E 03	0.0
26	-0.225073E 04	-0.345048E 04	0.0	-0.567715E 04	-0.437174E 04	0.0	0.331612E 03	0.0
27	-0.217903E 04	-0.329602E 04	0.0	-0.53073E 04	-0.421444E 04	0.0	0.404806E 03	0.0
28	-0.210280E 04	-0.312202E 04	0.0	-0.495683E 04	-0.403194E 04	0.0	0.475841E 03	0.0
29	-0.202470E 04	-0.293211E 04	0.0	-0.449971E 04	-0.381835E 04	0.0	0.554367E 03	0.0
30	-0.194345E 04	-0.272746E 04	0.0	-0.397433E 04	-0.357490E 04	0.0	0.635807E 03	0.0
31	-0.188960E 04	-0.251945E 04	0.0	-0.338683E 04	-0.330354E 04	0.0	0.703575E 03	0.0
32	-0.180671E 04	-0.229416E 04	0.0	-0.271872E 04	-0.299895E 04	0.0	0.782721E 03	0.0
33	-0.172576E 04	-0.206148E 04	0.0	-0.198731E 04	-0.266629E 04	0.0	0.867721E 03	0.0
34	-0.165461E 04	-0.182695E 04	0.0	-0.117640E 04	-0.230124E 04	0.0	0.947616E 03	0.0
35	-0.158932E 04	-0.159428E 04	0.0	-0.285620E 03	-0.190366E 04	0.0	0.103661E 04	0.0
36	-0.155189E 04	-0.140684E 04	0.0	0.514321E 03	-0.154834E 04	0.0	0.110682E 04	0.0
37	-0.151796E 04	-0.125875E 04	0.0	0.119958E 04	-0.124600E 04	0.0	0.116384E 04	0.0
38	-0.149136E 04	-0.111686E 04	0.0	0.192214E 04	-0.928923E 03	0.0	0.127034E 04	0.0
39	-0.146364E 04	-0.980654E 03	0.0	0.267633E 04	-0.598843E 03	0.0	0.12644E 04	0.0
40	-0.144397E 04	-0.853864E 03	0.0	0.347046E 04	-0.253393E 03	0.0	0.133852E 04	0.0
41	-0.142658E 04	-0.736453E 03	0.0	0.430010E 04	0.106016E 03	0.0	0.139974E 04	0.0
42	-0.141479E 04	-0.631088E 03	0.0	0.517118E 04	0.481108E 03	0.0	0.14446E 04	0.0
43	-0.140613E 04	-0.538432E 03	0.0	0.607985E 04	0.870453E 03	0.0	0.152130E 04	0.0
44	-0.138676E 04	-0.455700E 03	0.0	0.703418E 04	0.127699E 04	0.0	0.158212E 04	0.0
45	-0.138464E 04	-0.392265E 03	0.0	0.801604E 04	0.169456E 04	0.0	0.163811E 04	0.0
46	-0.138005E 04	-0.348383E 03	0.0	0.902858E 04	0.212413E 04	0.0	0.16936E 04	0.0
47	-0.137225E 04	-0.316169E 03	0.0	0.100799E 05	0.256793E 04	0.0	0.174896E 04	0.0
48	-0.137225E 04	-0.316169E 03	0.0	0.11654E 05	0.30244E 04	0.0	0.180307E 04	0.0
49	-0.135933E 04	-0.332971E 03	0.0	0.122848E 05	0.349349E 04	0.0	0.185454E 04	0.0
50	-0.134080E 04	-0.471828E 03	0.0	0.133865E 05	0.395347E 04	0.0	0.190333E 04	0.0

1

SAMPLE PROBLEM NO. 1
 THE SHELL IS A STAINLESS SANDWICH STRUCTURE WITH A NOMINAL THICKNESS OF TWO INCHES.
 THE LOAD IS APPLIED OVER A CIRCULAR AREA OF TWENTY INCHES RADIUS. THE CENTER OF THE CIRCULAR AREA IS LOCATED AT 90 DEGREES IN THE CIRCUMFERENTIAL DIRECTION AND AT 15 DEGREES INCLINATION.
 FIND BUCKLING LOAD FOR THE APOLOC AFT HEAT SHIELD
 NUMBER OF LOADING INCREMENTS = 4 *** TOTAL LOAD = 200 PSI *** ERROR = 6 PERCENT
 4 A HARMONICS WERE USED WITH NA = 6 *** NET = 29

(BLANK CARD)

4 0 8 50 29 1 1 6 0 4 20 0 1SAND 2.0 RUCK 0.06 1.0 0 0001
 APOL 0.0 45.45 20.0 --200 0

0.0

(BLANK CARD)

175.6 0.0 58.1

3

20 11.2
 15 4.87
 15 3.27

CONS 1 6 0.030

0.0	0.050	30 0	0.030	145.0	0.020	215.0	0.030
330.0	0.050						
0.0	0.050	40 0	0.030	145.0	0.020	215.0	0.030
320.0	0.050						
0.0	0.050	40 0	0.030	127.5	0.020	232.5	0.030
320.0	0.050						
0.0	0.050	50 0	0.030	127.5	0.020	232.5	0.030
310.0	0.050						
0.0	0.050	50 0	0.030	115.0	0.020	245.0	0.030
310.0	0.050						

0.0	0.050	60.0	0.030	95.0	0.020	135.0	0.012
225.0	0.020	265.0	0.030	300.0	0.050		
0.0	0.050	60.0	0.030	95.0	0.020	127.5	0.012
232.5	0.020	265.0	0.030	300.0	0.050		
0.0	0.050	60.0	0.030	95.0	0.020	127.5	0.012
232.5	0.020	265.0	0.030	300.0	0.050		
0.0	0.050	60.0	0.030	90.0	0.020	127.5	0.012
232.5	0.020	270.0	0.030	300.0	0.050		
0.0	0.050	60.0	0.030	90.0	0.020	127.5	0.012
232.5	0.020	270.0	0.030	300.0	0.050		
0.0	0.050	60.0	0.030	90.0	0.020	127.5	0.012
232.5	0.020	270.0	0.030	300.0	0.050		
0.0	0.050	60.0	0.030	90.0	0.020	127.5	0.012
232.5	0.020	270.0	0.030	300.0	0.050		
0.0	0.050	60.0	0.030	90.0	0.020	115.0	0.012
245.0	0.020	270.0	0.030	300.0	0.050		
0.0	0.050	60.0	0.030	90.0	0.020	115.0	0.012
245.0	0.020	270.0	0.030	300.0	0.050		
0.0	0.050	60.0	0.030	85.0	0.020	115.0	0.012
245.0	0.020	275.0	0.030	300.0	0.050		
0.0	0.050	60.0	0.030	85.0	0.020	115.0	0.012
245.0	0.020	275.0	0.030	300.0	0.050		
0.0	0.050	60.0	0.030	85.0	0.020	115.0	0.012
245.0	0.020	275.0	0.030	300.0	0.050		
0.0	0.050	60.0	0.030	85.0	0.020	110.0	0.012
245.0	0.020	275.0	0.030	300.0	0.050		

0.0	0.050	60.0	0.030	78.0	0.020	105.0	0.012
255.0	0.020	282.0	0.030	300.0	0.050		
		7					
0.0	0.050	60.0	0.030	78.0	0.020	105.0	0.012
255.0	0.020	282.0	0.030	300.0	0.050		
		7					
CONS	1	50	295000000.				
CONS	1	50	0.3				
	50						
	50						
		1	1	1	1	1	

2
 SAMPLE PROBLEM NO. 2
 HEMISPHERICAL SHELL UNDER UNIFORM EXTERNAL PRESSURE.
 THE THICKNESS IS CONSTANT OVER THE ENTIRE SHELL AS WELL AS
 THE POISSON'S RATIO AND THE YOUNG'S MODULUS.
 IT IS ASSUMED TO BE CLAMPED AT THE BASE.
 ONLY THE OA HARMONIC IS NEEDED IN THE ANALYSIS.

1 8 30 29 1 1 SAM2
 (BLANK CARD)

0.0
 5.0 (BLANK CARD) 5.0

2 70.0
 15 20.0
 15 CONS 1 30 0.01
 CONS 1 30 30000000.
 CONS 1 30 0.3

30
 30 1 1 1 1
 1 ICONS

-100.0
 SAMPLE PROBLEM NO. 3
 HEMISPHERICAL SHELL UNDER UNIFORM INTERNAL PRESSURE APPLIED
 ON ELEMENTS 26 THROUGH 30.
 THE SHELL HAS THE SAME GEOMETRIC AND ELASTIC PROPERTIES AS
 THOSE USED IN THE SAMPLE PROBLEM NO. 2.
 THIS IS A RERUN OF THE SAMPLE PROBLEM NO. 2 UNDER A NEW LOADING.

1 8 30 29 1 1 SAM3RRUN
 26 ICONS 10.0

1
 SAMPLE PROBLEM NO. 5
 SPHERICAL SHELL WITH VARYING THICKNESS PROPERTIES.
 THE STRUCTURE IS UNDER A UNIFORM PRESSURE DISTRIBUTED
 OVER A CIRCULAR AREA OF THREE INCHES RADIUS.
 THE CENTER OF THE CIRCULAR AREA WHERE THE LOADING IS APPLIED
 IS LOCATED AT TEN INCHES FROM THE CENTER OF THE SHELL AND AT 45
 DEGREES FROM THE ORIGIN (THETA 0.0).

(BLANK CARD)
 4 4 8 20 29 1 4 3 3SAM5
 APOL 45.0 10.0 3.0 -10.0
 0.0 30.0 120.0 270.0

(BLANK CARD)
 20.0 20.0

3
 5 30.0
 5 10.0
 1C 50.0

1 2C 3 0.1 30.0 0.2 120.0 0.1
 C.0 0.1 20 30000000.
 CONS 1 20 0.3

2C 1 1 1 1
 2C

¹
 SAMPLE PROBLEM NO. 6
 SOLVE THE SHELL STRUCTURE OF SAMPLE PROBLEM NO. 5 WITH NEW LOADING.
 THE CENTER OF THE CIRCULAR AREA WHERE THE LOADING IS APPLIED IS
 MOVED TO THE ORIGIN (THETA = 0.0)
 CALCULATE DISPLACEMENTS AT 6 ANGLES ALONG THE CIRCUMFERENCE
 (BLANK CARD)

4	4	8	20	29	1	6	3	3SAM6RRUN
APOL	C.0	0.0	30.0	10.0	120.0	3.0	-10.0	
						180.0	225.0	270.0

1
SAMPLE PROBLEM NO. 7
THE STRUCTURE IS A COMBINATION OF A SPHERICAL CAP AND A CYLINDER.
THE SHELL IS SUBJECTED TO A UNIFORM INTERNAL PRESSURE.

1 8 50 29 3 1 SAM7
(BLANK CARD)

0.0
(BLANK CARD)
10.0 10.0
4

10 80.0
15 10.0 1
15 12.0
10 38.0

CONS 1 50 0.08
CONS 1 50 30000000.
CONS 1 50 0.3
25 26 50

25 1
26 1
50 1 1 1 1
1 1CONS 10.0

APPENDIX B
LISTING OF THE COMPUTER PROGRAM
FOR THE LINEAR ANALYSIS

```

COMMON /A3/ NDP,NET, IXX(17)
COMMON /A5/ EM(50),POISSO(50)
COMMON /A7/ NTAPE, ISCRAL, ISCR A3, NELEMS, NH, IPAGE, PROBLE
COMMON /B9/ IA, IB
COMMON /B14/ XTHETA(10), NUNKTH
COMMON /B18/ NAMES, HS
COMMON /B19/ IELRES(10), REST(50,8), NEREST
COMMON /B24/ RO1(50), PHY1(50), PHY2(50), Z1(50), Z2(50)
COMMON /B30/ NA, NB
COMMON /D10/ ISAVE, IDISPL
COMMON /D40/ PHPRMI(50), RMDI(50), CPDM(50), SPDM(50), PPPH(50),
IARLC(50), CHEKC(50,8), R02(50), AIL(50), A2L(50), A3L(50), AL157(50)
COMMON /G1/ APP0, THELD, RC, RL, PR
COMMON /G2/ NCE(50), NAET(50), THETA(50,51), THICKC(50,51)
COMMON /G4/ ONED(51,68)
DOUBLE PRECISION ONED
INTEGER PROBLE, RERUN, RRUN, APOL
INTEGER APP0
INTEGER REST
DATA RERUN/4HRRUN/
IDISPL=0
READ(5,101)NPROBL
101 FORMAT(I5)
DO 100 IPROBL=1, NPROBL
CALL TABLES
CALL STIFOR
IF(PROBLE.EQ.0)CALL DISPLA
IF(PROBLE.EQ.0)CALL DISPL1
CALL STRESS
100 CONTINUE
IF(ISAVE.EQ.0)PRINT 102, ISCR A3
102 FORMAT(46X, 41HS A V E T A P E L O G I C A L U N I T I 2)
STOP
END
SUBROUTINE TABLES
COMMON /A3/ NDP,NET, IXX(17)

```



```

COMMON /A5/ EM(50), POISSO(50)
COMMON /A7/ NTAPE, ISCRA1, ISCRA3, NELEMS, NH, IPAGE, PROBLE
COMMON /B9/ IA, IB
COMMON /B14/ XTHETA(10), NUNKTH
COMMON /B18/ NAMELS, HS
COMMON /B19/ IELRES(10), REST(50,8), NEREST
COMMON /B24/ R01(50), PHY1(50), PHY2(50), Z1(50), Z2(50)
COMMON /B30/ NA, NB
COMMON /D10/ ISAVE, IDISPL
COMMON /D40/ PHPRMI(50), RMDI(50), CPDM(50), SPDM(50), PPPH(50),
1ARLC(50), CHEKC(50,8), R02(50), AIL(50), A2L(50), A3L(50), AL157(50)
COMMON /G1/ APPO, THELO, RC, RL, PR
COMMON /G2/ NCE(50), NAET(50), THETA(50,51), THICKC(50,51)
DIMENSION THET(51), THI(51)
DIMENSION RK(4), RRRE(4)
DIMENSION TITL(40)
REAL IXED
INTEGER PROBLE, RERUN, RRUN, APOL
INTEGER APPO
INTEGER CONS
INTEGER TITL
INTEGER REST
INTEGER BLAN
INTEGER CONST
INTEGER SAND
DATA SAND/4HSAND/
DATA CONST/4HCONS/
DATA RREST1/4HIXED/, RREST2/3HREE/
DATA NAME/2H /
DATA BLAN/4H /
DATA REKUN/4HRRUN/
NTAPE=1
ISCRA1=2
IPAGE = 1
WRITE(6,604)IPAGE
IPAGE = IPAGE + 1
604 FORMAT(1H1,62X,4HPAGEI3//)

```

```
WRITE(6,605)
605 FORMAT(56X,20HGENERAL INFORMATION////)
DO 600 I = 1,20
  READ(5,601)(TITL(J),J=1,40)
601 FORMAT(40A2)
  IF(TITL(I).EQ.NAME)GO TO 603
  WRITE(6,602)(TITL(J),J=1,40)
602 FORMAT(26X,40A2//)
600 CONTINUE
603 WRITE(6,620)
620 FOKMAT(1H0,55X,20H*****//)
  WRITE(6,621)
621 FORMAT(59X,14H*****//)
  WRITE(6,622)
622 FORMAT(63X,6H*****//)
  READ(5,8)IA,IB,ISCRA3,NELEMS,NET,NEREST,NUNKTH,NAMES,HS,NA,NB,
  IDEN,PROBLE,ISAVE
  READ(5,9)APPO,THELO,RC,RL,PR
  9 FORMAT(A4,6X,4F10.0)
  READ(5,66)(XTHETA(I),I=1,NUNKTH)
  66 FORMAT(8F10.0)
  NH = IA + IC
  8 FORMAT(7I4,A4,F10.0,2I3,2A4,I1)
  IF(PROBLE.EQ.RERUN)WRITE(6,450)IDEN
  IF(PROBLE.EQ.RERUN)RETURN
  REWIND ISCRAL
  REWIND ISCRAB
  WRITE(6,401)IDEN
401 FORMAT(1H0,54X,22HP R O B L E M A4)
450 FORMAT(1H0,28X,31HR E R U N O F P R O B L E M A4,38H W I T H
  I N E W F O R C E I N P U T)
  WRITE(6,604)IPAGE
  IPAGE = IPAGE + 1
  WRITE(6,300)
300 FORMAT(54X,23HN O M E N C L A T U R E//)
  WRITE(6,301)
301 FORMAT(16X,40HZ ***** SHELL AXIAL COORDINATE//)
```

```

WRITE(6,302)
302 FORMAT(16X,54HR
1L)/)
WRITE(6,303)
303 FORMAT(16X,56HE
1MENT/)
WRITE(6,304)
304 FORMAT(16X,50HNU
1)
WRITE(6,305)
305 FORMAT(16X,63HPHI
1 REVOLUTION/)
WRITE(6,306)
306 FORMAT(16X,98HE(S)
1E SHELL ELEMENT ALONG THE MERIDIONAL DIRECTION/)
WRITE(6,307)
307 FORMAT(16X,103HE(THETA)
1HE SHELL ELEMENT ALONG THE CIRCUMFERENTIAL DIRECTION/)
WRITE(6,308)
308 FORMAT(16X,94HE(S-THETA)
1E SHELL ELEMENT ALONG THE NORMAL DIRECTION/)
WRITE(6,309)
309 FORMAT(16X,83HK(S)
1ENT SURFACE IN MERIDIONAL PLANE/)
WRITE(6,310)
310 FORMAT(16X,110HK(THETA)
1MENT SURFACE IN THE PLANE PERPENDICULAR TO MERIDIONAL PLANE/)
WRITE(6,311)
311 FORMAT(16X,52HK(S-THETA)
1/)
WRITE(6,312)
312 FORMAT(16X,84HN(S)
IRCUMFERENTIAL SECTION OF A SHELL/)
WRITE(6,313)
313 FORMAT(16X,79HN(THETA)
IRIDIONAL SECTION OF A SHELL/)
WRITE(6,314)

```

***** SHELL RADIAL COORDINATE(CYLINDRICA

***** MODULUS OF ELASTICITY OF SHELL ELE

***** POISSON,S RATIO OF SHELL ELEMENT/

***** ANGLE BETWEEN MERIDIAN AND AXIS OF

***** STRAIN OF THE MIDDLE SURFACE OF TH

***** STRAIN OF THE MIDDLE SURFACE OF T

***** STRAIN OF THE MIDDLE SURFACE OF TH

***** CURVATURE CHANGE OF THE SHELL ELEM

***** CURVATURE CHANGE OF THE SHELL ELE

***** TWIST OF THE SHELL ELEMENT SURFAC:

***** NORMAL FORCE PER UNIT LENGTH OF CI

***** NORMAL FORCE PER UNIT LENGTH OF ME

```
314 FORMAT(16X,108HN(S-THETA) ***** SHEARING FORCE PER UNIT LENGTH OF
1 CIRCUMFERENTIAL SECTION OR MERIDIONAL SECTION OF A SHELL/)
WRITE(6,315)
315 FORMAT(16X,86HM(S) ***** BENDING MOMENT PER UNIT LENGTH OF
1 CIRCUMFERENTIAL SECTION OF A SHELL/)
WRITE(6,316)
316 FORMAT(16X,81HM(THETA) ***** BENDING MOMENT PER UNIT LENGTH OF
1 MERIDIONAL SECTION OF A SHELL/)
WRITE(6,317)
317 FORMAT(16X,109HM(S-THETA) ***** TWISTING MOMENT PER UNIT LENGTH O
IF CIRCUMFERENTIAL SECTION OR MERIDIONAL SECTION OF A SHELL/)
WRITE(6,318)
318 FORMAT(16X,95HQ(S) ***** SHEARING FORCE NORMAL TO THE SHELL
1 PER UNIT LENGTH OF CIRCUMFERENTIAL SECTION/)
WRITE(6,319)
319 FORMAT(16X,90HQ(THETA) ***** SHEARING FORCE NORMAL TO THE SHELL
1 PER UNIT LENGTH OF MERIDIONAL SECTION/)
WRITE(6,620)
WRITE(6,621)
WRITE(6,622)
DO 800 I=1,NELEMS
DO 801 J=1,8
REST(I,J) = 0
801 CONTINUE
800 CONTINUE
510 READ(5,510)Z1(1),Z2(1),R01(1),R02(1),PHY1(1),PHY2(1)
FORMAT(6F12.5)
IF(PHY2(1).EQ.0.)GO TO 521
RCAP=0.
DO 522 J=2,NELEMS
522 READ(5,510)Z1(J),Z2(J),R01(J),R02(J),PHY1(J),PHY2(J)
GO TO 354
521 CONTINUE
READ(5,3)RCAP,RFILET,RMAX,ZMAX
J = 1
TCR = 0
CALL WEH2(PHY1,PHY2,J,TCR)
```

```

NNODES = NELEMS + 1
DO 54 I=1,NNODES
CALL WEH3(I,NP,CORD,RADIUS,PHY2,J,NNODES,RCAP,RFILET,RMAX,ZMAX)
IF(I.EQ.NNODES)GO TO 56
Z1(I) = CORD
R01(I) = RADIUS
IF(I.EC.1)GO TO 54
56 Z2(I - 1) = CORD
R02(I - 1) = RADIUS
54 CONTINUE
354 IELT=0
252 READ(5,2)NAMET,IEL1,IEL2,THIK,NC
2 FORMAT(A4,1X,2I5,10,0,15)
IF(NAMET,NE,CONST)GO TO 250
IF(IEL1,NE,0.AND,IEL2,NE,0)GO TO 781
IEL1=1
IEL2=1
781 DO 251 I=IEL1,IEL2
IELT=IELT+1
NCE(I)=1
NAET(I)=NAMET
THETA(I,1)=0.
THICKC(I,1)=THIK
251 CONTINUE
IF(IELT,NE,NELEMS)GO TO 252
GO TO 253
250 READ(5,98)(THET(I),THI(I),I=1,NC)
IF(IEL1,EQ,0)IEL1=IELT+1
IF(IEL2,EQ,0)IEL2=IELT+1
DO 780 J=IEL1,IEL2
NCE(J)=NC
NAET(J)=NAMET
DO 460 JE=1,NC
THETA(J,JE)=THET(JE)
THICKC(J,JE)=THI(JE)
460 CONTINUE
780 IELT=IELT+1
```

```

IF(IELT.NE.NELEMS)GO TO 252
253 CONTINUE
98 FORMAT(8F10.0)
IELT = 0
262 READ(5,2)NAM, IEL1, IEL2, EM(IEL1)
IF(NAM.NE.CONST)GO TO 260
DO 261 I=IEL1, IEL2
EM(I) = EM(IEL1)
261 CONTINUE
IELT = IEL2
GO TO 265
260 IELT = IELT + 1
EM(IELT) = EM(IEL1)
265 IF(IELT.NE.NELEMS)GO TO 262
IELT = 0
272 READ(5,2)NAM, IEL1, IEL2, POISSO(IEL1)
IF(NAM.NE.CONST)GO TO 270
DO 271 I=IEL1, IEL2
POISSO(I) = POISSO(IEL1)
271 CONTINUE
IELT = IEL2
GO TO 275
270 IELT = IELT + 1
POISSO(IELT) = POISSO(IEL1)
275 IF(IELT.NE.NELEMS)GO TO 272
3 FORMAT(8F10.0)
LINES = 0
DO 612 J=1, NELEMS
IF(J.NE.1.AND.LINES.LT.24)GO TO 609
WRITE(6,604)IPAGE
IPAGE = IPAGE + 1
LINES = 0
WRITE(6,606)
606 FORMAT(44X, 45HELASTIC AND GEOMETRIC PROPERTIES OF STRUCTURE /)
WRITE(6,607)
607 FORMAT(15X, 7HELEMENT, 8X, 4HNODE, 12X, 11HCOORDINATES, 18X, 17HELASTIC C
IONSTRESS /)

```

```

WRITE(6,608)
608 FORMAT(17X,3HNO,10X,4HNOS,10X,1HZ,13X,1HR,17X,1HE,12X,2HNU,18X,3
1HPHI///)
609 JJ = J + 1
WRITE(6,610)J,J,Z1(J),R01(J),EM(J),POISSO(J),PHYI(J),JJ,Z2(J),
1R02(J),PHY2(J)
LINES = LINES + 1
610 FORMAT(17X,I2,12X,I2,5X,E12.5,3X,E12.5,5X,E12.5,2X,E12.5,9X,E12.5/
1(31X,I2,5X,E12.5,2X,E12.5,40X,E12.5))
612 CONTINUE
1 FORMAT(10I5)
WRITE(6,400)IA,NET,IB
400 FORMAT(1H0,16X,23HNUMBER OF A HARMONICS =I3,9X,28HNUMBER OF SIMPSO
IN STATIONS =I3,8X,23HNUMBER OF B HARMONICS =I3)
IF(RCAP.EQ.0.)GO TO 501
WRITE(6,500)RCAP
500 FORMAT(48X,30HRADIUS OF SPHERICAL SHELL =F8.3)
501 WRITE(6,620)
WRITE(6,621)
WRITE(6,622)
READ(5,1)(IELRES(I),I=1,NEREST)
DO 200 I=1,NEREST
READ(5,1) J,(REST(J,IJ),IJ=1,8)
200 CONTINUE
DO 5555 IEL=1,NELEMS
NAMET=NAET(IEL)
NC=NCE(IEL)
THETA(IEL,NC+1)=360.
IF(IEL.EQ.1)LLINES=54
IIFLAG=0
NCCP=NC
JJFLAG=0
IC=0
717 NCP=NCCP - 4
IF(NCP}701,701,702
702 NCP1=IC*4 + 1
NCP2 = NCP1 + 3

```

```

IC = IC + 1
GO TO 703
701 NCP1 = IC*4 + 1
NCP2 = NC
IIFLAG = 1
703 IF(LLINES.LT.54)GO TO 704
LLINES = 0
WRITE(6,604)IPAGE
IPAGE = IPAGE + 1
JJFLAG = 0
WRITE(6,706)
IF(NAMES.NE.SAND)GO TO 7777
WRITE(6,7778)HS
7778 FORMAT(31X,44HSANDWICH TYPE SHELL WITH FACE SHEETS SPACED F7.3,1X,
13HIN ,15HFROM EACH OTHER/)
WRITE(6,7779)
7779 FORMAT(43X,46HTHICKNESSES OF EACH FACE SHEET ARE GIVEN BELOW//)
7777 CONTINUE
LLINES = LLINES + 8
706 FORMAT(56X,20HTABLE OF THICKNESSES/)
704 CONTINUE
IF(NC.NE.1)GO TO 716
WRITE(6,2000)IEL,THICKC(IEL,1)
2000 FORMAT(1H0,32X,12HELEMENT NO. 13,20X,21HCONSTANT THICKNESS =F10.5
1//)
LLINES = LLINES + 4
GO TO 2001
716 IF(JJFLAG.EQ.0)WRITE(6,710)IEL
710 FORMAT(60X,12HELEMENT NO. 13/)
LLINES = LLINES + 2
IF(NAMET.NE.BLAN)GO TO 2002
WRITE(6,2003)(THETA(IEL, IY), THETA(IEL, IY+1), IY=NCPI, NCP2)
WRITE(6,709)(THICKC(IEL, IY), IY=NCPI, NCP2)
GO TO 2004
2003 FORMAT(1H0,5HTHETA,9H(DEGREES),3X,4(3X,F10.5,4H TO ,F10.5))
2002 WRITE(6,708)(THETA(IEL, IY), IY=NCPI, NCP2)
WRITE(6,709)(THICKC(IEL, IY), IY=NCPI, NCP2)

```



```

708 FORMAT(1H0,5HTHETA,9H(DEGREES),12X,F10.5,3(17X,F10.5))
709 FORMAT(1X,9HTHICKNESS,4(17X,F10.5))
2004 LINES = LINES + 3
      NCCP= NCP
      JFLAG = 1
      IF(IIFLAG.EQ.0)GO TO 717
2001 CONTINUE
5555 CONTINUE
      WRITE(6,604)IPAGE
      IPAGE = IPAGE + 1
      WRITE(6,900)
900  FORMAT(56X,20HBOUNDARY  CONDITIONS////)
      WRITE(6,901)
901  FORMAT(50X,32HDISPLACEMENT COMPONENTS OF NODES////)
      WRITE(6,902)
902  FORMAT(23X,8HNODE NO.,10X,5HAXIAL,12X,10HTANGENTIAL,11X,6HRADIAL,
112X,7HANGULAR////)
      DO 903 IRE=1,NELEMS
      JISE = 0
      DO 904 ISE=1,4
      RK(ISE)= 0.
      IF(REST(IP, ISE).EQ.0)GO TO 904
      RK(ISE)=1.
      JISE=1
904  CONTINUE
      IF(JISE.EQ.0)GO TO 903
      DO 905 ISE=1,4
      RRRE(ISE)=RREST2
      IF(RK(ISE).EQ.1.)RRRE(ISE)=RREST1
905  CONTINUE
      WRITE(6,906)IRE,(RRRE(ISE), ISE=1,4)
906  FORMAT(26X,I2,13X,1HF,A4,14X,1HF,A4,13X,1HF,A4,/)
903  CONTINUE
      JISE = 0
      DO 907 ISE=1,4
      RK(ISE)=0.
      ISEE=ISE + 4

```

```
IF(REST(NELEMS,ISEE).EQ.0)GO TO 907
RK(ISE)=1.
JISE = 1
907 CONTINUE
IF(JISE.EQ.0)GO TO 908
DO 909 ISE=1,4
RRRE(ISE)=RREST2
IF(RK(ISE).EQ.1.)RRRE(ISE)=RREST1
909 CONTINUE
INEL = NELEMS + 1
WRITE(6,906)INEL,(RRRE(ISE), ISE=1,4)
908 CONTINUE
WRITE(6,910)
910 FORMAT(1H0,47X,36HALL OTHER NODES ARE FREE TO DISPLACE//)
WRITE(6,620)
WRITE(6,621)
WRITE(6,622)
RETURN
END
SUBROUTINE WEH2(P1,P2,J,TCR)
COMMON /A7/ NTAPE,ISCR1,ISCR3,NELEMS,NH,IPAGE,PROBLE
DIMENSION P1(50),P2(50)
READ (5,21) NPARTS
READ (5,21) NE1,T1,NCYLIN
21 FORMAT (I5,F10.0,I5)
NP=1
XE1=NE1
H=T1/XE1
P1(1)=90.0
P2(1)=90.0-H
DO 31 I=2,NE1
XI=I
P1(I)=P2(I-1)
31 P2(I)=90.0-H*XI
IF (NPARTS-1) 35,35,32
32 READ (5,21) NE2,T2,NCYLIN
IF (NCYLIN) 2,1,2
```

```
1 NP=NP+1
  XE2=NE2
  H=T2/XE2
  DO 33 I=1,NE2
    XI=I
    J=NE1+I
    P1(J)=P2(J-1)
    P2(J)=P2(NE1)-H*XI
    IF (ABS(P2(J)-TCR)-0.005) 37,37,33
37 P2(J)=TCR
33 CONTINUE
  IF (NP-NPARTS) 34,35,35
34 NE1=NE1+NE2
  GO TO 32
2 J=NELEMS-NE2+1
  P2(J-1)=T2
  DO 101 K=J,NELEMS
    P1(K)=0.0
101 P2(K)=0.0
  GO TO 36
35 J=NELEMS+2
36 RETURN
  END
SUBROUTINE WEH3(I,N,C,R,P,J,NNODES,RCAP,RFILET,RMAX,ZMAX)
  DIMENSION P(50)
  TCR=ARCOS((RMAX-RFILET)/(RCAP-RFILET))
  N=I
  IF (I-1) 1,1,2
1 C=0.0
  R=1.0E-6
  K=J
  CKEEP=RCAP-(RCAP-RFILET)*SIN(TCR)
  GO TO 11
2 IF (I-J) 3,4,4
3 T=P(I-1)/57.2957795
  IF (I-TCR) 6,5,5
5 R=RCAP*COS(T)
```

```
C=RCAP-RCAP*SIN(T)
GO TO 11
6 R=(RMAX-RFILET)+RFILET*COS(T)
  C=RCAP-(RCAP-RFILET)*SIN(TCR)-RFILET*SIN(T)
GO TO 11
4 R=RMAX
  IF (I-K) 7,7,9
  7 IF (I-NNODES) 8,9,9
  8 READ (5,21) NE1,S1
  21 FORMAT (I5,F10.0)
  XE1=NE1
  H=S1/XE1
  9 XI=I-K
  C=CKEEP+H*XI
  IF (I-K-NE1+1) 11,10,10
  10 CKEEP=CKEEP+S1
  K=I+1
  11 RETURN
END
SUBROUTINE STIFOR
COMMON /A1/ G(30),GA(30),P(75),R(75),S(75),O(75)
COMMON /A2/ ONE(68),TWO(68)
COMMON /A3/ NDP,NET,IXX(17)
COMMON /A5/ EM(50),POISSO(50)
COMMON /A7/ NTAPE,ISCR1,ISCR3,NELEMS,NH,IPAGE,PROBLE
COMMON /B1/ AL(157)
COMMON /B2/ AR (30), AS(30), PHP(30), SINE(30), COSINE(30)
COMMON /B3/ PHPP
COMMON /B4/ F(8,8)
COMMON /B5/ ISIMP
COMMON /B6/ AK(8,8)
COMMON /B7/ PHIMAT(8,8)
COMMON /B8/ CHECK (8,8)
COMMON /B9/ IA,IB
COMMON /B14/ XTETHA(10),NUNKTH
COMMON /B15/ IELL,NDPP,IXXX(17),LINEL,ICON,ICOO,NSTOP
COMMON /B18/ NAMES,HS
```

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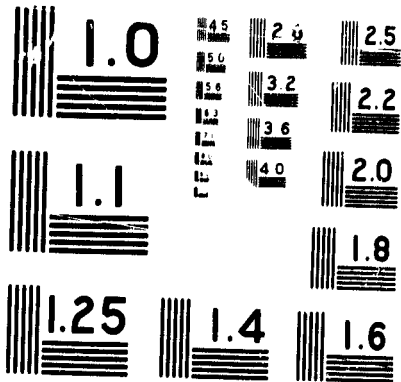
COMMON /B19/ IELRES(10),REST(50,8),NEREST
COMMON /B24/ R01(50),PHY1(50),PHY2(50),Z1(50),Z2(50)
COMMON /B30/ NA,NB
COMMON /C40/ IPRI
COMMON /D10/ ISAVE, IDISPL
COMMON /D40/ PHPRMI(50),RMDI(50),CPDM(50),SPDM(50),PPPH(50),
1ARLC(50),CHEKC(50,8,8),R02(50),AIL(50),A2L(50),A3L(50),AL157(50)
COMMON /G1/ APP0,THELO,RC,RL,PR
COMMON /G2/ NCE(50),NAET(50),THETA(50,51),TRI,CKC(50,51)
COMMON /G4/ ONED(51,68)
DOUBLE PRECISION FKK(9316),AAK(8,8)
DOUBLE PRECISION ONED
DIMENSION RK(4),RRRE(4)
DIMENSION TA(2,50)
INTEGER REST
INTEGER PROBLE,RERUN,RRUN,APOL
INTEGER APP0
INTEGER END
INTEGER SAND
DATA RERUN/4HRRUN/
DATA SAND/4HSAND/
DATA END/3HEND/
IPRI=0
ISIMP = 0

```

```

C EM= MODULUS OF ELASTICITY OF SHELL ELEMENT
C POISSO = POISSON'S RATIO OF SHELL ELEMENT
C ISOTROPIC SHELL ELEMENT ONLY
C IELRES IDENTIFIES WHICH NODE IS RESTRAINED
C REST NOT EQUAL TO ZERO RESTRAINS NODE DISPLACEMENT COMPONENT
C IA = NO. OF A HARMONICS
C IB = NO. OF B HARMONICS
C NTAPE = BIN TAPE WITH STIFFNESS MATRIX
C NTAPEF = BIN TAPE WITH FORCE MATRIX
C ISCRAL: ISCRAL2=BIN SCRATCH TAPES
C NELEMS = NO. OF ELEMENTS
C NEREST = NO. OF RESTRAINED NODES
C TOTAL NUMBER OF A AND B HARMONICS - NH

```



MICROCOPY RESOLUTION TEST CHART
NATIONAL BUREAU OF STANDARDS - 1963

```

N = 4*NH
DO 40 I=1,N
ONE(I) = 0.0
TWO(I) = C.0
40 CONTINUE
LINEL = 0
ICOO=1
IF(PROBLE.EQ.RERUN)GO TO 613
REWIND NTAPE
C      NUMBER OF ELEMENTS IN EACH TRIANGLE OF FKK - NN
IF(ISAVE.EQ.0)WRITE(ISCRA3)NEREST,(IELRES(I),I=1,NEREST),
1((REST(J,K),K=1,8),J=1,NELEMS),(EM(IS),POISSO(IS),IS=1,NELEMS)
NN = N*(N+1)/2
C      INDEX OF FIRST ELEMENT OF FKK SQUARE - N1
N1 = 2*NN + 1
C      INDEX OF LAST ELEMENT OF FKK SQUARE - N2
N2 = 2*NN + N*N
C      INITIALIZE UPPER TRIANGLE OF FKK
DO 10 I=1,NN
FKK(I) = 0.0
10 CONTINUE
DO 34 IEL=1,NELEMS
OBTAIN AR, AS, PHP, SINE, PHPP, ANL COSINE FOR EACH ELEMENT
CALL ELEMCA (R01(IEL),R02(IEL),PHY1(IEL),PHY2(IEL),Z1(IEL),
1Z2(IEL),DS,IEL)
C      CALL CRETAL TO OBTAIN AL ARRAY
CALL CRETAL(DS,IEL)
NAMET=NAET(IEL)
NC=NCE(IEL)
IF(ISAVE.EQ.0)WRITE(ISCRA3)NCE(IEL),NAET(IEL)
DO 300 I=1,NC
300 THETA(IEL,I)=THETA(IEL,I)/57.2957795
THETA(IEL,NC+1)=6.28318531
IF(ISAVE.EQ.0)WRITE(ISCRA3)(THETA(IEL,I),THICKC(IEL,I),I=1,NC)
THICKC(IEL,NC+1)=THICKC(IEL,1)
PI=3.14159265
IFO=NA+NB+1
```

```
DO 731 I1=1, IFO
TA(1, I1)=0.
731 TA(2, I1)=0.
   IEXPT=1
DO 701 I2=1, 2
SUM1=0.
DO 700 I1=1, NC
700 SUM1=SUM1+THICKC(IEL, I1)**IEXPT*(THETA(IEL, I1+1)-THETA(IEL, I1))
   TA(I2, 1)=SUM1/(2.*PI)
   IF(NC.EQ.1)GO TO 730
   IF(NA.EQ.0)GO TO 740
DO 702 I1=1, NA
SUM2=0.
HII=I1
DO 703 I3=1, NC
X=THETA(IEL, I3)
XX=THETA(IEL, I3+1)
703 SUM2=SUM2+THICKC(IEL, I3)**IEXPT*(SIN(HII*XX)-SIN(HII*X))
702 TA(I2, I1+1)=SUM2/(HII*PI)
740 IF(NB.EQ.0)GO TO 730
DO 704 I1=1, NB
SUM3=0.
HII=I1
DO 705 I3=1, NC
X=THETA(IEL, I3)
XX=THETA(IEL, I3+1)
705 SUM3=SUM3+THICKC(IEL, I3)**IEXPT*(COS(HII*XX)-COS(HII*X))
   I4=NA+I1+1
704 TA(I2, I4)=-SUM3/(HII*PI)
730 IF(NAMES.EQ.SAND)GO TO 706
   IEXPT=3
701 CONTINUE
706 IF(NAMES.NE.SAND)GO TO 715
DO 717 I1=1, IFO
717 TA(2, I1)=TA(1, I1)
715 CONTINUE
DO 32 I=1, NH
```



```
C
DO 32 J=I,NH
    FOLLOWING DETERMINES WHICH VALUES FOR AM AND AN ARE TO BE USED
    IF (I.GT. IA.OR. J.GT. IA) GO TO 12
    M1= I - 1
    N3= J - 1
    MI=M1+1
    NI=N3+1
    KEY1=0
    GO TO 16
12 IF (I.LE. IA) GO TO 14
    M1= - (I - IA - 1)
    N3= - (J - IA - 1)
    MI=-M1+1
    NI=-N3+1
    KEY1=2
    GO TO 16
14 M1= I - 1
    N3= - (J - IA - 1)
    MI=M1+1
    NI=-N3+1
    KEY1=1
16 HM = M1
    HN = N3
    SIMP1=0.
    SIMP2=0.
    SIMP3=0.
    SIMP4=0.
    IF((MI-1).EQ.(NI-1).AND.(MI-1).EQ.0.AND. KEY1.NE.1)GO TO 742
    IF((MI-1).EQ.(NI-1).AND.(MI-1).NE.0.AND. KEY1.NE.1)GO TO 744
    GO TO 741
742 IF(KEY1.EQ.0)GO TO 760
    SIMP3=TA(1,1)*2.0*PI
    SIMP4=TA(2,1)*2.0*PI
    GO TO 741
760 SIMP1=TA(1,1)*2.0*PI
    SIMP2=TA(2,1)*2.0*PI
    GO TO 741
```

```
744 SIMP1=TA(1,1)*PI
    SIMP2=TA(2,1)*PI
    SIMP3=SIMP1
    SIMP4=SIMP2
741 IF(NC.EQ.1)GO TO 311
    I2=NA
    I3=1
    IF(I2.NE.0)GO TO 747
    I3=2
    I2=NB
747 DO 745 I1=1,I2
    IF((NI+MI).NE.(I1+2).AND.IABS(NI-MI).NE.I1)GO TO 745
    IF(KEY1.NE.1)GO TO 750
    CALL INTE(MI,I1+1,NI,I3,SIM1)
    CALL INTE(I1+1,NI,MI,I3,SIM2)
    IF(I3.EQ.1)GO TO 751
    ASIM1=SIM1
    SIM1=SIM2
    SIM2=ASIM1
    GO TO 751
750 CALL INTE(MI,NI,I1+1,KEY1+I3-1,SIM1)
    CALL INTE(MI,NI,I1+1,1-KEY1+I3,SIM2)
751 I11=NA*(I3-1)+I1+1
    SIMP1=SIMP1+TA(1,I11)*SIM1
    SIMP2=SIMP2+TA(2,I11)*SIM1
    SIMP3=SIMP3+TA(1,I11)*SIM2
    SIMP4=SIMP4+TA(2,I11)*SIM2
745 CONTINUE
    IF(I3.EQ.2)GO TO 311
    IF(NB.EQ.0)GO TO 311
    I3=2
    I2=NB
    GO TO 747
311 IF(NAMES.NE.SAND)GO TO 314
    SIMP1=2.*SIMP1
    SIMP3=2.*SIMP3
    SIMP2=SIMP2*6.0*HS**2
```

```

SIMP4=SIMP4*6.0*HS**2
314 E1=EM( IEL)
FNU1=POISSO( IEL)
E2=E1
GXX=E1/(2.*(1.+FNU1))
CONST1=E1/(1.-FNU1*FNU1)
CC1=CONST1*SIMP1
CC=2.*CONST1*FNU1*SIMP1
DD1=CONST1*SIMP2/12.
DD=CONST1*FNU1*SIMP2/6.
GG1=GXX*SIMP3
GG2=GXX*SIMP4/12.0
C CALL SUBROUTINE TO GET FL - AN 8X8 MATRIX
CALL CREATL(HM,HN,CCI,CC,DD1,DD,GG1,GG2)
C CALL SUBROUTINE TO PRE AND POST MULTIPLY FL TO GET AK
CALL MMPLT2
C*****INTRODUCE BOUNDARY CONDITIONS
DO 110 IR=1,NEREST
IF( IELRES(IR).NE. IEL)GO TO 110
DO 101 JR=1,8
IF( REST( IEL, JR).EQ.0)GO TO 101
DO 102 JRR=1,8
AK( JR, JRR)=0.
AK( JRR, JR)=0.
102 CONTINUE
IF( I.EQ. J)AK( JR, JR)=1.
101 CONTINUE
110 CONTINUE
DO 481 L=1,8
DO 482 M=1,8
AAK( L, M)=AK( L, M)
482 CONTINUE
481 CONTINUE
C THE FOLLOWING TAKES EACH ELEMENT OF AK AND PUTS IT IN
C THE PROPER LOCATION OF FKK
DO 30 L=1,8
DO 30 M=1,8
```

```
IF (I.NE.J) GO TO 18
IF (L.GT.M) GO TO 30
18 IF (L.GT.4.OR.M.GT.4) GO TO 20
   ELEMENT IS IN UPPER LEFT 4X4 OF AK MATRIX.
   THUS, STORE IN 1ST TRIANGLE OF FKK(I1), I1=1,NN
C     INDEX = 4*(I-1) + L
C     JNDEX = 4*(J-1) + M
C     SINGLE INDEX OF FKK - KNDEX
C     KNDEX = JNDEX*(JNDEX-1)/2 + INDEX
C     ADD 2CD TRIANGLE OF PREVIOUS ELEMENT TO FIRST OF THIS ELEMENT
C     FKK(KNDEX)=FKK(KNDEX)+AAK(L,M)
C     GO TO 30
20 IF (L.LE.4.OR.M.LE.4) GO TO 22
   ELEMENT IS IN LOWER RIGHT 4X4 OF AK MATRIX
   THUS, STORE IN 2CD TRIANGLE OF FKK(I1), I1=NN+1,2*NN
C     INDEX = 4*(I-2) + L
C     JNDEX = 4*(J-2) + M
C     SINGLE INDEX OF FKK - KNDEX
C     KNDEX = NN + JNDEX*(JNDEX-1)/2 + INDEX
C     GO TO 28
22 IF (M.GT.4) GO TO 24
   ELEMENT IS IN LOWER LEFT 4X4 OF AK MATRIX.
   IF ALONG I=J DIAGONAL, DO NOT NEED
   OTHERWISE, TAKE TRANSPOSE AND STORE IN SQUARE OF
   FKK(I1), I1=2*NN+1,2*NN+N*N
C     INDEX = 4*(J-1) + M
C     JNDEX = 4*(I-2) + L
C     GO TO 26
24 INDEX = 4*(I-1) + L
   JNDEX = 4*(J-2) + M
   SINGLE INDEX OF FKK - KNDEX
C     26 KNDEX = 2*NN + (JNDEX-1)*N + INDEX
28 FKK(KNDEX)=AAK(L,M)
30 CONTINUE
32 CONTINUE
DO 903 I=1,N
I1=(I+I)/2
```

```
IF(FKK(I1).EQ.0.)FKK(I1)=1.0
903 CONTINUE
C WRITE 1ST TRIANGLE OF FKK ON TAPE AS A RECORD
WRITE(NTAPE)(FKK(I),I=1,NN)
C WRITE SQUARE OF FKK ON TAPE AS A RECORD
WRITE(NTAPE)(FKK(I),I=N1,N2)
C STORE 2CD TRAIANGLE IN FIRST
DO 33 I=1,NN
I1=I+NN
FKK(I) = FKK(I1)
33 CONTINUE
C WRITE 2CD TRIANGLE ON TAPE ONLY IF LAST ELEMENT
IF(IEL.NE.NELEMS)GO TO 906
DO 907 I=1,N
I1=(I+I)/2
IF(FKK(I1).EQ.0.)FKK(I1)=1.0
907 CONTINUE
C WRITE(NTAPE)(FKK(I),I=1,NN)
CALL SUBROUTINE TO CREATE RIGHT HAND SIDE
906 CALL FORCES(IEL)
34 CONTINUE
IF(PROBLE.NE.RERUN)GO TO 612
613 REWIND ISCR3
READ(ISCR3) NEREST,(IELRES(I),I=1,NEREST),((REST(J,K),K=1,8),
1 J=1,NELEMS),(EM(IS),POISSO(IS),IS=1,NELEMS)
DO 611 K=1,NELEMS
READ(ISCR3) PHPRIM,RMID,CPMD,SPMD,PHPP,ARCL,((CHECK(I,J),
I=1,8),J=1,8),R02(K)
PHPRMI(K)=PHPRIM
RMDI(K)=RMID
CPDM(K)=CPMD
SPDM(K)=SPMD
PPPH(K)=PHPP
ARLC(K)=ARCL
DO 4937 IST1=1,8
DO 4938 IST2=1,8
CHECKC(K,IST1,IST2)=CHECK(IST1,IST2)
```

```
4938 CONTINUE
4937 CONTINUE
  READ(ISCRA3)AIL(K),A2L(K),A3L(K),ALL157(K)
  READ(ISCRA3)NCE(K),NAET(K)
  NC=NCE(K)
  NAMET=NAET(K)
  READ(ISCRA3)(THETA(K,I),THICKC(K,I),I=1,NC)
  THETA(K,NC+1)=6.28718531
  THICKC(K,NC+1)=THICKC(K,1)
  CALL FORCES (K)
611 CONTINUE
612 DO 410 IR=1,NEREST
  IF(IELRES(IR).NE.NELEMS)GO TO 410
  DO 401 JR=5,8
  IF(REST(NELEMS, JR).EQ.0)GO TO 401
  DO 402 JJR=1,NH
  KR=JR+(JJR-1)*4-4
  TWO(KR)=0.
402 CONTINUE
401 CONTINUE
410 CONTINUE
  NNE=NELEMS+1
  DO 480 KK=1,N
  ONED(NNE, KK)=TWO(KK)
  C THE TAPE CREATED IN THIS PROGRAM IN USED FOR INPUT
  C FOR THE SOLUTION PROGRAM
  RETURN
  END
  SUBROUTINE ELEMCA(R1,R2,P1,P2,Z1,Z2,DSL,IEL)
  COMMON /A3/ NDP,NET,IXX(17)
  COMMON /A7/ NTAPE,ISCRA1,ISCRA3,NELEMS,NH,IPAGE,PROBLE
  COMMON /B2/ AR (30), AS(30), PHP(30), SINE(30), COSINE(30)
  COMMON /B3/ PHPP
  COMMON /B7/ PHIMAT (8,8)
  COMMON /B8/ CHECK(8,8)
  COMMON /D10/ ISAVE,IDISPL
  COMMON /D40/ PHPRMI(50),RMDI(50),CPDM(50),SPDM(50),PPPH(50),
```

```
1ARLC(50),CHEKC(50,8,8),R02(50),AIL(50),A2L(50),A3L(50),AL157(50)
DIMENSION PH(30),AM(8,8)
DELZ = ABS (Z2-Z1)
DELR=R2-R1
AL=SQRT(DELR**2+DELZ**2)
DELP=(P1-P2)/.572957795E+02
P1=P1/.572957795E+02
P2=P2/.572957795E+02
IF(DELP)14,15,14
ARCL=AL
15 GO TO 10
14 ARO=AL/(2.0*SIN(DELP/2.0))
ARCL=ARO*DELP
C COMPUTE TRANSFORMATION MATRIX
10 DO 102 I=1,8
DO 102 J=1,8
102 PHIMAT(I,J)=0.0
DO 105 I=1,3,2
PHIMAT(I,I)=COS(P1)
PHIMAT(I+1,I+1)=1.0
PHIMAT(3,I)=SIN(P1)
PHIMAT(I,3)=-PHIMAT(3,I)
DO 106 I=5,7,2
PHIMAT(I,I)=COS(P2)
PHIMAT(I+1,I+1)=1.0
PHIMAT(7,I)=SIN(P2)
PHIMAT(I,7)=-PHIMAT(7,I)
C COMPUTE M MATRIX
101 DO 101 I=1,8
DO 101 J=1,8
AM(I,J)=0.0
AM(1,3)=1.0
AM(2,4)=1.0
AM(5,1)=1.0
AM(7,2)=1.0
AL1=1.0/ARCL
AL2=AL1**2
```

0680
0690
0700
0710
0720
0730
0740
0750
0760
0770
0780
0790
0800
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0900
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0930
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0950
0960
0970
0980
0990
1000
1010

```
AL3=AL2*AL1
AM(6,1)=-AL1
AM(8,2)=-AL1
AM(3,3)=-3.0*AL2
AM(4,3)=2.0*AL3
AM(3,4)=-2.0*AL1
AM(4,4)=AL2
AM(6,5)=AL1
AM(8,6)=AL1
AM(3,7)=3.0*AL2
AM(4,7)=-2.0*AL3
AM(3,8)=-AL1
AM(4,8)=AL2
C COMPUTE ELEMENTS OF L MATRIX
IF(DELP)8,9,8
9 DO 7 I=1,NET
  PH(I)=PI
  PHP(I)=0.0
  COSINE(I)=COS(PI)
  SINE(I)=SIN(PI)
  CPMD= COSINE(I)
  SPFD=SINE(I)
  RMID=R2
  AS(I)=0.0
  IF(R1)24,40,24
    4C R1 = .1E-06
  24 CONTINUE
  ARCL=AL
  NET1=NET-1
  DP=AL/FLOAT(NET1)
  D=L=DP
  DO 52 I=2,NET
    J=I-1
    AS(I)=FLOAT(J)*DP
    AR(I)=R1
    DO 6 I=2,NET
      6 AR(I)=R1+AS(I)*SINE(I)
```

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1080
1090
1100
1110
1120
1130
1140
1150
1190
1200
1210
1220
1230
1240

1400
1410
1420
1430

1440
1450
1460
1470
1480
1490


```
PHPR1=0.0
PHPR=0.0
GO TO 19
CONTINUE
8 IF(R1)29,30,29
28 30 R1 = .1E-06
29 CONTINUE
NET1=NET-1
RZ = (R2-R1)/(Z2-Z1)
PHL=ATAN(RZ)
A2=(6.0*PHL-4.0*P1-2.0*P2)/ARCL
A3=(3.0*P1+3.0*P2-6.0*PHL)/ARCL**2
AM(4,1) = -A2/ARCL**2
AM(4,5) = - (A2+2.*A3*ARCL)/ARCL**2
AM(2,1) = -A2
AM(3,1) = 2.*A2/ARCL
AM(3,5) = (A2+2.*A3*ARCL)/ARCL
DSL=ARCL/FLOAT(NET1)
42 AS(1)=0.0
43 PH(1)=P1
SINE(1)=SIN(P1)
COSINE(1)=COS(P1)
PHP(1)=A2
NETP=(NET+1)/2
DO 25 I=2,NET
AS(I)=AS(I-1)+DSL
PH(I)=P1+A2*AS(I)+A3*AS(I)**2
PHP(I)=A2+2.0*A3*AS(I)
SINE(I)=SIN(PH(I))
COSINE(I)=COS(PH(I))
IF(I.NE.NETP)GO TO 25
SPMD = SINE(I)
CPMD = COSINE(I)
25 CONTINUE
AR(1)=R1
NETL=NET1/2
PHPRIM=PHP(NETP)
```

1500
1510

1530
1550

1730
1740

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1780

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1890
1900
1910
1920

1940
1960
1970

```
PHPP=2.0*A3
DO 22 I=2,NETP
  AR(I)=AR(I-1)+(SINE(I)+SINE(I-1))*DSL/2.0
  RMID = AR(NETP)
  AR(NET)=R2
DO 53 I=1,NETL
  NN=NET-I
  AR(NN)=AR(NN+1)-((SINE(NN+1)+SINE(NN))*DSL/2.0
19 DO 1000 I=1,8
DO 1000 J=1,8
CHECK(I,J) = 0.0
DO 1000 K=1,8
CHECK(I,J) = AM(I,K)*PHIMAT(J,K) + CHECK(I,J)
1000 CONTINUE
K=IEL
PHPRMI(K)=PHPRIM
RMDI(K)=RMID
CPDM(K)=CPMD
SPDM(K)=SPMD
PPPH(K)=PHPP
ARLC(K)=ARCL
DO 4937 IST1=1,8
DO 4938 IST2=1,8
CHECKC(K,IST1,IST2)=CHECK(IST1,IST2)
4938 CONTINUE
4937 CONTINUE
IF(ISAVE.EQ.0)WRITE(ISCRA3)PHPRIM,RMID,CPMD,SPMD,PHPP,ARCL,
1((CHECK(I,J),I=1,8),J=1,8),R2
RETURN
END
SUBROUTINE CRETAL (DS,IEL)
COMMON /A7/ NTAPE,ISCRA1,ISCRA3,NELEMS,NH,IPAGE,PROBLE
COMMON /B1/ AL(157)
COMMON /B2/ AR (30), AS(30), PHP(30), SINE(30), COSINE(30)
COMMON /B5/ISIMP
COMMON /A1/ G(30),GA(30), P(75),R(75),S(75),O(75)
COMMON /A3/ NDP,NET,IXX(17)
```

1980
1990
2000

2010
2020
2030
2040

2070
2080

```
COMMON /D10/ ISAVE, IDISPL
COMMON /D40/ PHPRMI(50), RMDI(50), CPDM(50), SPDM(50), PPPH(50),
IARLC(50), CHEKC(50,8,8), R02(50), AIL(50), A2L(50), A3L(50), AL15(50)
C INTEGRATION OF L(I,J)'S
DO 1 I=1,NET
  1 GA(I)=AR(I)
  ISIMP = 1
  AL(1) = SIMP (DS)
DO 2 I=1,NET
  2 G(I)=GA(I)*AS(I)
  AL(2) = SIMP (DS)
DO 3 I=1,NET
  3 G(I)=G(I)*AS(I)
  AL(3) = SIMP (DS)
DO 4 I=1,NET
  4 GA(I)=GA(I)*PHP(I)
  ISIMP = 1
  AL(4) = SIMP (DS)
DO 5 I=1,NET
  5 G(I)=GA(I)*AS(I)
  AL(5) = SIMP (DS)
DO 6 I=1,NET
  6 G(I)=G(I)*AS(I)
  AL(6) = SIMP (DS)
DO 7 I=1,NET
  7 G(I)=G(I)*AS(I)
  AL(7) = SIMP (DS)
DO 8 I=1,NET
  8 GA(I)=GA(I)*PHP(I)
  ISIMP = 1
  AL(8) = SIMP (DS)
DO 9 I=1,NET
  9 G(I)=GA(I)*AS(I)
  AL(9) = SIMP (DS)
DO 10 I=1,NET
  10 G(I)=G(I)*AS(I)
  AL(10) = SIMP (DS)
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DO 11 I=1,NET
11 G(I)=G(I)*AS(I)
   AL(11) = SIMP (DS)
DO 12 I=1,NET
12 G(I)=G(I)*AS(I)
   AL(12) = SIMP (DS)
DO 13 I=1,NET
13 G(I)=G(I)*AS(I)
   AL(13) = SIMP (DS)
DO 14 I=1,NET
14 G(I)=G(I)*AS(I)
   AL(14) = SIMP (DS)
DO 15 I=1,NET
15 GA(I)=PHP(I)*PHP(I)*SINE(I)
   ISIMP = 1
   AL(42) = SIMP (DS)
DO 16 I=1,NET
16 G(I)=GA(I)*AS(I)
   AL(43) = SIMP (DS)
DO 17 I=1,NET
17 GA(I)=PHP(I)*SINE(I)
   ISIMP = 1
   AL(37) = SIMP (DS)
DO 18 I=1,NET
18 G(I)=GA(I)*AS(I)
   AL(38) = SIMP (DS)
DO 19 I=1,NET
19 G(I)=G(I)*AS(I)
   AL(39) = SIMP (DS)
DO 20 I=1,NET
20 G(I)=G(I)*AS(I)
   AL(40) = SIMP (DS)
DO 21 I=1,NET
21 G(I)=G(I)*AS(I)
   AL(41) = SIMP (DS)
DO 22 I=1,NET
22 GA(I)=PHP(I)
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ISIMP = 1
AL(25) = SIMP (DS)
DO 23 I=1,NET
23 G(I)=GA(I)*AS(I)
AL(26) = SIMP (DS)
DO 24 I=1,NET
24 G(I)=G(I)*AS(I)
AL(27) = SIMP (DS)
DO 25 I=1,NET
25 G(I)=G(I)*AS(I)
AL(28) = SIMP (DS)
DO 26 I= 1,NET
26 G(I)=G(I)*AS(I)
AL(29) = SIMP (DS)
DO 27 I=1,NET
27 G(I)=GA(I)*COSINE(I)
AL(30) = SIMP (DS)
DO 28 I=1,NET
28 G(I)=G(I)*AS(I)
AL(31) = SIMP (DS)
DO 29 I=1,NET
29 G(I)=G(I)*AS(I)
AL(32) = SIMP (DS)
DO 30 I=1,NET
30 G(I)=G(I)*AS(I)
AL(33) = SIMP (DS)
DO 31 I=1,NET
31 G(I)=G(I)*AS(I)
AL(34) = SIMP (DS)
DO 32 I=1,NET
32 G(I) = G(I) * AS(I)
AL(35) = SIMP (DS)
DO 155 I = 1,NET
155 G(I) = G(I) * AS(I)
AL(36) = SIMP (DS)
DO 33 I=1,NET
33 GA(I)=1,
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ISIMP = 1
AL(15) = SIMP (DS)
DO 34 I=1,NET
34 G(I)=GA(I)*AS(I)
AL(16) = SIMP (DS)
DO 35 I=1,NET
35 G(I)=GA(I)*COSINE(I)
AL(17) = SIMP (DS)
DO 36 I=1,NET
36 G(I)=G(I)*AS(I)
AL(18) = SIMP (DS)
DO 37 I=1,NET
37 G(I)=G(I)*AS(I)
AL(19) = SIMP (DS)
DO 38 I=1,NET
38 G(I)=G(I)*AS(I)
AL(20) = SIMP (DS)
DO 39 I=1,NET
39 G(I)=GA(I)*SINE(I)
AL(21) = SIMP (DS)
DO 40 I=1,NET
40 G(I)=G(I)*AS(I)
AL(22) = SIMP (DS)
DO 41 I=1,NET
41 G(I)=G(I)*AS(I)
AL(23) = SIMP (DS)
DO 42 I=1,NET
42 G(I)=G(I)*AS(I)
AL(24) = SIMP (DS)
DO 43 I=1,NET
43 GA(I)=1./AR(I)
ISIMP = 1
AL(44) = SIMP (DS)
DO 44 I=1,NET
44 G(I)=GA(I)*AS(I)
AL(45) = SIMP (DS)
DO 45 I=1,NET
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45 G(I)=G(I)*AS(I)
   AL(46) = SIMP (DS)
   DO 46 I=1,NET
46 G(I)=G(I)*AS(I)
   AL(47) = SIMP (DS)
   DO 47 I=1,NET
47 G(I)=G(I)*AS(I)
   AL(48) = SIMP (DS)
   DO 48 I=1,NET
48 G(I)=GA(I)*COSINE(I)
   AL(49) = SIMP (DS)
   DO 49 I=1,NET
49 G(I)=G(I)*AS(I)
   AL(50) = SIMP (DS)
   DO 50 I=1,NET
50 G(I)=G(I)*AS(I)
   AL(51) = SIMP (DS)
   DO 51 I=1,NET
51 G(I)=G(I)*AS(I)
   AL(52) = SIMP (DS)
   DO 52 I=1,NET
52 G(I)=G(I)*AS(I)
   AL(53) = SIMP (DS)
   DO 53 I=1,NET
53 G(I)=GA(I)*COSINE(I)*COSINE(I)
   AL(54) = SIMP (DS)
   DO 54 I=1,NET
54 G(I)=G(I)*AS(I)
   AL(55) = SIMP (DS)
   DO 55 I=1,NET
55 G(I)=G(I)*AS(I)
   AL(56) = SIMP (DS)
   DO 56 I=1,NET
56 G(I)=G(I)*AS(I)
   AL(57) = SIMP (DS)
   DO 57 I=1,NET
57 G(I)=G(I)*AS(I)
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AL(58) = SIMP (DS)
DO 58 I=1,NET
58 G(I)=G(I)*AS(I)
AL(59) = SIMP (DS)
DO 59 I=1,NET
59 G(I)=G(I)*AS(I)
AL(60) = SIMP (DS)
DO 60 I=1,NET
60 GA(I)=GA(I)*SINE(I)
ISIMP = 1
AL(61) = SIMP (DS)
DO 61 I=1,NET
61 G(I)=GA(I)*AS(I)
AL(62) = SIMP (DS)
DO 62 I=1,NET
62 G(I)=G(I)*AS(I)
AL(63) = SIMP (DS)
DO 63 I=1,NET
63 G(I)=GA(I)*COSINE(I)
AL(64) = SIMP (DS)
DO 64 I=1,NET
64 G(I)=G(I)*AS(I)
AL(65) = SIMP (DS)
DO 65 I=1,NET
65 G(I)=G(I)*AS(I)
AL(66) = SIMP (DS)
DO 66 I=1,NET
66 G(I)=G(I)*AS(I)
AL(67) = SIMP (DS)
DO 67 I=1,NET
67 G(I)=G(I)*AS(I)
AL(68) = SIMP (DS)
DO 68 I=1,NET
68 GA(I)=GA(I)*SINE(I)
ISIMP = 1
AL(69) = SIMP (DS)
DO 69 I=1,NET
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69 G(I)=GA(I)*AS(I)
   AL(70) = SIMP (DS)
   DO 70 I=1,NET
70 G(I)=G(I)*AS(I)
   AL(71) = SIMP (DS)
   DO 71 I=1,NET
71 G(I)=G(I)*AS(I)
   AL(72) = SIMP (DS)
   DO 72 I=1,NET
72 G(I)=G(I)*AS(I)
   AL(73) = SIMP (DS)
   DO 73 I=1,NET
73 GA(I)=GA(I)*PHP(I)
   ISIMP = 1
   AL(86) = SIMP (DS)
   DO 74 I=1,NET
74 G(I) = GA(I)*AS(I)
   AL(87) = SIMP (DS)
   DO 75 I=1,NET
75 G(I)=G(I)*AS(I)
   AL(88) = SIMP (DS)
   DO 76 I=1,NET
76 G(I)=G(I)*AS(I)
   AL(89) = SIMP (DS)
   DO 77 I=1,NET
77 GA(I)=PHP(I)*SINE(I)/AR(I)
   ISIMP = 1
   AL(80) = SIMP (DS)
   DO 78 I=1,NET
78 G(I) = GA(I) * AS(I)
   AL(81) = SIMP (DS)
   DO 156 I = 1,NET
156 G(I) = G(I) * AS(I)
   AL(82) = SIMP (DS)
   DO 79 I=1,NET
79 G(I)=G(I)*AS(I)
   AL(83) = SIMP (DS)
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DO 80 I=1,NET
80 G(I)=GA(I)*COSINE(I)
   AL(84) = SIMP (DS)
   DO 81 I=1,NET
61 G(I)=G(I)*AS(I)
   AL(85) = SIMP (DS)
   DO 82 I=1,NET
82 GA(I)=PHP(I)/AR(I)
   ISIMP = I
   AL(74) = SIMP (DS)
   DO 83 I=1,NET
83 G(I)=GA(I)*AS(I)
   AL(75) = SIMP (DS)
   DO 84 I=1,NET
84 G(I)=G(I)*AS(I)
   AL(76) = SIMP (DS)
   DO 85 I=1,NET
85 G(I)=G(I)*AS(I)
   AL(77) = SIMP (DS)
   DO 86 I=1,NET
86 G(I)=GA(I)*COSINE(I)
   AL(78) = SIMP (DS)
   DO 87 I=1,NET
87 G(I)=G(I)*AS(I)
   AL(79) = SIMP (DS)
   DO 88 I=1,NET
88 GA(I)=GA(I)*PHP(I)
   ISIMP = I
   AL(90) = SIMP (DS)
   DO 89 I=1,NET
89 G(I)=GA(I)*AS(I)
   AL(91) = SIMP (DS)
   DO 90 I=1,NET
90 G(I)=G(I)*AS(I)
   AL(92) = SIMP (DS)
   DO 91 I=1,NET
91 G(I)=GA(I)*SINE(I)
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AL(93) = SIMP (DS)
DO 92 I=1,NET
92 G(I)=G(I)*AS(I)
AL(94) = SIMP (DS)
DO 93 I=1,NET
93 G(I)=G(I)*AS(I)
AL(95) = SIMP (DS)
DO 94 I=1,NET
94 G(I)=G(I)*SINE(I)
AL(98) = SIMP (DS)
DO 95 I=1,NET
95 G(I)=AS(I)*PHP(I)**2*SINE(I)**2/AR(I)
AL(97) = SIMP (DS)
DO 96 I=1,NET
96 G(I)=PHP(I)**2*SINE(I)**2/AR(I)
AL(96) = SIMP (DS)
DO 97 I=1,NET
97 GA(I)=PHP(I)*SINE(I)/AR(I)**2
ISIMP = 1
AL(111) = SIMP (DS)
DO 98 I=1,NET
98 G(I)=GA(I)*AS(I)
AL(112) = SIMP (DS)
DO 99 I=1,NET
99 G(I)=G(I)*AS(I)
AL(113) = SIMP (DS)
DO 100 I=1,NET
100 G(I)=G(I)*AS(I)
AL(114) = SIMP (DS)
DO 101 I=1,NET
101 G(I)=G(I)*AS(I)
AL(115) = SIMP (DS)
DO 102 I=1,NET
102 G(I)=GA(I)*COSINE(I)
AL(116) = SIMP (DS)
DO 103 I=1,NET
103 G(I)=G(I)*AS(I)
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AL(117) = SIMP (DS)
DO 104 I=1,NET
104 G(I)=G(I)*AS(I)
AL(118) = SIMP (DS)
DO 105 I=1,NET
105 G(I)=GA(I)*SINE(I)
AL(119) = SIMP (DS)
DO 106 I=1,NET
106 G(I)=G(I)*AS(I)
AL(120) = SIMP (DS)
DO 107 I=1,NET
107 G(I)=G(I)*AS(I)
AL(121) = SIMP (DS)
DO 108 I=1,NET
108 G(I)=G(I)*AS(I)
AL(122) = SIMP (DS)
DO 109 I=1,NET
109 G(I)=G(I)*AS(I)
AL(123) = SIMP (DS)
DO 110 I=1,NET
110 G(I)=GA(I)*SINE(I)*COSINE(I)
AL(124) = SIMP (DS)
DO 111 I=1,NET
111 G(I)=G(I)*AS(I)
AL(125) = SIMP (DS)
DO 112 I=1,NET
112 G(I)=G(I)*AS(I)
AL(126) = SIMP (DS)
DO 113 I=1,NET
113 GA(I)=SINE(I)/AR(I)**2
ISIMP = I
AL(99) = SIMP (DS)
DO 114 I=1,NET
114 G(I)=GA(I)*AS(I)
AL(100) = SIMP (DS)
DO 115 I=1,NET
115 G(I)=G(I)*AS(I)
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AL(101) = SIMP (DS)
DO 116 I=1,NET
116 G(I)=G(I)*AS(I)
AL(102) = SIMP (DS)
DO 117 I=1,NET
117 G(I)=G(I)*AS(I)
AL(103) = SIMP (DS)
DO 118 I=1,NET
118 G(I)=G(I)*AS(I)
AL(104) = SIMP (DS)
DO 119 I=1,NET
119 G(I)=GA(I)*COSINE(I)
AL(105) = SIMP (DS)
DO 120 I=1,NET
120 G(I)=G(I)*AS(I)
AL(106) = SIMP (DS)
DO 121 I=1,NET
121 G(I)=G(I)*AS(I)
AL(107) = SIMP (DS)
DO 122 I=1,NET
122 G(I)=G(I)*AS(I)
AL(108) = SIMP (DS)
DO 123 I=1,NET
123 G(I)=GA(I)*COSINE(I)*COSINE(I)
AL(109) = SIMP (DS)
DO 124 I=1,NET
124 G(I)=G(I)*AS(I)
AL(110) = SIMP (DS)
DO 125 I=1,NET
125 GA(I)=SINE(I)**2/AR(I)**3
ISIMP = 1
AL(142) = SIMP (DS)
DO 126 I=1,NET
126 G(I)=GA(I)*AS(I)
AL(143) = SIMP (DS)
DO 127 I=1,NET
127 G(I)=G(I)*AS(I)
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AL(144) = SIMP (DS)
DO 128 I=1,NET
128 G(I)=G(I)*AS(I)
AL(145) = SIMP (DS)
DO 129 I=1,NET
129 G(I)=G(I)*AS(I)
AL(146) = SIMP (DS)
DO 130 I=1,NET
130 G(I)=G(I)*AS(I)
AL(147) = SIMP (DS)
DO 131 I=1,NET
131 G(I)=G(I)*AS(I)
AL(148) = SIMP (DS)
DO 132 I=1,NET
132 GA(I)=GA(I)*COSINE(I)
ISIMP = 1
AL(149) = SIMP (DS)
DO 133 I=1,NET
133 G(I)=GA(I)*AS(I)
AL(150) = SIMP (DS)
DO 134 I=1,NET
134 G(I)=G(I)*AS(I)
AL(151) = SIMP (DS)
DO 135 I=1,NET
135 G(I)=G(I)*AS(I)
AL(152) = SIMP (DS)
DO 136 I=1,NET
136 G(I)=G(I)*AS(I)
AL(153) = SIMP (DS)
DO 137 I=1,NET
137 G(I)=GA(I)*COSINE(I)
AL(154) = SIMP (DS)
DO 138 I=1,NET
138 G(I)=G(I)*AS(I)
AL(155) = SIMP (DS)
DO 139 I=1,NET
139 G(I)=G(I)*AS(I)
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AL(156) = SIMP (DS)
DO 140 I=1,NET
140 GA(I)=COSINE(I)/AR(I)**3
ISIMP = 1
AL(134) = SIMP (DS)
DO 141 I=1,NET
141 G(I)=GA(I)*AS(I)
AL(135) = SIMP (DS)
DO 142 I=1,NET
142 G(I)=G(I)*AS(I)
AL(136) = SIMP (DS)
DO 143 I=1,NET
143 G(I)=G(I)*AS(I)
AL(137) = SIMP (DS)
DO 144 I=1,NET
144 G(I)=G(I)*AS(I)
AL(138) = SIMP (DS)
DO 145 I=1,NET
145 G(I)=GA(I)*COSINE(I)
AL(139) = SIMP (DS)
DO 146 I=1,NET
146 G(I)=G(I)*AS(I)
AL(140) = SIMP (DS)
DO 147 I=1,NET
147 G(I)=G(I)*AS(I)
AL(141) = SIMP (DS)
DO 148 I=1,NET
148 G(I)=1./AR(I)**3
AL(127) = SIMP (DS)
DO 149 I=1,NET
149 G(I)=G(I)*AS(I)
AL(128) = SIMP (DS)
DO 150 I=1,NET
150 G(I)=G(I)*AS(I)
AL(129) = SIMP (DS)
DO 151 I=1,NET
151 G(I)=G(I)*AS(I)
```

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AL(130) = SIMP (DS)
DO 152 I=1,NET
152 G(I)=G(I)*AS(I)
AL(131) = SIMP (DS)
DO 153 I=1,NET
153 G(I)=G(I)*AS(I)
AL(132) = SIMP (DS)
DO 154 I=1,NET
154 G(I)=G(I)*AS(I)
AL(133) = SIMP (DS)
DO 157 I=1,NET
157 G(I) = AR(I)*AS(I)*AS(I)*AS(I)
AL(157) = SIMP (DS)
AIL(IEL)=AL(1)
A2L( IEL)=AL(2)
A3L( IEL)=AL(3)
AL157( IEL)=AL(157)
IF( ISAVE.EQ.0)WRITE(15CRA3)AL(1),AL(2),AL(3),AL(157)
RETURN
END
FUNCTION SIMP(DS)
COMMON /A1/ G(30),GA(30),P(75),R(75),S(75),O(75)
COMMON /A3/ NDP,NET,IXX(17)
COMMON /B5/ISIMP
NET=NET
NET3 = NET
NET1 = NET3 - 1
NET2 = NET3 - 2
SUM=0.0
IF(ISIMP-1)3,4,3
3 DO 1 I=2,NET1,2
1 SUM = SUM + 4.0*G(I)
DO 2 I=3,NET2,2
2 SUM = SUM + 2.0*G(I)
AG1 = G(1)
AG2 = G(NET3)
GO TO 10
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4 DO 11 I=2,NET1,2
11 SUM = SUM + 4.0*GA(I)
22 DO 22 I=3,NET2,2
22 SUM = SUM + 2.0*GA(I)
AG1 = GA(1)
AG2 = GA(NET3)
10 SIMP = (AG1 + SUM + AG2)*DS/3.0
ISIMP = 0
RETURN
END
SUBROUTINE INTE(I,J,L,KEY,SIMS)
PI=3.14159265
SIMS=0.
IF((I+J).NE.(L+1)).AND.(IABS(J-I).NE.(L-1))RETURN
IF(KEY.EQ.0)GO TO 10
IF(KEY.EQ.2)GO TO 20
RETURN
10 K=1
IF(J.EQ.1)K=2
IF(I.EQ.1)K=K+1
IF(L.EQ.1)K=K+1
FK=K
SIMS=FK*PI/2.0
RETURN
20 IF(J.EQ.1.OR.I.EQ.1)RETURN
IF(L.NE.1)GO TO 30
SIMS=PI
RETURN
30 SIMS=PI/2.0
IF(J+I-L.EQ.1)SIMS=-PI/2.0
RETURN
END
SUBROUTINE CREATL (AM,AN,C1,C2,C,D1,D2,D,G1,G2)
COMMON /B1/ AL(157)
COMMON /B3/ PHPP
COMMON /B4/ F(8,8)
DIMENSION E(8,8)
0470
0480
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IFLAG = 0
C   THE ABOVE IS A FLAG USED TO DETERMINE WHETHER THE ELEMENTS
C   ABOVE OR BELOW THE DIAGONAL ARE BEING CALCULATED
C   CALCULATE THE DIAGONAL ELEMENTS OF F
      F(1,1) =
A   C1*AL(8) + C2*AL(54) - C*AL(30) + 4.*G2*AM*AN*AL(142) + D2*AM*
1   AM*AN*AN*AL(127)
      F(2,2) =
A   C1*AL(10) + C2*AL(56) - C*AL(32) + D2*AM*AM*AN*AN*AL(129) - D2
1   *(AM*AM+AN*AN)*AL(100) + D2*AL(69) + 4.*G2*AM*AN*AL(144) - 8.*
2   G2*AM*AN*AL(100) + 4.*G2*AM*AN*AL(44)
      F(3,3) =
A   C1*AL(12) + C2*AL(58) - C*AL(34) + 4.*D1*AL(1) + D2*AM*AM*AN*
1   AN*AL(131) - 2.*D2*(AM*AM+AN*AN)*AL(102) + 4.*D2*AL(71) - D*(
2   AM*AM+AN*AN)*AL(46) + 4.*D*AL(22) + 4.*G2*AM*AN*AL(146) - 16.*
3   G2*AM*AN*AL(102) + 16.*G2*AM*AN*AL(46)
      F(4,4) =
A   C1*AL(14) + C2*AL(60) - C*AL(36) + 36.*D1*AL(3) + D2*AM*AM*AN*
1   AN*AL(133) - 3.*D2*(AM*AM+AN*AN)*AL(104) - 3.*D*(AM*AM+AN*AN)*
2   AL(48) + 18.*D*AL(24) + 9.*D2*AL(73) + 4.*G2*AM*AN*AL(148) -
3   24.*G2*AM*AN*AL(104) + 36.*G2*AM*AN*AL(48)
      F(5,5) =
A   C2*AL(69) + G1*AM*AN*AL(44) + D1*PHPP**2 *AL(1) + D2*AL(96) +
1   D*PHPP*AL(37) + G2*AM*AN*AL(90)
      F(6,6) =
A   C1*AL(1) + C2*AL(71) + C*AL(22) + G1*AM*AN*AL(46) + D1*PHPP**2
1   * AL(3) + 2.*D1*PHPP*AL(5) + D1*AL(8) + D2*AL(98) + D*PHPP*
2   AL(39) + D*AL(43) + G2*AM*AN*AL(92)
      F(7,7) =
A   C2*AM*AN*AL(44) + G1*AL(69) + D2*AM*AN*AL(139) + 4.*G2*AL(154)
1   + 4.*G2*AL(124) + G2*AL(96)
      F(8,8) =
A   C2*AM*AN*AL(46) + G1*AL(71) - 2.*G1*AL(22) + D2*AM*AN*AL(141)
1   + G1*AL(1) + 4.*G2*AL(156) - 4.*G2*AL(110) + 4.*G2*AL(126) +
2   G2*AL(54) - 2.*G2*AL(85) + G2*AL(98)
C   CALCULATE ELEMENTS ABOVE THE DIAGONAL AND CALL THEM E
      IC E(1,2) =

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A C1*AL(9) + C2*AL(55) - C*AL(31) + D2*AM*AM*AN*AL(128) + 4.*
1 G2*AM*AN*AL(143) - D2*AM*AM*AL(99) - 4.*G2*AM*AN*AL(99)
E(1,3) =
A C1*AL(10) + C2*AL(56) - C*AL(32) + D2*AM*AM*AN*AL(129)
1 + 4.*G2*AM*AN*AL(144) - 2.*D2*AM*AM*AL(100) - D*AM*AM*AL(44) -
2 8.*G2*AM*AN*AL(100)
E(1,4) =
A C1*AL(11) + C2*AL(57) - C*AL(33) + D2*AM*AM*AN*AL(130) - 3.*
1 *D2*AM*AM*AL(101) - 3.*D*AM*AM*AL(45) + 4.*G2*AM*AN*AL(145) -
2 12.*G2*AM*AN*AL(101)
E(1,5) =
A C2*AL(64) - C/2.*AL(37) - D2*AM*AM*AL(111) - D/2.*AM*AM*PHPP*
1 AL(44) - 2.*G2*AM*AN*AL(111)
E(1,6) =
A - C1*AL(4) + C2*AL(65) - C/2.*AL(38) - D2*AM*AM*AL(112) - D/2.*
1 *AM*AM*PHPP*AL(45) - D/2.*AM*AM*AL(74) + C/2.*AL(17) - 2.*G2*
2 AM*AN*AL(112)
E(1,7) =
A C2*AM*AL(49) - C/2.*AN*AL(25) + D2*AM*AM*AN*AL(134) + 4.*G2*AM
1 *AL(149) + 2.*G2*AM*AL(119)
E(1,8) =
A C2*AM*AL(50) - C/2.*AN*AL(26) + D2*AM*AM*AN*AL(135) + 4.*G2*AM
1 *AL(150) - 2.*G2*AM*AL(105) + 2.*G2*AM*AL(120)
E(2,3) =
A C1*AL(11) + C2*AL(57) - C*AL(33) + D2*AM*AM*AN*AL(130) - D2
1 *(2.*AM*AM*AN*AN)*AL(101) + 2.*D2*AL(70) - D*AM*AM*AL(45) + D*
2 AL(21) + 4.*G2*AM*AN*AL(145) - 12.*G2*AM*AN*AL(101) + 8.*G2*AM
3 *AN*AL(45)
E(2,4) =
A C1*AL(12) + C2*AL(58) - C*AL(34) + D2*AM*AM*AN*AL(131) - D2
1 *(3.*AM*AM*AN*AN)*AL(102) + 3.*D2*AL(71) - 3.*D*AM*AM*AL(46) +
2 3.*D*AL(22) + 4.*G2*AM*AN*AL(146) + 12.*G2*AM*AN*AL(46) - 16.*
3 G2*AM*AN*AL(102)
E(2,5) =
A C2*AL(65) - C/2.*AL(38) - D2*AM*AM*AL(112) + D2*AL(86) - D/2.*
1 AM*AM*PHPP*AL(45) + 2/2.*PHPP*AL(21) - 2.*G2*AM*AN*AL(112) +
2 2.*G2*AM*AN*AL(74)

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```

E(2,6) =
A  - C1*AL(5) + C2*AL(66) - C/2.*AL(39) + C/2.*AL(18) - D2*AM*AM*
1  AL(113) + D2*AL(87) - D/2.*AM*AM*PHPP*AL(46) + D/2.*PHPP*
2  AL(22) - D/2.*AM*AM*AL(75) + D/2.*AL(37) - 2.*G2*AM*AN*AL(113)
3  + 2.*G2*AM*AN*AL(75)
E(2,7) =
A  C2*AN*AL(50) - C/2.*AN*AL(26) + D2*AM*AM*AN*AL(135) - D2*AN*
1  AL(105) + 4.*G2*AM*AL(150) - 4.*G2*AM*AL(105) - 2.*G2*AM*
2  AL(80) + 2.*G2*AM*AL(120)
E(2,8) =
A  C2*AN*AL(51) - C/2.*AN*AL(27) + D2*AM*AM*AN*AL(136) - D2*AN*
1  AL(106) + 4.*G2*AM*AL(151) + 2.*G2*AM*AL(121) - 6.*G2*AM*
2  AL(106) + 2.*G2*AM*AL(49) - 2.*G2*AM*AL(81)
E(3,4) =
A  C1*AL(13) + C2*AL(59) - C*AL(35) + 12.*D1*AL(2) + D2*AM*AM*AN*
1  AN*AL(132) - D2*(3.*AM*AM*2.*AN*AN)*AL(103) + 6.*D2*AL(72) - D
2  *(AN*AN+3.*AM*AM)*AL(47) + 9.*D*AL(23) + 4.*G2*AM*AN*AL(147) -
3  20.*G2*AM*AN*AL(103) + 24.*G2*AM*AN*AL(47)
E(3,5) =
A  C2*AL(66) - C/2.*AL(39) + 2.*D1*PHPP*AL(1) - D2*AM*AM*AL(113)
1  + 2.*D2*AL(87) + D*AL(37) - D/2.*AM*AM*PHPP*AL(46) + D*PHPP*
2  AL(22) - 2.*G2*AM*AN*AL(113) + 4.*G2*AM*AN*AL(75)
E(3,6) =
A  - C1*AL(6) + C2*AL(67) - C/2.*AL(40) + 2.*D1*PHPP*AL(2) + 2.*
1  D1*AL(4) - D2*AM*AM*AL(114) + 2.*D2*AL(88) + 2.*D*AL(38) - D/
2  2.*AM*AM*PHPP*AL(47) + D*PHPP*AL(23) - D/2.*AM*AM*AL(76) + C/
3  2.*AL(19) - 2.*G2*AM*AN*AL(114) + 4.*G2*AM*AN*AL(76)
E(3,7) =
A  C2*AN*AL(51) - C/2.*AN*AL(27) + D2*AM*AM*AN*AL(136) - 2.*D2*AN
1  *AL(106) - D*AN*AL(49) - 4.*G2*AM*AL(81) + 4.*G2*AM*AL(151) +
2  2.*G2*AM*AL(121) - 8.*G2*AM*AL(106)
E(3,8) =
A  C2*AN*AL(52) - C/2.*AN*AL(28) + D2*AM*AM*AN*AL(137) - 2.*D2*AN
1  *AL(107) - D*AN*AL(50) + 4.*G2*AM*AL(152) - 10.*G2*AM*AL(107)
2  + 2.*G2*AM*AL(122) + 4.*G2*AM*AL(50) - 4.*G2*AM*AL(82)
E(4,5) =
A  C2*AL(67) - C/2.*AL(40) + 6.*D1*PHPP*AL(2) - D2*AM*AM*AL(114)

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1 + 3.*D2*AL(88) + 3.*D*AL(38) - D/2.*AM*AM*PHPP*AL(47) + 3./2.*
2 D*PHPP*AL(23) - 2.*G2*AM*AN*AL(114) + 6.*G2*AM*AN*AL(76)
E(4,6) =
A - C1*AL(7) + C2*AL(68) - C/2.*AL(41) + 6.*D1*PHPP*AL(3) + 6.*
1 D1*AL(5) - D2*AM*AM*AL(115) + 3.*D2*AL(89) - D/2.*AM*AM*PHPP*
2 AL(48) + 3./2.*D*PHPP*AL(24) - D/2.*AM*AM*AL(77) + 9./2.*D*
3 AL(39) + C/2.*AL(20) - 2.*G2*AM*AN*AL(115) + 6.*G2*AM*AN*
4 AL(77)
E(4,7) =
A C2*AM*AL(52) - C/2.*AN*AL(28) + D2*AM*AM*AN*AL(137) - 3.*D2*AN
1 *AL(107) - 3.*D*AN*AL(50) + 4.*G2*AM*AL(152) + 2.*G2*AM*
2 AL(122) - 12.*G2*AM*AL(107) - 6.*G2*AM*AL(82)
E(4,8) =
A C2*AM*AL(53) - C/2.*AN*AL(29) + D2*AM*AM*AN*AL(138) - 3.*D2*AN
1 *AL(108) - 3.*D*AN*AL(51) + 4.*G2*AM*AL(153) - 14.*G2*AM*
2 AL(108) + 2.*G2*AM*AL(123) + 6.*G2*AM*AL(51) - 6.*G2*AM*AL(83)
E(5,6) =
A C2*AL(70) + C/2.*AL(21) + G1*AM*AN*AL(45) + D1*PHPP**2 *AL(2)
1 + D1*PHPP*AL(4) + D2*AL(97) + D*PHPP*AL(38) + D/2.*AL(42) + G2
2 *AM*AN*AL(91)
E(5,7) =
A C2*AM*AL(61) + G1*AM*AL(61) - D2*AN*AL(116) - D/2.*AM*PHPP*
1 AL(49) - 2.*G2*AM*AL(116) - G2*AM*AL(93)
E(5,8) =
A C2*AM*AL(62) + G1*AM*AL(62) - G1*AM*AL(15) - D2*AN*AL(117) - D
1 /2.*AM*PHPP*AL(50) - 2.*G2*AM*AL(117) + G2*AM*AL(78) - G2*AM*
2 AL(94)
E(6,7) =
A C2*AM*AL(62) + C/2.*AN*AL(15) + G1*AM*AL(62) -D2*AN*AL(117) -
1 D/2.*AM*PHPP*AL(50) -D/2.*AN*AL(78) -2.*G2*AM*AL(117) - G2*AM*AL(94)
E(6,8) =
A C2*AM*AL(63) + C/2.*AN*AL(16) + G1*AM*AL(63) - G1*AM*AL(16) -
1 D2*AN*AL(118) - D/2.*AM*PHPP*AL(51) - D/2.*AN*AL(79) - 2.*G2*
2 AM*AL(118) + G2*AM*AL(79) - G2*AM*AL(95)
E(7,8) =
A C2*AM*AN*AL(45) + G1*AL(70) - G1*AL(21) + D2*AM*AN*AL(140) +
1 4.*G2*AL(155) - 2.*G2*AL(109) + 4.*G2*AL(125) - G2*AL(84) + G2

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```
2 *AL(97)
  IFLAG = IFLAG + 1
  IF (IFLAG.NE.1) GO TO 20
  THE ELEMENTS OF F THAT ARE NEEDED ARE THOSE ABOVE THE DIAGONAL
  C      THUS F IS IDENTICAL TO E
  DO 15 I=1,7
  II=I+1
  DO 15 J=II,8
  F(I,J) = E(I,J)
  C      15 CONTINUE
  C      INTERCHANGE AM AND AN TO RECALCULATE E FOR THE TERMS OF F
  C      BELOW THE DIAGONAL
  TEMP = AM
  AM = AN
  AN = TEMP
  GO TO 10
  C      THE F ELEMENTS BELOW THE DIAGONAL ARE SET EQUAL TO E ELEMENTS
  C      ABOVE THE DIAGONAL
  DO 20 I=1,8
  II = I-1
  DO 25 J=1,II
  F(I,J) = E(J,I)
  C      25 CONTINUE
  RETURN
  END
  SUBROUTINE MMPLT3
  COMMON /B4/ F(8,8)
  COMMON /B6/ AK(8,8)
  COMMON /B8/ CHECK(8,8)
  DO 1 I=1,8
  DO 1 J=1,8
  AK(I,J)=0.0
  DO 1 K=1,8
  1 AK(I,J)=AK(I,J)+F(I,K)*CHECK(K,J)
  DO 2 I=1,8
  DO 2 J=1,8
  F(I,J)=0.0
```

```
DO 2 K=1,8
2 F(I,J)=F(I,J)+CHECK(K,I)*AK(K,J)
DO 3 I=1,8
DO 3 J=1,8
3 AK(I,J)=F(I,J)
RETURN
END
SUBROUTINE FORCES( IEL )
COMMON /A1/ G(30),GA(30), P(75),R(75),S(75),O(75)
COMMON /A2/ ONE(68),TWO(68)
COMMON /A3/ NDP,NET,IXX(17)
COMMON /A7/ NTAPE,ISCRA1,ISCRA3,NELEMS,NH,IPAGE,PROBLE
COMMON /B5/ ISIMP
COMMON /B8/ CHECK(8,8)
COMMON /B9/ IA,IB
COMMON /B15/ IELL,NDPP, IXXX(17),LINEL,ICON,ICOO,NSTOP
COMMON /B19/ IELRES(10),REST(50,8),NEREST
COMMON /B24/ RO1(50),PHY1(50),PHY2(50),Z1(50),Z2(50)
COMMON /C40/ IPRI
COMMON /D10/ ISAVE, IDISPL
COMMON /D40/ PHPRMI(50),RMDI(50),CPDM(50),SPDM(50),PPPH(50),
IARLC(50),CHEKC(50,8,8),R02(50),AIL(50),A2L(50),A3L(50),AL157(50)
COMMON /G1/ APPO,THELO,RC,RL,PR
COMMON /G4/ ONED(51,68)
DIMENSION Q(8),QQ(8)
DOUBLE PRECISION ONED
INTEGER CONST
INTEGER CONS
INTEGER APOL
INTEGER APPO
INTEGER REST
DATA APOL/4HAPOL/
DATA CONST/4HCONS/
IF( APPG, NE, APOL ) GO TO 770
I=IEL
DO 780 J=1, NH
780 IXX(J)=0
```

```
THELO1=THELO/57.2957795
RA=RC*COS(THELO1)
RB=RC*SIN(THELO1)
DO 703 J=1,3
P(J)=0.
R(J)=0.
S(J)=0.
D(J)=0.
703 CONTINUE
RFF=R02(I)
IF(RFF.GT.(RC+RL))GO TO 403
IF(RFF.LE.(RC-RL))GO TO 403
IF(I.NE.1)GO TO 760
RF=R02(I)/2.
GO TO 761
760 RF=RFF-(R02(I)-R02(I-1))/2.
761 RR=RA*RA+RF*RF+RB*RB-RL*RL
RRR=4.*RB*RB*RF*RF-RR*RR
RRA=4.*RA*RA+4.*RB*RB
RRB=4.*RR*RA
X3=SQRT(ABS(RRB*RRB+4.*RRA*RRR))
X1=(RRB+X3)/(2.*RRA)
X2=(RRB-X3)/(2.*RRA)
IF(THELO.NE.0.)GO TO 712
Y1=SQRT(ABS(RF*RF-X1*X1))
Y2=-Y1
GO TO 716
712 Y1=(RR-2.*X1*RA)/(2.*RB)
Y2=(RR-2.*X2*RA)/(2.*RB)
716 THE1=ATAN2(Y1,X1)*57.2957795
THE2=ATAN2(Y2,X2)*57.2957795
IF(Y1.GE.0..AND.Y2.GE.0.)GO TO 740
IF(Y1.LE.0..AND.Y2.LE.0.)GO TO 750
R(1)=PR
R(3)=PR
IF(Y1)721,721,723
723 O(2)=THE1
```



```

O(3)=360.+THE2
GO TO 724
721 O(2)=THE2
O(3)=360.+THE1
IF(X2.LT.0.)O(2)=180.+THE2
GO TO 724
740 R(2)=PR
O(2)=THE1
O(3)=THE2
IF(X2.LT.0.)O(3)=180.+THE2
GO TO 724
750 R(2)=PR
O(2)=360.+THE1
O(3)=360.+THE2
724 NDP=3
GO TO 500
770 ICO=0
IF(IEL.EQ.1)GO TO 402
IF(NSTOP.EQ.CONST)GO TO 500
IF(ICON.EQ.0)GO TO 406
402 READ(5,100)IELL,NDPP,NSTOP,(IXXX(I),I=1,NH)
100 FORMAT(2I5,A4,1X,17I1)
406 IF(IEL.NE.IELL)GO TO 403
400 ICON=1
NDP = NDPP
DO 401 I=1,NH
IXX(I) = IXXX(I)
401 CONTINUE
GO TO 407
403 ICON=0
NDP = 0
DO 404 I=1,NH
IXX(I)=0
404 CONTINUE
407 CONTINUE
IF(NDP.NE.0)GO TO 77
P(I)=0.
```

```
R(1)=0.
S(1)=0.
O(1)=0.
NDP = 1
GO TO 78
77 CONTINUE
C *****
C FIRST DATA POINT MUST BE AT 0.0 DEGREES
C *****
C READ(5,101)(P(I),R(I),S(I),O(I),I=1,NDP)
500 DO 3006 IL=1,NDP
IPRI=IPRI+1
O(NDP+1)=360.
LLITT=0
IF(IPRI.EQ.1.AND.IL.EQ.1)LINEL=50
IF(LINEL.LT.50)GO TO 3007
WRITE(6,3000)IPAGE
IPAGE = IPAGE +1
LINEL=0
3000 FORMAT(1H1,62X,4HPAGEI3//)
LLITT=1
WRITE(6,3001)
3001 FORMAT(51X,30HAPPLIED LOADS ON THE STRUCTURE//)
LINEL=LINEL + 6
GO TO 3010
3007 IF(LLITT.EQ.1)GO TO 3020
IF(IL.NE.1)GO TO 3008
3010 WRITE(6,3002)IEL
3002 FORMAT(60X,12HELEMENT NO. I3/)
LINEL=LINEL+2
IF(LLITT.EQ.1)GO TO 3020
IF(ICOO.EQ.0)GO TO 3008
3020 WRITE(6,3003)
3003 FORMAT(56X,20HPRESSURE COMPONENTS/)
WRITE(6,3004)
```

```

3004 FORMAT(20X,10HMERIDIONAL,20X,6HNORMAL,20X,10HTANGENTIAL,11X,19HFRO
1M THETA TO THETA,9H(DEGREES)/)
LINEL=LINEL+3
3008 WRITE(6,3005)P(IL),R(IL),S(IL),O(IL),/(IL+1)
3005 FORMAT(2X,3(18X,F10.3),12X,2(2X,F8.3)/)
LINEL=LINEL+2
3006 CONTINUE
ICOO=0
ICO=1
DO 50 I=1,NDP
50 O(I)=O(I)/57.29578
101 FORMAT(4F10.0)
P(NDP+1)=P(I)
S(NDP+1)=S(I)
R(NDP+1)=R(I)
O(NDP+1) = 2.*3.1415926
78 IZ = IA
INDEX = 0
CALL FORCE1(IZ,INDEX,1,ICO,IEL)
INDEX= 4*IA
IZ=IB
CALL FORCE1(IZ,INDEX,0,ICO,IEL)
IAB= (IA+IB)*4
DO 310 IR=1,NEREST
IF(IELRES(IR).NE.IEL)GO TO 310
DO 301 JR=1,4
IF(REST(IEL,JR).EQ.0)GO TO 301
DO 302 JJR=1,NH
KR=JR+(JJR-1)*4
ONE(KR)=0.
302 CONTINUE
301 CONTINUE
310 CONTINUE
DO 480 KK=1,IAB
480 ONED(IEL,KK)=ONE(KK)
DO 20 JACK = 1,IAB
20 ONE(JACK) = TWO(JACK)

```

```

RETURN
END
SUBROUTINE FORCE1(IZ,INDE,ICHANG,ICO,IEL)
COMMON /A1/ G(30),GA(30),P(75),R(75),S(75),O(75)
COMMON /A2/ ONE(68),TWO(68)
COMMON /A3/ NDP,NET,IXX(17)
COMMON /A7/ NTAPE,ISCRA1,ISCRA3,NELEMS,NH,IPAGE,PROBLE
COMMON /B5/ISIMP
COMMON /B8/ CHECK(8,8)
COMMON /B9/ IA,IB
COMMON /B15/ IELL,NDPP,IXX(17),LINEL,ICON,ICOO,NSTOP
COMMON /D40/ PHPRMI(50),RMDI(50),CPDM(50),SPDM(50),PPPH(50),
1ARLC(50),CHEKC(50,8,8),R02(50),AIL(50),A2L(50),A3L(50),AL157(50)
DIMENSION QQ(8),Q(8),OP(101),FORCE(8)
DATA NAMEA/IHA/,NAMEB/IHB/
IF(IZ.EQ.0) RETURN
JACK = (INDE /4)+1
DO 1 IAAA=1,IZ
99 IAA=IAAA-1
DO 2 KKK=1,8
FORCE(KKK)=0.
2 Q(KKK)=0.
HIA=IAA
DO 3 I=1,NDP
IF(NDP.EQ.1)GO TO 105
X=O(I)*HIA
XX=O(I+1)*HIA
IF(ICHANG.EQ.0)GO TO 12
IF(IAA.NE.0)GO TO 800
PINT=P(I)*(O(I+1))-O(I)}
SINT=0.
RINT=R(I)*(O(I+1))-O(I)}
GO TO 101
800 PINT=P(I)*(SIN(XX)-SIN(X))/HIA
SINT=-S(I)*(COS(XX)-COS(X))/HIA
RINT=R(I)*(SIN(XX)-SIN(X))/HIA
GO TO 101

```

```
12 IF (IAA.NE.0) GO TO 801
   PINT=0.
   SINT=S(I)*(O(I+1)-O(I))
   RINT=0.
   GO TO 101
801 PINT=-P(I)*(COS(XX)-COS(X))/HIA
   SINT=S(I)*(SIN(XX)-SIN(X))/HIA
   RINT=-R(I)*(COS(XX)-COS(X))/HIA
   GO TO 101
105 IF (IAA.NE.0) GO TO 102
   IF (ICHANG.EQ.0) GO TO 103
   PINT = 2.*3.141592 * P(I)
   SINT = 0.
   RINT = 2.*3.141592 * R(I)
   GO TO 101
103 RINT = 0.
   SINT = 2.*3.141592 * S(I)
   PINT = 0.
   GO TO 101
102 PINT = 0.
   RINT = 0.
   SINT = 0.
101 Q(1)=Q(I)+RINT*A1L(IEL)
   Q(2)=Q(2)+RINT*A2L(IEL)
   Q(3)=Q(3)+RINT*A3L(IEL)
   Q(4)=Q(4)+RINT*A157(IEL)
   Q(5)=Q(5)+PINT*A1L(IEL)
   Q(6)=Q(6)+PINT*A2L(IEL)
   Q(7)=Q(7)+SINT*A1L(IEL)
   Q(8)=Q(8)+SINT*A2L(IEL)
   DO 5 IFLAG = 1,8
   QQ(IFLAG) = 0.
   DO 5 IFL = 1,8
   QQ(IFLAG) = QQ(IFLAG) + CHECK(IFL,IFLAG)*Q(IFL)
   IF (IXX(JACK).EQ.0) GO TO 600
   READ(5,500)(FORCE(I), I=1,8)
500 FORMAT(8F10.0)
```

```
LLIT=0
NAMEH=NAMEB
IF(IICANG.EQ.1)NAMEH=NAMEA
IF(LINEL.EQ.0)GO TO 4010
IF(LINEL.LT.50)GO TO 4007
4010 WRITE(6,4000)IPAGE
4000 FORMAT(1H1,62X,4HPAGEI3//)
IPAGE = IPAGE +1
LINEL=0
WRITE(6,4020)
4020 FORMAT(51X,30HAPPLIED LOADS ON THE STRUCTURE//)
LLIT=1
LINEL=LINEL+8
GO TO 4015
4007 IF(IC0.EQ.0.AND.IAAA.EQ.1)GO TO 4015
IF(IAAA.NE.1)GO TO 4016
WRITE(6,4001)
WRITE(6,4003)
GO TO 4016
4015 WRITE(6,4002)IEL
4002 FORMAT(60X,12HELEMENT NO. I3/)
IF(LLIT.EQ.1)GO TO 4030
IF(IC0.EQ.0)GO TO 4016
4030 WRITE(6,4003)
4003 FORMAT(20X,5HAXIAL,20X,10HTANGENTIAL,19X,6HRADIAL,20X,7HANGULAR,5X
1,12HHARMONIC NO./)
WRITE(6,4001)
4001 FORMAT(1H0,44X,42HCONCENTRATED LINE LOAD COMPONENTS AT NODES/)
LINEL = LINEL + 3
4016 WRITE(6,4004)(FORCE(I),I=1,4),IAA,NAMEH
WRITE(6,4005)(FORCE(I),I=5,8),IAA,NAMEH
4004 FORMAT(1X,10HFIRST NODE,10X,F10.3,18X,F10.3,17X,F10.3,16X,F10.3,8X
1,I2,A2)
4005 FORMAT(1X,11HSECOND NODE,9X,F10.3,18X,F10.3,17X,F10.3,16X,F10.3,8X
1,I2,A2)
LINEL = LINEL + 4
IC00=1
```

```

600 INDEX=4*IAA + INDE
90 ONE(INDEX+1) = QQ(1) + ONE(INDEX+1) + FORCE(1)
ONE(INDEX+2) = QQ(2) + ONE(INDEX+2) + FORCE(2)
ONE(INDEX+3) = QQ(3) + ONE(INDEX+3) + FORCE(3)
ONE(INDEX+4) = QQ(4) + ONE(INDEX+4) + FORCE(4)
TWO(INDEX+1) = QQ(5) + FORCE(5)
TWO(INDEX+2) = QQ(6) + FORCE(6)
TWO(INDEX+3) = QQ(7) + FORCE(7)
TWO(INDEX+4) = QQ(8) + FORCE(8)
JACK = JACK +1
1 CONTINUE
RETURN
END
SUBROUTINE DISPA
COMMON /A7/ ITL, ISOL2, ISCRA3, NELEMS, NH, IPAGE, PROBLE
COMMON /B9/ IA, IB
COMMON /D10/ ISAVE, IDISPL
COMMON /G4/ ONED(51,68)
DOUBLE PRECISION T(2346), RHS1(68), S(4624), COE(68), RHS2(68)
DOUBLE PRECISION ONED
REWIND ITL
REWIND ISOL2
IDISPL=1
IORD=4*NH
ISIZS = IORD*IORD
ISIZT = (ISIZS+IORD)/2
NELP1=NELEMS+1
READ(ITL)(T(I), I=1, ISIZT)
DO 600 I=1, IORD
600 RHS1(I)=ONED(I, I)
DO 1 I=2, IORD
L = I+1
K = I-1
DO 1 J=1, K
JI = J+(1*I-1)/2
JJ = (J+J+J)/2
II = (I*I+I)/2

```

```
T(JI) = T(JI)/T(JJ)
T(II) = T(II)-T(JI)*T(JI)+T(JJ)
IF(I-IORD) 101,1,101
101 DO 3 M=L, IORD
   IM = I+(M*M-M)/2
   JM = J+(M*M-M)/2
3 T(IM) = T(IM)-T(JI)*T(JM)
1 RHSI(I) = RHSI(I)-T(JI)*RHSI(J)
DO 4 IELEM=1, NELEMS
READ(ITL)(S(I), I=1, ISIZS)
DO 5 I=2, IORD
K=I-1
DO 5 J=1, K
JI = J+(I*I-I)/2
DO 5 M=1, IORD
MDUM = (M-1)*IORD
IM = I+MDUM
JM = J+MDUM
5 S(IM) = S(IM)-T(JI)*S(JM)
WRITE(ISOL2)(T(I), I=1, ISIZT)
NNE=IELEM+1
DO 800 KK=1, IORD
800 ONED(IELEM, KK)=RHSI(KK)
DO 10 I=1, IORD
II = (I*I+I)/2
10 COE(I) = T(II)
READ(ITL)(T(I), I=1, ISIZT)
DO 601 KK=1, IORD
601 RHS2(KK)=ONED(NNE, KK)
DO 11 I=1, IORD
II = (I*I+I)/2
LEAD = (I-1)*IORD
DO 12 M=I, IORD
LEADM = (M-1)*IORD
JM = I+(M*M-M)/2
DO 12 J=1, IORD
JI = LEAD+J
```



```
IF(M-I) 105,106,105
106 T(II) = T(II)-S(JI)*S(JI)/COE(J)
   S(JI) = S(JI)/COE(J)
   GO TO 12
105 JM = LEADM+J
   T(IM) = T(IM)-S(JI)*S(JM)
12 CONTINUE
   DO 11 J=1,IORD
   JI = LEAD+J
11 RHS2(I) = RHS2(I)-S(JI)*RHS1(J)
   DO 15 I=2,IORD
   L = I+1
   K = I-1
   DO 15 J=1,K
   JI = J+(I*I-I)/2
   JJ = (J*J+J)/2
   II = (I*I+I)/2
   T(JI) = T(JI)/T(JJ)
   T(II) = T(II)-T(JI)*T(JI)*T(JJ)
   IF (I-IORD) 109,15,109
109 DO 17 M=L,IORD
   IM = I+(M*M-M)/2
   JM = J+(M*M-M)/2
17 T(IM) = T(IM)-T(JI)*T(JM)
15 RHS2(I) = RHS2(I)-T(JI)*RHS2(J)
   DO 18 I=1,IORD
18 RHS1(I) = RHS2(I)
   4 WRITE(ISO2)(S(I),I=1,ISIZS)
   WRITE(ISO2)(T(I),I=1,ISIZT)
801 ONED(NELEMS+1,KK)=RHS1(KK)
   BACKSPACE ISO2
   REWIND ITL
   DO 202 IELEM=1,NELP1
   KKK=(IELEM-1)*IORD
   DO 803 I=1,IORD
   KKTE=KKK+I
```

```
S(KKTE)=ONED(IELEM,I)
803 CONTINUE
202 CONTINUE
DO 804 IELEM=1,NELP1
KKK=(IELEM-1)*IORD
DO 820 I=1,IORD
KKTE=KKK+I
KE=NELP1-IELEM + 1
ONED(KE,I)=S(KKTE)
820 CONTINUE
804 CONTINUE
DO 805 I=1,IORD
RHS1(I)=ONED(I,I)
805 READ(ISOL2)(T(I),I=1,ISIZT)
ISOLB = ITL
WRITE(ISOLB)(T(I),I=1,ISIZT)
BACKSPACE ISOL2
KORNER = (IORD*IORD+IORD)/2
RHS1(IORD) = RHS1(IORD)/T(KORNER)
IOM1 = IORD-1
DO 21 KK=1,IOM1
I = IORD-KK
II = (I*I+I)/2
RHS1(I) = RHS1(I)/T(II)
K = I+1
DO 21 J=K,IORD
IJ = I+(J*J-J)/2
21 RHS1(I) = RHS1(I)-RHS1(J)*T(IJ)
DO 806 I=1,IORD
806 ONED(I,I)=RHS1(I)
DO 23 IELEM=1,NELEMS
DO 24 I=1,IORD
24 RHS2(I)=RHS1(I)
NNE=IELEM + 1
DO 807 KK=1,IORD
807 RHS1(KK)=ONED(NNE,KK)
BACKSPACE ISOL2
```

```
BACKSPACE ISOL2
READ(ISOL2)(T(I),I=1,ISIZT)
READ(ISOL2)(S(I),I=1,ISIZS)
WRITE(ISOLB)(S(I),I=1,ISIZS)
WRITE(ISOLB)(T(I),I=1,ISIZT)
BACKSPACE ISOL2
BACKSPACE ISOL2
602 DO 25 KK=1,IORD
    I=IORD-KK+1
    II=(I+I)/2
    RHS1(I)=RHS1(I)/T(II)
    K=I+1
    IF(KK-1)107,108,107
107 DO 26 J=K,IORD
    IJ=I+(J-J)/2
26 RHS1(I)=RHS1(I)-RHS1(J)*T(IJ)
108 DO 25 J=1,IORD
    IJ=I+(J-1)*IORD
25 RHS1(I)=RHS1(I)-RHS2(J)*S(IJ)
DO 808 KK=1,IORD
808 ONED(NNE, KK)=RHS1(KK)
23 CONTINUE
DO 809 IELEM=1,NELP1
KKK=(IELEM-1)*IORD
DO 810 I=1,IORD
KKTE=KKK+I
S(KKTE)=ONED(IELEM,I)
810 CONTINUE
809 CONTINUE
DO 811 IELEM=1,NELP1
KKK=(IELEM-1)*IORD
DO 821 I=1,IORD
KKTE=KKK+I
KE=NELP1-IELEM + 1
ONED(KE, I)=S(KKTE)
821 CONTINUE
811 CONTINUE
```

```
REWIND ISOLB
IF(ISAVE.NE.0)RETURN
REWIND ISOL2
READ(ITL)(T(I),I=1,ISIZT)
WRITE(ISCRA3)(T(I),I=1,ISIZT)
DO 400 I=1,NELEMS
READ(ITL)(S(J),J=1,ISIZS)
WRITE(ISCRA3)(S(J),J=1,ISIZS)
READ(ITL)(T(J),J=1,ISIZT)
WRITE(ISCRA3)(T(J),J=1,ISIZT)
400 CONTINUE
DO 401 I=1,NELEMS
READ(ISOL2)(T(J),J=1,ISIZT)
WRITE(ISCRA3)(T(J),J=1,ISIZT)
READ(ISOL2)(S(J),J=1,ISIZS)
WRITE(ISCRA3)(S(J),J=1,ISIZS)
401 CONTINUE
READ(ISOL2)(T(J),J=1,ISIZT)
WRITE(ISCRA3)(T(J),J=1,ISIZT)
REWIND ITL
REWIND ISOL2
RETURN
END
SUBROUTINE DISPL1
COMMON /A7/ ITL,ISOL2,ISCRA3,NELEMS,NH,IPAGE,PROBLE
COMMON /B9/ IA,IB
COMMON /D10/ ISAVE,IDISPL
COMMON /G4/ ONED(51,68)
DOUBLE PRECISION T(2346),RHS1(68),S(4624),RHS2(68)
DOUBLE PRECISION ONED
IF(IDISPL.NE.1)GO TO 501
NTAA=ITL
ITL=ISOL2
ISOL2=NTAA
501 CONTINUE
IORD=4*NH
ISIZS = IORD*IORD
```

```
ISIZT = (ISIZS+IORD)/2
NELP1 = NELEMS+1
IF(IDISPL.NE.0)GO TO 500
REWIND ITL
REWIND ISOL2
READ(ISCRA3)(T(I),I=1,ISIZT)
WRITE(ITL)(T(I),I=1,ISIZT)
DG 400 I=1,NELEMS
READ(ISCRA3)(S(J),J=1,ISIZS)
WRITE(ITL)(S(J),J=1,ISIZS)
READ(ISCRA3)(T(J),J=1,ISIZT)
WRITE(ITL)(T(J),J=1,ISIZT)
400 CONTINUE
DO 401 I=1,NELEMS
READ(ISCRA3)(T(J),J=1,ISIZT)
WRITE(ISOL2)(T(J),J=1,ISIZT)
READ(ISCRA3)(S(J),J=1,ISIZS)
WRITE(ISOL2)(S(J),J=1,ISIZS)
401 CONTINUE
READ(ISCRA3)(T(J),J=1,ISIZT)
WRITE(ISOL2)(T(J),J=1,ISIZT)
REWIND ITL
REWIND ISOL2
500 READ(ISOL2)(T(I),I=1,ISIZT)
DO 600 I=1,IORD
600 RHS1(I)=ONED(I,I)
DO 1 I=2,IORD
K = I-1
DO 1 J=1,K
JI = J+(I*J-1)/2
1 RHS1(I) = RHS1(I)-T(JI)*RHS1(J)
DO 3 IELEM=1,NELEMS
READ(ISOL2)(S(I),I=1,ISIZS)
NNE=IELEM+1
DO 800 KK=1,IORD
800 ONED(IELEM,KK)=RHS1(KK)
READ(ISOL2)(T(I),I=1,ISIZT)
```

```
DO 601 KK=1, IORD
601 RHS2(KK)=ONED(NNE, KK)
DO 4 I=1, IORD
LEAD = (I-1)*IORD
DO 4 J=1, IORD
JI = LEAD+J
4 RHS2(I) = RHS2(I)-S(JI)*RHS1(J)
DO 6 I=2, IORD
K = I-1
DO 6 J=1, K
JI = J+(I*J-1)/2
6 RHS2(I) = RHS2(I)-T(JI)*RHS2(J)
DO 3 I=1, IORD
3 RHS1(I) = RHS2(I)
DO 801 KK=1, IORD
801 ONED(NELEMS+1, KK)=RHS1(KK)
DO 202 IELEM=1, NELP1
KKK=(IELEM-1)*IORD
DO 803 I=1, IORD
KKTE=KKK+I
S(KKTE)=ONED(IELEM, I)
803 CONTINUE
202 CONTINUE
DO 804 IELEM=1, NELP1
KKK=(IELEM-1)*IORD
DO 820 I=1, IORD
KKTE=KKK+I
KE=NELP1-IELEM + 1
ONED(KE, I)=S(KKTE)
820 CONTINUE
804 CONTINUE
DO 805 I=1, IORD
805 RHS1(I)=ONED(I, I)
REWIND ISOL2
READ(ITL)(T(I), I=1, ISIZT)
KORNER = (IORD*IORD+IORD)/2
RHS1(IORD) = RHS1(IORD)/T(KORNER)
```

```
IOM1 = IORD-1
DO 11 KK=1, IOM1
  I = IORD-KK
  II = (I*I+I)/2
  RHS1(I) = RHS1(I)/T(I)
  K = I+1
  DO 11 J=K, IORD
    IJ = I+(J*J-J)/2
    11 RHS1(I) = RHS1(I)-RHS1(J)*T(IJ)
    DO 806 I=1, IORD
      ONED(I, I)=RHS1(I)
    DO 13 IELEM=1, NELEMS
      DO 14 I=1, IORD
        14 RHS2(I) = RHS1(I)
        NNE=IELEM +1
      DO 807 KK=1, IORD
        RHS1(KK)=ONED(NNE, KK)
      READ(ITL)(S(I), I=1, ISIZS)
      READ(ITL)(T(I), I=1, ISIZT)
      DO 15 KK=1, IORD
        I = IORD-KK+1
        II = (I*I+I)/2
        RHS1(I) = RHS1(I)/T(II)
        K = I+1
        IF (KK-1) 101, 102, 101
      101 DO 16 J=K, IORD
        IJ = I+(J*J-J)/2
        16 RHS1(I) = RHS1(I)-RHS1(J)*T(IJ)
      102 DO 15 J=1, IORD
        IJ = I+(J-1)*IORD
        15 RHS1(I) = RHS1(I)-RHS2(J)*S(IJ)
      DO 808 KK=1, IORD
        ONED(NNE, KK)=RHS1(KK)
      13 CONTINUE
      DO 809 IELEM=1, NELP1
        KKK=(IELEM-1)*IORD
      DO 810 I=1, IORD
```

113

```

KKTE=KKK+I
S(KKTE)=ONED(IELEM,I)
810 CONTINUE
809 CONTINUE
DO 811 IELEM=1,NELP1
KKK=(IELEM-1)*IORD
DO 821 I=1,IORD
KKTE=KKK+I
KE=NELP1-IELEM + 1
ONED(KE,I)=S(KKTE)
821 CONTINUE
811 CONTINUE
REWIND ITL
IDISPL=IDISPL+1
RETURN
END
SUBROUTINE STRESS
COMMON /A5/ EM(50),POISSO(50)
COMMON /A7/ NTAPE,ISCRAL,ISCRA3,NELEMS,NH,IPAGE,PROBLE
COMMON /B9/ IA,IB
COMMON /B10/ C(8,8),PHPMID,RMID,CPMD,SPMD,PHPP,ARCL
COMMON /B11/ ST(50,10,6),STR(50,10,8),T(10),CAPA(6)
COMMON /B14/ XTHETA(10),NUNKTH
COMMON /B18/ NAMES,HS
COMMON /B19/ IELRES(10),REST(50,8),NEREST
COMMON /B22/ Q3(51,68)
COMMON /D10/ ISAVE, IDISPL
COMMON /D40/ PHPRMI(50),RMDI(50),CPDM(50),SPDM(50),PPPH(50),
IARLC(50),CHEKC(50,8,8),R02(50),A1L(50),A2L(50),A3L(50),AL157(50)
COMMON /G2/ NCE(50),NAET(50),THETE(50,51),THICKC(50,51)
COMMON /G4/ ONED(51,68)
DOUBLE PRECISION ONED
DIMENSION DRE(4)
REWIND ISCR3
C NN-TOTAL NO. OF Q'S FOR ONE ELEMENT
CALL SOLV
AXT=0.

```



```
TANT=0.
RADT=0.
ANGT=0.
DO 100 II=1,NUNKTH
WRITE(6,200)IPAGE
IPAGE = IPAGE +1
WRITE(6,501)
501 FORMAT(50X,32HDISPLACEMENT COMPONENTS OF NODES/)
WRITE(6,503)XTHETA(II)
WRITE(6,502)
502 FORMAT(23X,8HNODE NO.,10X,5HAXIAL,12X,10HTANGENTIAL,11X,6HRADIAL,
112X,7HANGULAR/)
503 FORMAT(55X,6HXTHETA=F8.3,3X,7HDEGREES/)
NNODES=NELEMS+1
DO 10 IJ=1,NNODES
DO 102 IN=1,4
DRE(IN)=0.
102 CONTINUE
104 INE=IJ
105 IF(IA.EQ.0)GO TO 106
DO 107 IG=1,IA
ID=(IG-1)*4 + 1
IT=IG - 1
TI=IT
TH=TI*XTHETA(II)/57.2957795
DRE(1)=DRE(1) + Q3(INE, ID)*COS(TH)
DRE(2)=DRE(2) + Q3(INE, ID+1)*SIN(TH)
DRE(3)=DRE(3) + Q3(INE, ID+2)*COS(TH)
DRE(4)=DRE(4) + Q3(INE, ID+3)*SIN(TH)
107 CONTINUE
106 IF(IB.EQ.0)GO TO 108
DO 109 IG=1,IB
ID=(IG-1)*4+1+4*IA
IT=IG - 1
TI=IT
TH=TI*XTHETA(II)/57.2957795
DRE(1)=DRE(1)+Q3(INE, ID)*SIN(TH)
```

```

DRE(2)=DRE(2)+Q3(INE, ID+1)*COS(TH)
DRE(3)=DRE(3)+Q3(INE, ID+2)*SIN(TH)
DRE(4)=DRE(4)+Q3(INE, ID+3)*SIN(TH)
109 CONTINUE
108 IF(ABS(DRE(1)).LT.ABS(AXT))GO TO 170
AXT=DRE(1)
W1=XTHETA(II)
I2=INE
170 IF(ABS(DRE(2)).LT.ABS(TANT))GO TO 171
TANT=DRE(2)
W3=XTHETA(II)
I4=INE
171 IF(ABS(DRE(3)).LT.ABS(RADT))GO TO 172
RADT=DRE(3)
W5=XTHETA(II)
I6=INE
172 IF(ABS(DRE(4)).LT.ABS(ANGT))GO TO 173
ANGT=DRE(4)
W7=YTHETA(II)
I8=INE
173 CONTINUE
WRITE(6,110)INE,(DRE(IK),IK=1,4)
110 FORMAT(26X,I2,4X,4(4X,E15.8))
10 CONTINUE
100 CONTINUE
DO 210 I=1,NUNKTH
WRITE(6,200)IPAGE
IPAGE = IPAGE + 1
200 FORMAT(1H1,62X,4HPAGEI3(/))
WRITE(6,201)
201 FORMAT(37X,58HSTRAINS AND CURVATURES AT THE CENTER OF EACH SHELL E
ELEMENT/)
WRITE(6,202)XTHETA(I)
202 FORMAT(55X,6HTHETA=F8.3,3X,7HDEGREES/)
WRITE(6,204)
204 FORMAT(4X,11HELEMENT NO.,10X,4HE(S),13X,8HE THETA ,10X,10HE(S) THE
1A),12X,4HK(S),13X,8HK(THETA),10X,10HK(S,THETA)/)

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```
DO 211 J=1,NELEMS
WRITE(6,203)J,ST(J,I,K),K=1,6}
203 FORMAT(8X,I2,5X,6(4X,E15.8)}
211 CONTINUE
210 CONTINUE
DO 310 I=1,NUNKTH
WRITE(6,200)IPAGE
IPAGE = IPAGE + 1
WRITE(6,301)
301 FORMAT(39X,53HSTRESS RESULTANTS AT THE CENTER OF EACH SHELL ELEMEN
1T/)
WRITE(6,202)XTHETA(I)
WRITE(6,304)
304 FORMAT(1X,11HELEMENT NO.,6X,4HN(S),9X,8HN(THETA),6X,10HN(S-THETA),
18X,4HM(S),9X,8HM(THETA),6X,10HM(S-THETA),8X,4HQ(S),9X,8HQ(THETA))//
2)
DO 311 J=1,NELEMS
WRITE(6,208)J,STR(J,I,K),K=1,8}
208 FORMAT(5X,I2,4X,8(2X,E13.6)}
311 CONTINUE
310 CONTINUE
WRITE(6,200)IPAGE
IPAGE = IPAGE + 1
WRITE(6,150)
150 FORMAT(35X,62HMAXIMUM COMPUTED NODE DISPLACEMENT COMPONENTS OF THE
1 STRUCTURE////)
WRITE(6,151)
151 FORMAT(23X,5HAXIAL,20X,10HTANGENTIAL,19X,6HRADIAL,20X,7HANGULAR//
1)
WRITE(6,152)AXT,TANT,RADT,ANGT
152 FORMAT(6X,4(12X,E15.8)///)
WRITE(6,153)
153 FORMAT(4(27H
WRITE(6,154)
154 FORMAT(1H0,5X,4(13X,14HTHETA NODE)///)
WRITE(6,155)W1,I2,W3,14,W5,I6,W7,I8
155 FORMAT(1H0,4X,4(12X,F8.3,5X,I2)///)
```

```

SN = 0.
TN = 0.
TSN= 0.
SM = 0.
TM = 0.
TSM= 0.
QS=0.
QST=0.
DO 180 I=1,NELEMS
DO 181 J=1,NUNKTH
IF(ABS(STR(I,J,1)).LT.ABS(SN))GO TO 190
SN=STR(I,J,1)
W1=XTHETA(J)
I2=I
190 IF(ABS(STR(I,J,2)).LT.ABS(TN))GO TO 191
TN=STR(I,J,2)
W3=XTHETA(J)
I4=I
191 IF(ABS(STR(I,J,3)).LT.ABS(TSN))GO TO 192
TSN=STR(I,J,3)
W5=XTHETA(J)
I6=I
192 IF(ABS(STR(I,J,4)).LT.ABS(SM))GO TO 193
SM=STR(I,J,4)
W7=XTHETA(J)
I8=I
193 IF(ABS(STR(I,J,5)).LT.ABS(TM))GO TO 194
TM=STR(I,J,5)
W9=XTHETA(J)
I10=I
194 IF(ABS(STR(I,J,6)).LT.ABS(TSM))GO TO 195
TSM=STR(I,J,6)
W11=XTHETA(J)
I12=I
195 IF(ABS(STR(I,J,7)).LT.ABS(QS))GO TO 196
QS=STR(I,J,7)
W13=XTHETA(J)

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I14=I
196 IF(ABS(STR(I,J,8)).LT.ABS(QST))GO TO 197
   QST=STR(I,J,8)
   W15=XTHETA(J)
   I16=I
197 CONTINUE
181 CONTINUE
180 CONTINUE
   WRITE(6,156)
156 FORMAT(1H0,34X,61HMAXIMUM COMPUTED STRESS RESULTANT COMPONENTS OF
1THE STRUCTURE////)
   WRITE(6,157)
157 FORMAT(8X,4HN(S),10X,8HN(THETA),7X,10HN(S-THETA),9X,4HM(S),10X,
18HM(THETA),7X,10HM(S-THETA),9X,4HQ(S),10X,8HQ(THETA)///)
   WRITE(6,158)SN,TN,TSN,SM,TM,TSM,QS,QST
158 FORMAT(8(3X,E13.6)///)
   WRITE(6,159)
159 FORMAT(1H0,8X,2HAT,7(14X,2HAT)///)
   WRITE(6,160)
160 FORMAT(8(3X,13HELEMENT THETA)///)
   WRITE(6,161)I2,W1,I4,W3,I6,W5,I8,W7,I10,W9,I12,W11,I14,W13,I16,W15
161 FORMAT(1H0,8(4X,I2,2X,F8.3)///)
RETURN
END
SUBROUTINE SOLV
COMMON /A5/ EM(50),POISSO(50)
COMMON /A7/ NTAPE,ISCRAL,ISCRAB,NELEMS,NH,IPAGE,PROBLE
COMMON /B9/ IA,IB
COMMON /B10/ C(8,8),PHPMID,RMID,CPMD,SPMD,PHPP,ARCL
COMMON /B11/ ST(50,10,6),STR(50,10,8),T(10),CAPA(6)
COMMON /B14/ XTHETA(10),NUNKTH
COMMON /B18/ NAMES,HS
COMMON /B19/ IELRES(10),REST(50,8),NEREST
COMMON /B22/ Q3(51,68)
COMMON /D40/ PHPRMI(50),RMDI(50),CPDM(50),SPDM(50),PPPH(50),
1ARLC(50),CHEKCC(50,8),R02(50),A2L(50),A3L(50),AL(57,50)
COMMON /G2/ NCE(50),NAET(50),THEYE(50,51),THICKC(50,51)

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```
COMMON /G4/ ONED(51,68)
DOUBLE PRECISION ONED
DIMENSION Q1(68), Q2(68)
DIMENSION Q(8), AB(17,8)
DIMENSION THETA(51), THIC(51)
NN=4*(IA+IB)
DO 800 I=1, NN
  800 Q1(I)=ONED(I, I)
DO 300 II=1, NN
  300 Q3(1, II)=Q1(II)
DO 10 IJ=1, NELEMS
  DO 801 I=1, NN
    801 Q2(I)=ONED(IJ+1, I)
  DO 301 II=1, NN
    Q3(IJ+1, II)=Q2(II)
  301 CONTINUE
C KA-COUNTER FOR ADDRESSING Q'S IN THE ARRAYS Q1 AND Q2
C IAB-TOTAL NO. OF HARMONICS FOR ONE NODE
PHPMID=PHPRMI(IJ)
RMID=RMDI(IJ)
CPMD=CPDM(IJ)
SPMD=SPDM(IJ)
PHPP=PPPH(IJ)
ARCL=ARLC(IJ)
DO 4939 ITS1=1, 8
  DO 4940 ITS2=1, 8
    C(ITS1, ITS2)=CHEKC(IJ, ITS1, ITS2)
  4940 CONTINUE
  4939 CONTINUE
KA=1
IAB=IA+IB
C COMPUTE ALPHAS FOR ALL HARMONICS AT ONE NODE
DO 15 I=1, IAB
  C PUT Q1'S AND Q2'S FOR ONE HARMONIC INTO COLUMN MATRIX
  DO 20 K=1, 4
    Q(K)=Q1(KA)
    Q(K+4)=Q2(KA)
```

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KA=KA+1
20 CONTINUE
C MULTIPLY GIVEN 8X8 TRANSFORMATION MATRIX C BY COLUMN MATRIX OF ALPHAS
C FOR THE FIRST HARMONIC AND STORE IN ARRAY AB
DO 25 J=1,8
SUM=0.
DO 30 M=1,8
SUM=C(J,M)*Q(M)+SUM
30 CONTINUE
AB(I,J)=SUM
25 CONTINUE
15 CONTINUE
NC=NCE(IJ)
NAME=NAET(IJ)
DO 4937 ITH=1,NC
THETA(ITH)=THETE(IJ,ITH)
THIC(ITH)=THICKC(IJ,ITH)
4937 CONTINUE
C DO COMPUTATION FOR N NUMBER OF THETAS PER ELEMENT
DO 35 KJ=1,NUNKTH
IF(NC.NE.1)GO TO 100
T(KJ) = THIC(1)
GO TO 101
100 CONTINUE
DO 65 J=1,NC
TH2=THETA(J+1)*57.2957795
TH1=THETA(J)*57.2957795
IF(XTHETA(KJ).GE.0.AND.XTHETA(KJ).LE.0)TH2)GO TO 12
65 CONTINUE
12 T(KJ)=THIC(J)
C COMPUTE STRAIN RESULTANTS AND RETURN IN ARRAY ST
101 CONTINUE
CALL STRAIN(AB,KJ,IAB,IJ)
C COMPUTE STRESS RESULTANTS AND RETURN IN ARRAY STR(I,J,K) WHERE I=ELEMENT
C COMPUTE CAPAS-PARTIAL DERIVATIVES OF THE RADIUS OF CURVATURE
C NO. J=THETA NO. K=COLUMN MATRIX 1-8 GIVING VALUES OF NS,NTHETA,NSTHETA,
C MS, MTHETA, MSTHETA, QS, QTHETA RESPECTIVELY

```

```

CALL STRESR(IJ,KJ)
35 CONTINUE
C MOVE Q'S FOR NODE P+1 TO NODE P
DO 40 L=1,NN
Q1(L)=Q2(L)
40 CONTINUE
10 CONTINUE
RETURN
END
SUBROUTINE STRAIN(AB,KJ,IAB,IJ)
COMMON /A5/ EM(50),POISSO(50)
COMMON /B9/ IA,IB
COMMON /B10/ C(8,8),PHPMID,RMID,CPMD,SPMD,PHPP,ARCL
COMMON /B11/ ST(50,10,6),STR(50,10,8),T(10),CAPA(6)
COMMON /B14/ XTHETA(10),NUNKTH
COMMON /B19/ IELRES(10),REST(50,8),NEREST
DIMENSION AB(17,8),U(40)
S=ARCL/2.
DO 45 I=1,40
U(I)=0.
45 CONTINUE
C TEST FOR A HARMONICS, IF NO, GO TO TEST FOR B HARMONICS
IF(IA.EQ.0) GO TO 110
C COMPUTE US-OR U(1),VTHETA-U(2),UTHEAT-U(3),VS-U(4),WS-U(5),WSS-U(6),
C WTHETA-U(7),WTHETATHETA-U(8),U-U(9),V-V(10),W-U(11),WSTHETA-U(12),
C WSSS-U(13),WSSSTHETA-U(14),WTHETATHETAS-U(15),WTHETATHETATHETA-U(16),
C UTHETATHETA-U(17),USTHETA-U(18),VTHETATHETA-U(19),VSTHETA-U(20) IE
C U,V,W, AND THEIR PARTIAL DERIVATIVES FOR A HARMONICS
DO 50 I=1,IA
AI=I-1
X=AI*XTHETA(KJ)
X=X/57.29577795
U(1)=AB(I,6)*COS(X)+U(1)
U(2)=AI*(AB(I,7)+AB(I,8)*S)*COS(X)+U(2)
U(3)=-AI*(AB(I,5)+AB(I,6)*S)*SIN(X)+U(3)
U(4)=AB(I,8)*SIN(X)+U(4)
U(5)=AB(I,2)+2.*AB(I,3)*S+3.*AB(I,4)*S**2)*COS(X)+U(5)

```



```

U(6)=(2.*AB(I,3)+6.*AB(I,4)*S)*COS(X)+U(6)
U(7)=-{(AI*(AB(I,1)+AB(I,2)*S+AB(I,3)*S**2+AB(I,4)*S**3)*SIN(X)}
1+U(7)
U(8)=-{(AI**2)*(AB(I,1)+AB(I,2)*S+AB(I,3)*S**2+AB(I,4)*S**3)*
1COS(X)}+U(8)
U(9)=(AB(I,5)+AB(I,6)*S)*COS(X)+U(9)
U(10)=(AB(I,7)+AB(I,8)*S)*SIN(X)+U(10)
U(11)=(AB(I,1)+AB(I,2)*S+AB(I,3)*S**2+AB(I,4)*S**3)*COS(X)+U(11)
U(12)=-AI*(AB(I,2)+2.*AB(I,3)*S+3.*AB(I,4)*S**2)*SIN(X)+U(12)
U(13)=6.*AB(I,4)*COS(X)+U(13)
U(14)=-{(AI*(2.*AB(I,3)+6.*AB(I,4)*S)*SIN(X)}+U(14)
U(15)=-{(AI**2)*(AB(I,2)+2.*AB(I,3)*S+3.*AB(I,4)*S**2)*COS(X)}+
1 U(15)
U(16)={(AI**3)*(AB(I,1)+AB(I,2)*S+AB(I,3)*S**2+AB(I,4)*S**3)*SIN(X)}
1 +U(16)
U(17)=-{(AI**2)*(AB(I,5)+AB(I,6)*S)*COS(X)}+U(17)
U(18)=-AI*AB(I,6)*SIN(X)+U(18)
U(19)=-{(AI**2)*(AB(I,7)+AB(I,8)*S)*SIN(X)}+U(19)
U(20)=AI*AB(I,8)*COS(X)+U(20)
50 CONTINUE
C CHECK FOR B HARMONICS
110 IF(IAB.EQ.1A) GO TO 120
K=IA+1
DO 55 J=K,IAB
AI=J-IA-1
Y=AI*XTHETA(KJ)
Y=Y/57.2957795
U(21)=AB(J,6)*SIN(Y)+U(21)
U(22)=-{(AI*(AB(J,7)+AB(J,8)*S)*SIN(Y)}+U(22)
U(23)=AI*(AB(J,5)+AB(J,6)*S)*COS(Y)+U(23)
U(24)=AB(J,8)*COS(Y)+U(24)
U(25)={AB(J,2)+2.*AB(J,3)*S+3.*AB(J,4)*S**2)*SIN(Y)+U(25)
U(26)={AB(J,3)+2.*6.*AB(J,4)*S)*SIN(Y)+U(26)
U(27)=AI*(AB(J,1)+AB(J,2)*S+AB(J,3)*S**2+AB(J,4)*S**3)*COS(Y)+
1U(27)
U(28)=-{(AI**2)*(AB(J,1)+AB(J,2)*S+AB(J,3)*S**2+AB(J,4)*S**3)*

```

```

1 SIN(Y))+U(28)
U(29)=(AB(J,5)+AB(J,6)*S)*SIN(Y)+U(29)
U(30)=(AB(J,7)+AB(J,8)*S)*COS(Y)+U(30)
U(31)=(AB(J,1)+AB(J,2)*S+AB(J,3)*S**2+AB(J,4)*S**3)*SIN(Y)+U(31)
U(32)=AI*(AB(J,2)+2.*AB(J,3)*S+3.*AB(J,4)*S**2)*COS(Y)+U(32)
U(33)=6.*AB(J,4)*SIN(Y)+U(33)
U(34)=AI*(2.*AB(J,3)+6.*AB(J,4)*S)*COS(Y)+U(34)
U(35)=-((AI**2)*(AB(J,2)+2.*AB(J,3)*S+3.*AB(J,4)*S**2)*SIN(Y))+
1 U(35)
U(36)=-((AI**3)*(AB(J,1)+AB(J,2)*S+AB(J,3)*S**2+AB(J,4)*S**3)*
1 COS(Y))+U(36)
U(37)=-((AI**2)*(AB(J,5)+AB(J,6)*S)*SIN(Y))+U(37)
U(38)=AI*AB(J,6)*COS(Y)+U(38)
U(39)=-((AI**2)*(AB(J,7)+AB(J,8)*S)*COS(Y))+U(39)
U(40)=-AI*AB(J,8)*SIN(Y)+U(40)

```

55 CONTINUE

C ADD THE A AND B HARMONICS FOR AN ELEMENT

120 DO 60 I=1,20

U(I)=U(I)+U(I+20)

60 CONTINUE

C USING THE DISPLACEMENT FUNCTIONS AND THEIR PARTIALS, COMPUTE THE NORMAL

C STRAINS AND SHEAR STRAIN FOR AN ELEMENT

ES=U(1)-PHPMID*U(11)

ETHETA=1./RMID*(U(2)+U(9)*SPMD+U(11)*CPMD)

ESTHE=1./RMID*U(3)-U(10)/RMID*SPMD+U(4)

C COMPUTE STRAIN ENERGY USING EQNS. FOR THE STRAINS.

C COMPUTE 3 CURVATURE CHANGES OF SHELL SURFACE.

C ALL EQNS. FOR ONE ANGLE THETA OF ONE ELEMENT.

ST(IJ,KJ,1) = ES

ST(IJ,KJ,2) = ETHETA

ST(IJ,KJ,3) = ESTHE

ST(IJ,KJ,4) =

1 -(U(6)+U(9)*PHPP+PHPMID*U(1))

ST(IJ,KJ,5) =

1 -1./RMID*(1./RMID*U(8)+U(5)*SPMD+PHPMID*U(9)*SPMD-1./RMID*

IU(2)*CPMD)

ST(IJ,KJ,6) =

```

1 1.0/RMID*(2.0/RMID*U(7)*SPMD-2.0/RMID*U(10)*SPMD*CPMD-2.0*U(12)
1-PHPMID*U(3)+CPMD*U(4)-U(10)*SPMD*PHPMID)
CAPA(1)=-((U(13)+2.0*U(1)*PHPP)
CAPA(2)=-((U(14)+U(3)*PHPP +U(18)*PHPMID)
CAPA(3)=(1.0/RMID**2)*(-U(15)+(2.0/RMID)*SPMD*U(8)-RMID*SPMD*U(6)-
RMID*PHPMID*CPMD*U(5)+SPMD**2*U(5)-PHPP *RMID*SPMD*U(9)
+SPMD**2*PHPMID*U(9)-RMID*PHPMID*SPMD*U(1)-PHPMID**2*RMID
*CPMD*U(9)+CPMD*U(20)-(2.0/RMID)*CPMD*SPMD*U(2)-PHPMID*
SPMD*U(2))
CAPA(4)=(1.0/RMID**2)*(-U(16)-RMID*SPMD*U(12)-RMID*PHPMID*SPMD*
U(3)+CPMD*U(19))
1 CAPA(5)=(1.0/RMID**2)*(-(4.0/RMID)*SPMD**2*U(7)+2.0*SPMD*U(12)+2.0*
PHPMID*CPMD*U(7)+(4.0/RMID)*SPMD**2*CPMD*U(10)-2.0*SPMD*
CPMD*U(4)-2.0*CPMD**2*PHPMID*U(10)+2.0*SPMD**2*PHPMID*
U(10)+2.0*SPMD*U(12)-2.0*RMID*U(14)-RMID*PHPP*U(3)+PHPMID
*SPMD*U(3)-PHPMID*RMID*U(18)-PHPMID*RMID*SPMD*U(4)-SPMD
*CPMD*U(4)-RMID*SPMD*U(4)*PHPMID-PHPMID**2*RMID*CPMD*
U(10)+SPMD**2*PHPMID*U(10)-RMID*SPMD*PHPP*U(10))
CAPA(6)=(1.0/RMID)*((2.0/RMID)*SPMD*U(8)-(2.0/RMID)*SPMD*CPMD*U(2)
-2.0*U(15)-PHPMID*U(17)+CPMD*U(20)-SPMD*PHPMID*U(2))
1 RETURN
END
SUBROUTINE STRESR(IJ,KJ)
COMMON /A5/ EM(50),POISSO(50)
COMMON /B9/ IA,IB
COMMON /B10/ C(8,8),PHPMID, RMID,CPMD,SPMD,PHPP,ARCL
COMMON /B11/ ST(50,10,6),STR(50,10,8),T(10),CAPA(6)
COMMON /B14/ XTHETA(10),NUNKTH
COMMON /B18/ NAMES,HS
COMMON /B19/ IELRES(10),REST(50,8),NEREST
INTEGER SAND
DATA SAND/4HSAND/
EE1 = EM(IJ)
FNU1 = POISSO(IJ)
FNU2 = FNU1
IF(NAMES.NE.SAND)GO TO 2
A=2.0*(EE1*T(KJ))/(1.0 - FNU1*FNU2)

```

```

CXX=(EE1*T(KJ)*HS**2)/(2.*{1.-FNUI*FNUI})
GO TO 3
2 A=(EE1*T(KJ))/(1.-FNUI*FNUI)
CXX=(EE1*T(KJ)**3)/(12.*{1.-FNUI*FNUI})
3 B=A
D=CXX
G = EE1/(2.*{1.0 + FNUI})
C FOLLOWING ARE PARTIALS OF MS,MTHETA,MSTHETA
PMSS=CXX*CAPA(1)+FNUI*CXX*CAPA(3)
PMTT=FNUI*2*CAPA(2)+D*CAPA(4)
IF(NAMES.NE.SAND)GO TO 4
PMSTS=G*T(KJ)*HS**2*CAPA(5)/2.
PMSTT=G*T(KJ)*HS**2*CAPA(6)/2.
GO TO 5
4 PMSTS=G*(T(KJ)**3/12.)*CAPA(5)
PMSTT=G*(T(KJ)**3/12.)*CAPA(6)
C COMPUTE STRESS RESULTANTS
5 STR(IJ,KJ,1) = A*ST(IJ,KJ,1) + A*FNUI*ST(IJ,KJ,2)
STR(IJ,KJ,2) = FNUI*2*ST(IJ,KJ,1) + B*ST(IJ,KJ,2)
STR(IJ,KJ,4)=CXX*ST(IJ,KJ,4)+FNUI*CXX*ST(IJ,KJ,5)
STR(IJ,KJ,5) = FNUI*2*ST(IJ,KJ,4) + D*ST(IJ,KJ,5)
STR(IJ,KJ,7)=(1./RMID)*(RMID*PMSS+PMSTT+(STR(IJ,KJ,4)-
1 STR(IJ,KJ,5))*SPMD)
IF(NAMES.NE.SAND)GO TO 6
STR(IJ,KJ,3)=2.*G*T(KJ)*ST(IJ,KJ,3)
STR(IJ,KJ,6)=G*T(KJ)*HS**2*ST(IJ,KJ,6)/2.
STR(IJ,KJ,8)=(1./RMID)*(RMID*PMSTS+PMTT+2.*SPMD*STR(IJ,KJ,6))
RETURN
6 STR(IJ,KJ,3) = G*T(KJ)*ST(IJ,KJ,3)
STR(IJ,KJ,6) = (T(KJ)**3/12.)*ST(IJ,KJ,6)*G
STR(IJ,KJ,8)=(1./RMID)*(RMID*PMSTS+PMTT+2.*SPMD*STR(IJ,KJ,6))
RETURN
END

```

APPENDIX C
LISTING OF THE COMPUTER PROGRAM
FOR THE NONLINEAR ANALYSIS

```

COMMON /A3/ NDP, NET, IXX(12)
COMMON /A5/ EM(50), POISSO(50)
COMMON /A7/ NTAPE, ISCRAI, ISCRA3, NELEMS, NH, IPAGE, PROBLE
COMMON /B9/ IA, IB
COMMON /B14/ XTHETA(10), NUNKTH
COMMON /B18/ NAMES, HS
COMMON /B19/ IELRES(10), REST(50,8), NEREST
COMMON /B24/ RO1(50), PHY1(50), Z1(50), Z2(50)
COMMON /B30/ NA, NB
COMMON /C1/ NDELQ
COMMON /D10/ ISAVE, IDISPL
COMMON /D40/ PHPRMI(50), RMDI(50), CPDM(50), SPDM(50), PPPH(50),
IARLC(50), CHEKC(50,8), R02(50), A1L(50), A2L(50), A3L(50), A157(50),
2A4L(50), A5L(50), A6L(50), A7L(50), A15L(50), A16L(50), A17L(50), A18L(50),
3), A19L(50), A20L(50), A21L(50), A22L(50), A158(50), A159(50)
COMMON /F1/ ITER, EROR, X, ITLAST, NSTRAI, IPLAST, XSM
COMMON /G1/ APPO, THELD, RC, RL, PR
COMMON /G2/ NCE(50), NAET(50), THETA(50,51), THICKC(50,51)
COMMON /G4/ ONEDD(51,48)
COMMON /G5/ ONED(51,48)
COMMON /G9/ TTA(50,50)
DOUBLE PRECISION ONED
INTEGER PROBLE, RERUN, RRUN, APOL, APPO
DATA RERUN/4HRRUN/
INTEGER REST
IDISPL=0
READ(5,101)NPROBL
101 FORMAT(I5)
DO 100 IPROBL=1, NPROBL
CALL TABLES
CALL STIFOR
IF(PROBLE, NE, RERUN)CALL DISPLA
IF(PROBLE, EQ, RERUN)CALL DISPLI
CALL NONLIN
100 CONTINUE
IF(ISAVE, EQ, 0)PRINT 102, ISCRA3

```

102 FORMAT(46X,4IHS AVE TAPE LOGICAL UNIT I2)

STOP

END

SUBROUTINE TABLES

COMMON /A3/ NDP,NET,IXX(12)

COMMON /A5/ EM(50),POISSO(50)

COMMON /A7/ NTAPE,ISCRAI,ISCRA3,NELEMS,NH,IPAGE,PRUBLE

COMMON /B9/ IA,IB

COMMON /B14/ XTHETA(10),NUNKTH

COMMON /B18/ NAMES,HS

COMMON /B19/ IELRES(10),REST(50,8),NEREST

COMMON /B24/ RO1(50),PHY1(50),PHY2(50),Z1(50),Z2(50)

COMMON /B30/ NA,NB

COMMON /C1/ NDELQ

COMMON /D10/ ISAVE, IDISPL

COMMON /D40/ PHPRMI(50),RMDI(50),CPDM(50),SPDM(50),PPPH(50),

1ARLC(50),CHEKC(50,8),RO2(50),AIL(50),A2L(50),A3L(50),AL157(50),

2A4L(50),A5L(50),A6L(50),A7L(50),A15L(50),A16L(50),A17L(50),A18L(50)

3),A19L(50),A20L(50),A21L(50),A22L(50),AL158(50),AL159(50)

COMMON /E1/ ITER,EROR,X,ITLAST,NSTRAI,IPLAST,XSM

COMMON /G1/ APPO,THELO,RC,RL,PR

COMMON /G2/ NCE(50),NAET(50),THETA(50,51),THICKC(50,51)

DIMENSION THET(51),THI(51)

DIMENSION RK(4),RRRE(4)

DIMENSION TITL(40)

INTEGER TITL

INTEGER REST

INTEGER BLAN

INTEGER CONST

INTEGER SAND

INTEGER CONS

INTEGER PROBLE,RERUN,RRUN,APOL,APPO

REAL IXED

DATA SAND/4HSAND/

DATA CONST/4HCONS/

DATA RREST1/4HIXED/,RREST2/3HREE/

DATA NAME/2H ,

```
DATA BLAN/4H /
DATA RERUN/4HRRUN/
NTAPE=1
ISCRAL=2
IPAGE = 1
WRITE(6,604)IPAGE
IPAGE = IPAGE + 1
604 FORMAT(1H1,62X,4HPAGEI3///<>)
WRITE(6,605)
605 FORMAT(56X,20HGENERAL INFORMATION///<>)
DO 600 I =1,20
READ(5,601)(TITL(J),J=1,40)
601 FORMAT(40A2)
IF(TITL(1).EQ.NAME)GO TO 603
WRITE(6,602)(TITL(J),J=1,40)
602 FORMAT(26X,40A2///<>)
600 CONTINUE
603 WRITE(6,620)
620 FORMAT(1H0,55X,20H*****//)
WRITE(6,621)
621 FORMAT(59X,14H*****//)
WRITE(6,622)
622 FORMAT(63X,6H*****)
READ(5,8)IA,IB,ISCRAL,NELEMS,NET,NEREST,NUNKTH,NA,NB,NDELQ,
ITER,ITLAST,ISAVE,NAMES,HS,IDEN,PROBLE,EROR,X,NSTRAI,IPLAST
2,XSM
READ(5,9)APPO,THELO,RC,RL,PR
9 FORMAT(A4,6X,4F10.0)
READ(5,66)(XTHETA(I),I=1,NUNKTH)
66 FORMAT(8F10.0)
NH = IA + IB
IF(PROBLE.EQ.RERUN)WRITE(6,450)IDEN
IF(PROBLE.EQ.RERUN)RETURN
REWIND ISCRAL
8 FORMAT(13I3,A4,F5.0,2A4,2F6.2,2I1,F7.0)
REWIND ISCRAL
WRITE(6,401)IDEN
```



```

401 FORMAT(1H0,54X,22HP R O B L E M A5)
450 FORMAT(1H0,28X,31HR E R U N O F P R O B L E M A5,38H W I T H
IN E W F O R C E I N P U T)
WRITE(6,604)IPAGE
IPAGE = IPAGE + 1
WRITE(6,300)
300 FORMAT(54X,23HN O M E N C L A T U R E//)
WRITE(6,301)
301 FORMAT(16X,40HZ ***** SHELL AXIAL COORDINATE/)
WRITE(6,302)
302 FORMAT(16X,54HR ***** SHELL RADIAL COORDINATE(CYLINDRICA
IL)//)
WRITE(6,303)
303 FORMAT(16X,56HE ***** MODULUS OF ELASTICITY OF SHELL ELE
MENT/)
WRITE(6,304)
304 FORMAT(16X,50HNU ***** POISSON'S RATIO OF SHELL ELEMENT/
)
WRITE(6,305)
305 FORMAT(16X,63HPHI ***** ANGLE BETWEEN MERIDIAN AND AXIS OF
1 REVOLUTION/)
IF(NSTRAI,NE,0)GO TO 480
WRITE(6,306)
306 FORMAT(16X,98HE(S) ***** STRAIN OF THE MIDDLE SURFACE OF TH
1E SHELL ELEMENT ALONG THE MERIDIONAL DIRECTION/)
WRITE(6,307)
307 FORMAT(16X,103HE(THETA) ***** STRAIN OF THE MIDDLE SURFACE OF T
HE SHELL ELEMENT ALONG THE CIRCUMFERENTIAL DIRECTION/)
WRITE(6,308)
308 FORMAT(16X,94HE(S-THETA) ***** STRAIN OF THE MIDDLE SURFACE OF TH
1E SHELL ELEMENT ALONG THE NORMAL DIRECTION/)
WRITE(6,309)
309 FORMAT(16X,83HK(S) ***** CURVATURE CHANGE OF THE SHELL ELEM
ENT SURFACE IN MERIDIONAL PLANE/)
WRITE(6,310)
310 FORMAT(16X,110HK(THETA) ***** CURVATURE CHANGE OF THE SHELL ELE
MENT SURFACE IN THE PLANE PERPENDICULAR TO MERIDIONAL PLANE/)

```

```

WRITE(6,311)
311 FORMAT(16X,52HK(S-THETA) ***** TWIST OF THE SHELL ELEMENT SURFACE
1/)
480 WRITE(6,312)
312 FORMAT(16X,84HN(S) ***** NORMAL FORCE PER UNIT LENGTH OF CI
IRCUMFERENTIAL SECTION OF A SHELL/)
WRITE(6,313)
313 FORMAT(16X,79HN(THETA) ***** NORMAL FORCE PER UNIT LENGTH OF ME
IRIDIONAL SECTION OF A SHELL/)
WRITE(6,314)
314 FORMAT(16X,108HN(S-THETA) ***** SHEARING FORCE PER UNIT LENGTH OF
1 CIRCUMFERENTIAL SECTION OR MERIDIONAL SECTION OF A SHELL/)
WRITE(6,315)
315 FORMAT(16X,86HM(S) ***** BENDING MOMENT PER UNIT LENGTH OF
ICIRCUMFERENTIAL SECTION OF A SHELL/)
WRITE(6,316)
316 FORMAT(16X,81HM(THETA) ***** BENDING MOMENT PER UNIT LENGTH OF
1MERIDIONAL SECTION OF A SHELL/)
WRITE(6,317)
317 FORMAT(16X,109HM(S-THETA) ***** TWISTING MOMENT PER UNIT LENGTH O
IF CIRCUMFERENTIAL SECTION OR MERIDIONAL SECTION OF A SHELL/)
WRITE(6,318)
318 FORMAT(16X,95HQ(S) ***** SHEARING FORCE NORMAL TO THE SHELL
1 PER UNIT LENGTH OF CIRCUMFERENTIAL SECTION/)
WRITE(6,319)
319 FORMAT(16X,90HQ(THETA) ***** SHEARING FORCE NORMAL TO THE SHELL
1 PER UNIT LENGTH OF MERIDIONAL SECTION/)
WRITE(6,620)
WRITE(6,621)
WRITE(6,622)
DO 800 I=1,NELEMS
DO 801 J=1,8
REST(I,J) = 0
801 CONTINUE
800 CONTINUE
READ(5,510)Z1(1),Z2(1),R01(1),R02(1),PHY1(1),PHY2(1)
510 FORMAT(6E12.5)

```

```

IF(PHY2(1).EQ.0.)GO TO 521
RCAP=0.
DO 522 J=2,NELEMS
522 READ(5,510)Z1(J),Z2(J),R01(J),R02(J),PHY1(J),PHY2(J)
GO TO 354
521 CONTINUE
READ(5,3)RCAP,RFILET,RMAX
J = 1
TCR = 0
CALL WEH2(PHY1,PHY2,J,TCR)
NNODES = NELEMS + 1
DO 54 I=1,NNODES
CALL WEH3(I,NP,CORD,RADIUS,PHY2,J,NNODT,S,RCAP,RFILET,RMAX,ZMAX)
IF(I.EQ.NNODES)GO TO 56
Z1(I) = CORD
R01(I) = RADIUS
IF(I.EQ.1)GO TO 54
56 Z2(I - 1) = CORD
R02(I - 1) = RADIUS
54 CONTINUE
354 IELT=0
252 READ(5,2)NAMET,IEL1,IEL2,THIK,NC
2 FORMAT(A4,1X,2I5,F10.0,I5)
IF(NAMET.NE.CONST)GO TO 250
IF(IEL1.NE.0.AND.IEL2.NE.0)GO TO 781
IEL1=1
IEL2=1
781 DO 251 I=IEL1,IEL2
IELT=IELT+1
NCE(I)=1
NAET(I)=NAMET
THETA(I,1)=0.
THICK(I,1)=THIK
251 CONTINUE
IF(IELT.NE.NELEMS)GO TO 252
GO TO 253
250 READ(5,98)(THET(I),THICK(I),I=1,NC)

```

```

IF(IEL1.EQ.0) IEL1=IELT+1
IF(IEL2.EQ.0) IEL2=IELT+1
DO 780 J=IEL1,IEL2
NCE(J)=NC
NAET(J)=NAMEI
DO 460 JE=1,NC
THETA(J,JE)=THET(JE)
THICKC(J,JE)=THI(JE)
460 CONTINUE
780 IELT=IELT+1
IF(IELT.NE.NELEMS) GO TO 252
253 CONTINUE
98 FORMAT(8F10.0)
IELT = 0
262 READ(5,2)NAM,IEL1,IEL2,EM(IEL1)
IF(NAM.NE.CONST) GO TO 260
DO 261 I=IEL1,IEL2
EM(I) = EM(IEL1)
261 CONTINUE
IELT = IEL2
GO TO 265
260 IELT = IELT + 1
EM(IELT) = EM(IEL1)
265 IF(IELT.NE.NELEMS) GO TO 262
IELT = 0
272 READ(5,2)NAM,IEL1,IEL2,POISSO(IEL1)
IF(NAM.NE.CONST) GO TO 270
DO 271 I=IEL1,IEL2
POISSO(I) = POISSO(IEL1)
271 CONTINUE
IELT = IEL2
GO TO 275
270 IELT = IELT + 1
POISSO(IELT) = POISSO(IEL1)
275 IF(IELT.NE.NELEMS) GO TO 272
3 FORMAT(8F10.0)
LINES = 0

```

```

DO 612 J=1,NELEMS
IF(J.NE.1.AND.LINES.LT.24)GO TO 609
WRITE(6,604)IPAGE
PAGE = IPAGE + 1
LINES = 0
WRITE(6,606)
606 FORMAT(44X,45HELASTIC AND GEOMETRIC PROPERTIES OF STRUCTURE//)
WRITE(6,607)
607 FORMAT(15X,7HELEMENT,8X,4HNODE,12X,11HCOORDINATES,18X,17HELASTIC C
IONSTANTS/)
WRITE(6,608)
608 FORMAT(17X,3HND,10X,4HNOS,10X,1HZ,13X,1HR,17X,1HE,12X,2HNU,18X,3
1H9HI//)
609 JJ = J + 1
WRITE(6,610)J,Z1(J),R01(J),EM(J),POISSO(J),PHY1(J),JJ,Z2(J),
IRO2(J),PHY2(J)
LINES = LINES + 1
610 FORMAT(17X,I2,12X,I2,5X,E12.5,3X,E12.5,5X,E12.5,2X,E12.5,9X,E12.5/
1(31X,I2,5X,E12.5,3X,E12.5,40X,E12.5))
612 CONTINUE
1 FORMAT(10I5)
WRITE(6,400)IA,NET,IB
400 FORMAT(1H0,16X,23HNUMBER OF A HARMONICS =I3,9X,28HNUMBER OF SIMPSO
IN STATIONS =I3,8X,23HNUMBER OF B HARMONICS =I3)
IF(RCAP.EQ.0.)GO TO 501
WRITE(6,500)RCAP
500 FORMAT(48X,30HRADIUS OF SPHERICAL SHELL =F8.3)
501 WRITE(6,620)
WRITE(6,621)
WRITE(6,622)
READ(5,1)(IELRES(I),I=1,NEREST)
DO 200 I=1,NEREST
READ(5,1) J,(REST(J,IJ),IJ=1,8)
200 CONTINUE
DO 555 IEL=1,NELEMS
NAMET=NAET(IEL)
NC=NCE(IEL)

```

```
THETA(IEL,NC+1)=360.
IF(IEL.EQ.1)LLINES=54
IIFLAG=0
NCCP=NC
JJFLAG=0
IC=0
717 NCP=NCCP - 4
IF(NCP)701,701,702
702 NCP1=IC*4 + 1
NCP2 = NCP1 + 3
IC = IC + 1
GO TO 703
701 NCP1 = IC*4 + 1
NCP2 = NC
IIFLAG = 1
703 IF(LLINES.LT.54)GO TO 704
LLINES = 0
WRITE(6,604)IPAGE
IPAGE = IPAGE + 1
JJFLAG = 0
WRITE(6,706)
IF(NAMES.NE.SAND)GO TO 7777
WRITE(6,7778)HS
7778 FORMAT(31X,44HSANDWICH TYPE SHELL WITH FACE SHEETS SPACED F7.3,IX,
13HIN ,15HFROM EACH OTHER/)
WRITE(6,7779)
7779 FORMAT(43X,46HTHICKNESSES OF EACH FACE SHEET ARE GIVEN BELOW//)
7777 CONTINUE
LLINES = LLINES + 8
706 FORMAT(56X,20HTABLE OF THICKNESSES/)
704 CONTINUE
IF(NC.NE.1)GO TO 716
WRITE(6,2000)IEL,THICKC(IEL,1)
2000 FORMAT(1H0,32X,12HELEMENT NO. I3,20X,21HCONSTANT THICKNESS =F10.5
I//)
LLINES = LLINES + 4
GO TO 2001
```

```
716 IF(JJFLAG.EQ.0)WRITE(6,710)IEL
710 FORMAT(60X,12HELEMENT NO. I3/)
    LINES = LINES + 2
    IF(NAMET.NE.BLAN)GO TO 2002
    WRITE(6,2003)(THETA(IEL, IY), THETA( IEL, IY+1), IY=NCPI, NCP2)
    WRITE(6,709)(THICKC( IEL, IY), IY=NCPI, NCP2)
    GO TO 2004
2003 FORMAT(1H0,5HTHETA,9H(DEGREES),3X,4(3X,F10.5,4H TO ,F10.5))
2002 WRITE(6,708)(THETA( IEL, IY), IY=NCPI, NCP2)
    WRITE(6,709)(THICKC( IEL, IY), IY=NCPI, NCP2)
708 FORMAT(1H0,5HTHETA,9H(DEGREES),12X,F10.5,3(17X,F10.5))
709 FORMAT(1X,9HTHICKNESS,4(17X,F10.5))
2004 LINES = LINES + 3
    NCCP = NCP
    JJFLAG = 1
    IF(IIIFLAG.EQ.0)GO TO 717
2001 CONTINUE
5555 CONTINUE
    REWIND ISCRAL
    WRITE(6,604)IPAGE
    IPAGE = IPAGE + 1
    WRITE(6,900)
900 FORMAT(56X,20HBOUNDARY CONDITIONS////)
    WRITE(6,901)
901 FORMAT(50X,32HDISPLACEMENT COMPONENTS OF NODES////)
    WRITE(6,902)
902 FORMAT(23X,8HNODE NO.,10X,5HAXIAL,12X,10HTANGENTIAL,11X,6HRADIAL,
112X,7HANGULAR////)
    DO 903 IRE=1,NELEMS
    JISE = 0
    DO 904 ISE=1,4
    RK(ISE)= 0
    IF(REST(IRE, ISE).EQ.0)GO TO 904
    RK(ISE)=1
    JISE=1
904 CONTINUE
    IF(JISE.EQ.0)GO TO 903
```

```
DO 905 ISE=1,4
RRRE(ISE)=RREST2
IF(RK(ISE).EQ.1.)RRRE(ISE)=RREST1
905 CONTINUE
WRITE(6,906)IRE,(RRRE(ISE),ISE=1,4)
906 FORMAT(26X,I2,13X,1HF,A4,15X,1HF,A4,14X,1HF,A4,13X,1HF,A4/)
903 CONTINUE
JISE = 0
DO 907 ISE=1,4
RK(ISE)=0.
ISEE=ISE + 4
IF(REST(NELEMS,ISEE).EQ.0)GO TO 907
RK(ISE)=1.
JISE = 1
907 CONTINUE
IF(JISE.EQ.0)GO TO 908
DO 909 ISE=1,4
RRRE(ISE)=RREST2
IF(RK(ISE).EQ.1.)RRRE(ISE)=RREST1
909 CONTINUE
INEL = NELEMS + 1
WRITE(6,906)INEL,(RRRE(ISE),ISE=1,4)
908 CONTINUE
WRITE(6,910)
910 FORMAT(1H0,47X,36HALL OTHER NODES ARE FREE TO DISPLACE//)
WRITE(6,620)
WRITE(6,621)
WRITE(6,622)
RETURN
END
SUBROUTINE WEH2(P1,P2,J,TGR)
COMMON /A7/ NTAPE,ISCRA1,ISCRA3,NELEMS,NH,IPAGE,PROBLE
DIMENSION P1(50),P2(50)
READ (5,21) NPARTS
READ (5,21) NE1,T1,NCYLIN
21 FORMAT (I5,F10.0,I5)
NP=1
```



```
XE1=NE1
H=T1/XE1
P1(I)=90.0
P2(I)=90.0-H
DO 31 I=2,NE1
  XI=I
  P1(I)=P2(I-1)
  P2(I)=90.0-H*XI
31 IF (NPARTS-1) 35,35,32
32 READ (5,21) NE2,T2,NCYLIN
  IF (NCYLIN) 2,1,2
  1 NP=NP+1
  XE2=NE2
  H=T2/XE2
  DO 33 I=1,NE2
    XI=I
    J=NE1+I
    P1(J)=P2(J-1)
    P2(J)=P2(NE1)-H*XI
    IF (ABS(P2(J)-TCR)-0.005) 37,37,33
37 P2(J)=TCR
33 CONTINUE
  IF (NP-NPARTS) 34,35,35
34 NE1=NE1+NE2
  GO TO 32
  2 J=NELEMS-NE2+1
  P2(J-1)=T2
  DO 101 K=J,NELEMS
    P1(K)=0.0
101 P2(K)=0.0
  GO TO 36
35 J=NELEMS+2
36 RETURN
END
SUBROUTINE WEH3(I,N,C,R,P,J,NNODES,RCAP,RFILET,RMAX,ZMAX)
  DIMENSION P(50)
  TCR=ARCOS((RMAX-RFILET)/(RCAP-RFILET))
```

```
N=I
IF (I-1) 1,1,2
1 C=0.0
R=1.E-6
K=J
CKEEP=RCAP-(RCAP-RFILET)*SIN(TCR)
GO TO 11
2 IF (I-J) 3,4,4
3 T=P(I-1)/57.2957795
IF (T-TCR) 6,5,5
5 R=RCAP*COS(T)
C=RCAP-RCAP*SIN(T)
GO TO 11
6 R=(RMAX-RFILET)+RFILET*COS(T)
C=RCAP-(RCAP-RFILET)*SIN(TCR)-RFILET*SIN(T)
GO TO 11
4 R=RMAX
IF (I-K) 7,7,9
7 IF (I-NNODES) 8,9,9
8 READ (5,21) NE1,S1
21 FORMAT (I5,F10.0)
XE1=NE1
H=S1/XE1
9 XI=I-K
C=CKEEP+H*XI
IF (I-K-NE1+1) 11,10,10
10 CKEEP=CKEEP+S1
K=I+1
11 RETURN
END
SUBROUTINE STIFOR
COMMON /A1/ G(30),GA(30),P(75),R(75),S(75),O(75)
COMMON /A2/ ONE(48),TWO(48)
COMMON /A3/ NDP,NET,IXX(12)
COMMON /A5/ EM(50),POISSO(50)
COMMON /A7/ NTAPE,ISCRAI,ISCRAB,NELEMS,NH,IPAGE,PROBLE
COMMON /B1/ AL(159)
```

```
COMMON /B2/ AR (30), AS(30), PHP(30), SINE(30), COSINE(30)
COMMON /B3/ PHPP
COMMON /B4/ F(8,8)
COMMON /B5/ ISIMP
COMMON /B6/ AK(8,8)
COMMON /B7/ PHIMAT(8,8)
COMMON /B8/ CHECK(8,8)
COMMON /B9/ IA, IB
COMMON /B14/ XTHETA(10), NUNKTH
COMMON /B15/ IELL, NDPP, IXX(12), LINEL, ICON, IC00, NSTOP
COMMON /B18/ NAMES, HS
COMMON /B19/ IELRES(10), REST(50,8), NEREST
COMMON /B24/ ROI(50), PHY1(50), PHY2(50), Z1(50), Z2(50)
COMMON /B30/ NA, NB
COMMON /C1/ NDELQ
COMMON /C40/ IPRI
COMMON /D10/ ISAVE, IDISPL
COMMON /D40/ PHPRMI(50), RMDI(50), CPDM(50), SPDM(50), PPPH(50),
1ARLC(50), CHEKC(50,8,8), R02(50), A1L(50), A2L(50), A3L(50), A157(50),
2A4L(50), A5L(50), A6L(50), A7L(50), A15L(50), A16L(50), A17L(50), A18L(50),
3), A19L(50), A20L(50), A21L(50), A22L(50), A158(50), A159(50)
COMMON /G1/ APPO, THELO, RC, RL, PR
COMMON /G2/ NCE(50), NAET(50), THETA(50,51), THICKC(50,51)
COMMON /G4/ ONEDD(51,48)
COMMON /G9/ TTA(50,50)
DOUBLE PRECISION FKK(4656), AAK(8,8)
DIMENSION RK(4), RRRRE(4)
DIMENSION TA(2,50)
INTEGER PROBLE, RERUN, RRUN, APOL, APPO
INTEGER REST
INTEGER END
INTEGER SAND
DATA SAND/4HSAND/
DATA END/3HEND/
DATA RERUN/4HRRUN/
XNDELQ=NDELQ
IPRI=0
```

```

ISIMP = 0
C EM= MODULUS OF ELASTICITY OF SHELL ELEMENT
C POISSO = POISSON'S RATIO OF SHELL ELEMENT
C ISOTROPIC SHELL ELEMENT ONLY
C IELRES IDENTIFIES WHICH NODE IS RESTRAINED
C REST NOT EQUAL TO ZERO RESTRAINS NODE DISPLACEMENT COMPONENT
C IA = NO. OF A HARMONICS
C IB = NO. OF B HARMONICS
C NTAPE = PIN TAPE WITH STIFFNESS MATRIX
C NTAPEF = BIN TAPE WITH FORCE MATRIX
C ISCRAL, ISCR2=BIN SCRATCH TAPES
C NELEMS = NO. OF ELEMENTS
C NEREST = NO. OF RESTRAINED NODES
C TOTAL NUMBER OF A AND B HARMONICS - NH
N = 4*NH
DO 40 I=1,N
ONE(I) = 0.0
TWO(I) = 0.0
40 CONTINUE
LINEL = 0
ICOO=1
IF(PROBLE.EQ.0)RERUN)GO TO 613
REWIND NTAPE
C NUMBER OF ELEMENTS IN EACH TRIANGLE OF FKK - NN
IF(ISAVE.EQ.0)WRITE(ISCRA3)NEREST,(IELRES(I),I=1,NEREST),
1((REST(J,K),K=1,8),J=1,NELEMS),(EM(IS),POISSO(IS),IS=1,NELEMS)
NN = N*(N+1)/2
C INDEX OF FIRST ELEMENT OF FKK SQUARE - N1
N1 = 2*NN + 1
C INDEX OF LAST ELEMENT OF FKK SQUARE - N2
N2 = 2*NN + N*N
C INITIALIZE UPPER TRIANGLE OF FKK
DO 10 I=1,NN
FKK(I) = 0.0
10 CONTINUE
DO 34 IEL=1,NELEMS
OBTAIN AR, AS, PHP, SINE, PHPP, AND COSINE FOR EACH ELEMENT

```

```
CALL ELEMCA (R01( IEL), R02( IEL), PH1( IEL), PHY2( IEL), Z1, IEL),  
I22( IEL), DS, IEL)  
C CALL CRETAL TO OBTAIN AL ARRAY  
CALL CRETAL( DS, IEL)  
NC=NCE( IEL)  
NAMET=NAET( IEL)  
IF( ISAVE.EQ.0) WRITE( ISCRAB) NCE( IEL), NAET( IEL)  
DO 300 I=1, NC  
300 THETA( IEL, I)=THETA( IEL, I)/57.2957795  
IF( ISAVE.EQ.0) WRITE( ISCRAB) ( THETA( IEL, I), THICKC( IEL, I), I=1, NC)  
THETA( IEL, NC+1)=6.28318531  
THICKC( IEL, NC+1)=THICKC( IEL, 1)  
PI=3.14159265  
IFO=NA+NB+1  
DO 731 I1=1, IFO  
TA( 1, I1)=0.  
731 TA( 2, I1)=0.  
IEXPT=1  
DO 701 I2=1, 2  
SUM1=0.  
DO 700 I1=1, NC  
700 SUM1=SUM1+THICKC( IEL, I1)**IEXPT*( THETA( IEL, I1+1)--THETA( IEL, I1))  
TA( I2, 1)=SUM1/( 2.*PI)  
IF( NC.EQ.1) GO TO 730  
IF( NA.EQ.0) GO TO 740  
DO 702 I1=1, NA  
SUM2=0.  
HII=I1  
DO 703 I3=1, NC  
X=THETA( IEL, I3)  
XX=THETA( IEL, I3+1)  
703 SUM2=SUM2+THICKC( IEL, I3)**IEXPT*( SIN( HII*XX)--SIN( HII*X))  
702 TA( I2, I1+1)=SUM2/( HII*PI)  
740 IF( NB.EQ.0) GO TO 730  
DO 704 I1=1, NB  
SUM3=0.  
HII=I1
```

```

DO 705 I3=1,NC
X=THETA(IEL,I3)
XX=THETA(IEL,I3+1)
705 SUM3=SUM3+THICKC(IEL,I3)**IEXPT*(COS(HI I*XX)-COS(HI I*X))
I4=NA+I1+1
704 TA(I2,I4)=-SUM3/(HII*PI)
730 IF(NAMES.EQ.SAND)GO TO 706
IEXPT=3
701 CONTINUE
706 IF(NAMES.NE.SAND)GO TO 715
DO 717 I1=1,IFO
717 TA(2,I1)=TA(1,I1)
715 CONTINUE
DO 32 I=1,NH
DO 32 J=I,NH

```

C FOLLOWING DETERMINES WHICH VALUES FOR AM AND AN ARE TO BE USED

IF (I.GT. IA.OR. J.GT. IA) GO TO 12

```

M1= I - 1
N3= J - 1
MI=M1+1
NI=N3+1
KEY1=0
GO TO 16
12 IF (I.LE. IA) GO TO 14
M1= - (I - IA - 1)
N3= - (J - IA - 1)
MI=-M1+1
NI=-N3+1
KEY1=2
GO TO 16
14 M1= I - 1
N3= - (J - IA - 1)
MI=M1+1
NI=-N3+1
KEY1=1
16 HM = M1
HN = N3

```

C CALL SUBROUTINE TO GET FL - AN 8X8 MATRIX

```
SIMP1=0.
SIMP2=0.
SIMP3=0.
SIMP4=0.
IF((MI-1).EQ.(NI-1).AND.(MI-1).EQ.0.AND.KEY1.NE.1)GO TO 742
IF((MI-1).EQ.(NI-1).AND.(MI-1).NE.0.AND.KEY1.NE.1)GO TO 744
GO TO 741
742 IF(KEY1.EQ.0)GO TO 760
SIMP3=TA(1,1)*2.0*PI
SIMP4=TA(2,1)*2.0*PI
GO TO 741
760 SIMP1=TA(1,1)*2.0*PI
SIMP2=TA(2,1)*2.0*PI
GO TO 741
744 SIMP1=TA(1,1)*PI
SIMP2=TA(2,1)*PI
SIMP3=SIMP1
SIMP4=SIMP2
741 IF(NC.EQ.1)GO TO 311
I2=NA
I3=1
IF(I2.NE.0)GO TO 747
I3=2
I2=NB
747 DO 745 I1=1,I2
IF((NI+MI).NE.(I1+2).AND.IABS(NI-MI).NE.1)GO TO 745
IF(KEY1.NE.1)GO TO 750
CALL INTE(MI,I1+1,NI,I3,SIM1)
CALL INTE(I1+1,NI,MI,I3,SIM2)
IF(I3.EQ.1)GO TO 751
ASIMI=SIMI
SIM1=SIM2
SIM2=ASIMI
GO TO 751
750 CALL INTE(MI,NI,I1+1,KEY1+I3-1,SIM1)
CALL INTE(MI,NI,I1+1,I-KEY1+I3,SIM2)
```

```
751 I11=NA*(I3-1)+I1+1
    SIMP1=SIMP1+TA(1,I11)*SIMP1
    SIMP2=SIMP2+TA(2,I11)*SIMP1
    SIMP3=SIMP3+TA(1,I11)*SIMP2
    SIMP4=SIMP4+TA(2,I11)*SIMP2
745 CONTINUE
    IF(I3.EQ.2)GO TO 311
    IF(NB.EQ.0)GO TO 311
    I3=2
    I2=NB
    GO TO 747
311 IF(NAMES.NE.SAND)GO TO 314
    SIMP1=2.*SIMP1
    SIMP3=2.*SIMP3
    SIMP2=SIMP2*6.0*HS**2
    SIMP4=SIMP4*6.0*HS**2
314 EI=EM(IEL)
    FNUI=POISSO(IEL)
    E2=E1
    GXX=E1/(2.*(1.+FNUI))
    CONST1=E1/(1.-FNUI*FNUI)
    CCI=CONST1*SIMP1
    CC=2.*CONST1*FNUI*SIMP1
    DD1=CONST1*SIMP2/12.
    DD=CONST1*FNUI*SIMP2/6.
    GGI=GXX*SIMP3
    GG2=GXX*SIMP4/12.0
    CALL CREATL(HM,HN,CCI,CC,DD1,DD,GG1,GG2)
    CALL SUBROUTINE TO PRE AND POST MULTIPLY FL TO GET AK
C
    CALL MMPLT3
C*****INTRODUCE BOUNDARY CONDITIONS
DO 110 IR=1,NEREST
IF(IELRES(IR).NE. IEL)GO TO 110
DO 101 JR=1,8
IF(REST( IEL,JR).EQ.0)GO TO 101
DO 102 JRR=1,8
AK(JR,JRR)=0.
```



```
AK(JRR, JR) = 0.
102 CONTINUE
IF (1.EQ. J) AK(JR, JR) = 1.
101 CONTINUE
110 CONTINUE
DO 481 L=1, 8
DO 482 M=1, 8
AAK(L, M) = AK(L, M)
482 CONTINUE
481 CONTINUE
C THE FOLLOWING TAKES EACH ELEMENT OF AK AND PUTS IT IN
C THE PROPER LOCATION OF FKK
DO 30 L=1, 8
DO 30 M=1, 8
IF (I.NE. J) GO TO 18
IF (L.GT. M) GO TO 30
18 IF (L.GT. 4. OR. M.GT. 4) GO TO 20
ELEMENT IS IN UPPER LEFT 4X4 OF AK MATRIX.
THUS, STORE IN 1ST TRIANGLE OF FKK(I1), I1=1, NN
INDEX = 4*(I-1) + L
JNDEX = 4*(J-1) + M
C SINGLE INDEX OF FKK - KNDEX
KNDEX = JNDEX*{(JNDEX-1)/2 + INDEX
C ADD 2CD TRIANGLE OF PREVIOUS ELEMENT TO FIRST OF THIS ELEMENT
FKK(KNDEX) = FKK(KNDEX) + AAK(L, M)
GO TO 30
20 IF (L.LE. 4. OR. M.LE. 4) GO TO 22
ELEMENT IS IN LOWER RIGHT 4X4 OF AK MATRIX
THUS, STORE IN 2CD TRIANGLE OF FKK(I1), I1=NN+1, 2*NN
INDEX = 4*(I-2) + L
JNDEX = 4*(J-2) + M
C SINGLE INDEX OF FKK - KNDEX
KNDEX = NN + JNDEX*{(JNDEX-1)/2 + INDEX
GO TO 28
22 IF (M.GT. L) GO TO 24
ELEMENT IS IN LOWER LEFT 4X4 OF AK MATRIX.
IF ALONG I=J DIAGONAL, DO NOT NEED
```

```
C      OTHERWISE, TAKE TRANSPOSE AND STORE IN SQUARE OF
C      FKK(I1), I1=2*NN+1,2*NN+N*N
      INDEX = 4*(J-1) + M
      JINDEX = 4*(I-2) + L
      GO TO 26
      24 INDEX = 4*(I-1) + L
      JINDEX = 4*(J-2) + M
C      SINGLE INDEX OF FKK - KINDEX
      26 KINDEX = 2*NN + (JINDEX-1)*N + INDEX
      28 FKK(KINDEX)=AAK(L,M)
      30 CONTINUE
      32 CONTINUE
      DO 903 I=1,N
      I1=(I+I)/2
      IF(FKK(I1).EQ.0.0)FKK(I1)=1.0
      903 CONTINUE
C      WRITE 1ST TRIANGLE OF FKK ON TAPE AS A RECORD
      WRITE (NTAPE) (FKK(I),I=1,NN)
C      WRITE SQUARE OF FKK ON TAPE AS A RECORD
      WRITE (NTAPE) (FKK(I),I=NI,N2)
C      STORE 2CD TRIANGLE IN FIRST
      DO 33 I=1,NN
      I1=I+NN
      FKK(I) = FKK(I1)
      33 CONTINUE
C      WRITE 2CD TRIANGLE ON TAPE ONLY IF LAST ELEMENT
      IF(IEL.NE.NELEMS)GO TO 906
      DO 907 I=1,N
      I1=(I+I)/2
      IF(FKK(I1).EQ.0.0)FKK(I1)=1.0
      907 CONTINUE
      WRITE(NTAPE)(FKK(I),I=1,NN)
C      CALL SUBROUTINE TO CREATE RIGHT HAND SIDE
      906 CALL FORCES(IEL)
      IF(ISAVE.EQ.0)WRITE(ISCRA3)(TA(1,K),K=1,IFO)
      DO 492 K=1,IFO
      492 TTA(IEL,K)=TA(J,K)
```

```
34 CONTINUE
   IF(PROBLE.NE.RERUN)GO TO 612
613 REWIND IS CRA3
   READ(ISCRA3)NEREST,(IELRES(I),I=1,NEREST),(REST(J,K),K=1,8),
   1J=1,NELEMS),(EM(IS),POISSO(IS),IS=1,NELEMS)
   DO 611 K=1,NELEMS
   READ(ISCRA3)PHPRIM,RMID,CPMD,SPMD,PHPP,ARCL,((CHECK(I,J),
   1I=1,8),J=1,8),RO2(K)
   PHPRMI(K)=PHPRIM
   RMDI(K)=RMID
   CPDM(K)=CPMD
   SPDM(K)=SPMD
   PPPH(K)=PHPP
   ARLC(K)=ARCL
   DO 4937 IST1=1,8
   DO 4938 IST2=1,8
   CHEKC(K,IST1,IST2)=CHECK(IST1,IST2)
4938 CONTINUE
4937 CONTINUE
   READ(ISCRA3)A1L(K),A2L(K),A3L(K),A4L(K),A5L(K),A6L(K),A7L(K),
   1A15L(K),A16L(K),A17L(K),A18L(K),A19L(K),A20L(K),A21L(K),A22L(K),
   2A157(K),A158(K),A159(K)
   READ(ISCRA3)NCE(K),NAET(K)
   NC=NCE(K)
   NAMET=NAET(K)
   READ(ISCRA3)(THETA(K,I),THICKC(K,I),I=1,NC)
   THETA(K,NC+1)=6.28318531
   THICKC(K,NC+1)=THICKC(K,1)
   READ(ISCRA3)(TA(1,KJ),KJ=1,IFO)
   DO 491 KJ=1,IFO
491 TTA(K,KJ)=TA(1,KJ)
   CALL FORCES(K)
611 CONTINUE
612 DO 410 IR=1,NEREST
   IF(IELRES(IR).NE.NELEMS)GO TO 410
   DO 401 JR=5,9
   IF(REST(NELEMS, JR).EQ.0)GO TO 401
```

```

DO 402 JJR=1, NH
KR=JR+(JJR-1)*4-4
TWO(KR)=0.
402 CONTINUE
401 CONTINUE
410 CONTINUE
NNE=NELEMS + 1
DO 480 KK=1, N
480 ONEDD(NNE, KK)=TWO(KK)/XNDELQ
C THE TAPE CREATED IN THIS PROGRAM IN USED FOR INPUT
C FOR THE SOLUTION PROGRAM
RETURN
END
SUBROUTINE ELEMCA(R1, R2, P1, P2, Z1, Z2, DSL, IEL)
COMMON /A3/ NDP, NET, IXX(12)
COMMON /A7/ NTAPE, ISCRA1, ISCRA3, NELEMS, NH, IPAGE, PROBLE
COMMON /B2/ AR (30), AS(30), PHP(30), SINE(30), COSINE(30)
COMMON /B3/ PHPP
COMMON /B7/ PHIMAT (8,8)
COMMON /B8/ CHECK(8,8)
COMMON /D10/ ISAVE, IDISPL
COMMON /D40/ PHPRMI(50), RMDI(50), CPDM(50), SPDM(50), PPPH(50),
IARLC(50), CHEKC(50,8,8), R02(50), A1L(50), A2L(50), A3L(50), A157(50),
2A4L(50), A5L(50), A6L(50), A7L(50), A15L(50), A16L(50), A17L(50), A18L(50),
3), A19L(50), A20L(50), A21L(50), A22L(50), A158(50), A159(50)
DIMENSION PH(30), AM(8,8)
DELZ = ABS (Z2-Z1)
DELR=R2-R1
AL=SQRT(DELR**2+DELZ**2)
DELP=(P1-P2)/.572957795E+02
P1=P1/.572957795E+02
P2=P2/.572957795E+02
IF(DELP) 14, 15, 14
15 ARCL=AL
GO TO 10
14 ARD=AL/(2.0*SIN(DELP/2.0))
ARCL=ARO*DELP
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0690
0700
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C      COMPUTE TRANSFORMATION MATRIX
10    DO 102 I=1,8
      DO 102 J=1,8
102   PHIMAT(I,J)=0.0
      DO 105 I=1,3,2
105   PHIMAT(I,I)=COS(P1)
      PHIMAT(I+1,I+1)=1.0
      PHIMAT(3,1)=SIN(P1)
      PHIMAT(1,3)=-PHIMAT(3,1)
      DO 106 I=5,7,2
106   PHIMAT(I,I)=COS(P2)
      PHIMAT(I+1,I+1)=1.0
      PHIMAT(7,5)=SIN(P2)
      PHIMAT(5,7)=-PHIMAT(7,5)
C      COMPUTE M MATRIX
101   DO 101 I=1,8
      DO 101 J=1,8
101   AM(I,J)=0.0
      AM(1,3)=1.0
      AM(2,4)=1.0
      AM(5,1)=1.0
      AM(7,2)=1.0
      AL1=1.0/ARCL
      AL2=AL1*2
      AL3=AL2*AL1
      AM(6,1)=-AL1
      AM(8,2)=-AL1
      AM(3,3)=-3.0*AL2
      AM(4,3)=2.0*AL3
      AM(3,4)=-2.0*AL1
      AM(4,4)=AL2
      AM(6,5)=AL1
      AM(8,6)=AL1
      AM(3,7)=3.0*AL2
      AM(4,7)=-2.0*AL3
      AM(3,8)=-AL1
      AM(4,8)=AL2
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```

```
C      COMPUTE ELEMENTS OF L MATRIX
      IF(DELP)8,9,8
      DO 7 I=1,NET
      PH(I)=PI
      PHP(I)=0.0
      COSINE(I)=COS(PI)
      SINE(I)=SIN(PI)
      CPMD=COSINE(I)
      SPMD=SINE(I)
      RMID=R2
      AS(1)=0.0
      IF(R1)24,40,24
      40 R1 = .1E-06
      24 CONTINUE
      ARCL=AL
      NET1=NET-1
      DP=AL/FLOAT(NET1)
      DSL=DP
      DO 52 I=2,NET
      J=I-1
      52 AS(I)=FLOAT(J)*DP
      AR(I)=R1
      DO 6 I=2,NET
      6 AR(I)=R1+AS(I)*SINE(I)
      PHPRIM=0.0
      PHPP=0.0
      GO TO 19
      8 CONTINUE
      28 IF(R1)29,30,29
      30 R1 = .1E-06
      29 CONTINUE
      NET1=NET-1
      RZ = (R2-R1)/(Z2-Z1)
      PHL=ATAN(RZ)
      A2=(6.0*PHL-4.0*PI-2.0*P2)/ARCL
      A3=(3.0*PI+3.0*P2-6.0*PHL)/ARCL**2
      AM(4,1) = -A2/ARCL**2
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AM(4,5) = - (A2+2.*A3*ARCL)/ARCL**2
AM(2,1) = -A2
AM(3,1) = 2.*A2/ARCL
AM(3,5) = (A2+2.*A3*ARCL)/ARCL
DSL=ARCL/FLOAT(NET1)
42 AS(1)=0.0
43 PH(1)=PI
SINE(1)=SIN(PI)
COSINE(1)=COS(PI)
PHP(1)=A2
NETP=(NET+1)/2
DO 25 I=2,NET
AS(I)=AS(I-1)+DSL
PH(I)=PI+A2*AS(I)+A3*AS(I)**2
PHP(I)=A2+2.0*A3*AS(I)
SINE(I)=SIN(PH(I))
COSINE(I)=COS(PH(I))
IF(I.NE.NETP)GO TO 25
SPMD = SINE(I)
CPMD = COSINE(I)
25 CONTINUE
AR(1)=R1
NETL=NET1/2
PHPRIM=PHP(NETP)
PHPP=2.0*A3
DO 22 I=2,NETP
AR(I)=AR(I-1)+((SINE(I)+SINE(I-1))*DSL/2.0
22 RMID = AR(NETP)
AR(NET)=R2
DO 53 I=1,NETL
NN=NET-I
53 AR(NN)=AR(NN+1)-((SINE(NN+1)+SINE(NN))*DSL/2.0
19 DO 1000 I=1,8
DO 1000 J=1,8
CHECK(I,J) = 0.0
DO 1000 K=1,8
CHECK(I,J) = AM(I,K)*PHIMAT(J,K) + CHECK(I,J)
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1000 CONTINUE
K=IEL
PHPRMI(K)=PHPRIM
RMDI(K)=RMID
CPDM(K)=CPMD
SPDM(K)=SPMD
PPPH(K)=PHPP
ARLC(K)=ARCL
DO 4937 IST1=1,8
DO 4938 IST2=1,8
CHECKC(K,IST1,IST2)=CHECK(IST1,IST2)
4938 CONTINUE
4937 CONTINUE
IF(ISAVE.EQ.0)WRITE(ISCRA3)PHPRIM,RMID,CPMD,SPMD,PHPP,ARCL,
1((CHECK(I,J),I=1,8),J=1,8),R2
RETURN
END
SUBROUTINE CRETAL(DS,IEL)
COMMON /A1/ G(30),GA(30),P(75),R(75),S(75),O(75)
COMMON /A3/ NDP,NET,IXX(12)
COMMON /A7/ NTAPE,ISCRA1,ISCRA3,NELEMS,WH,IPAGE,PROBLE
COMMON /B1/ AL(159)
COMMON /B2/ AR(30),AS(30),PHP(30),SINE(30),COSINE(30)
COMMON /B5/ ISIMP
COMMON /D10/ ISAVE, IDISPL
COMMON /D40/ PHPRMI(50),RMDI(50),CPDM(50),SPDM(50),PPPH(50),
1ARLC(50),CHECKC(50,8,8),R02(50),AIL(50),A2L(50),A3L(50),AL157(50),
2A4L(50),A5L(50),A6L(50),A7L(50),A15L(50),A16L(50),A17L(50),A18L(50),
3A19L(50),A20L(50),A21L(50),A22L(50),AL158(50),AL159(50)
INTEGRATION OF L(I,J)'S
DO 1 I=1,NET
1 GA(I)=AR(I)
ISIMP = 1
AL(1) = SIMP (DS)
DO 2 I=1,NET
2 G(I)=GA(I)+AS(I)
AL(2) = SIMP (DS)

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DO 3 I=1,NET
  3 G(I)=G(I)*AS(I)
  AL(3) = SIMP (DS)
  DO 4 I=1,NET
    4 GA(I)=GA(I)*PHP(I)
    ISIMP = I
    AL(4) = SIMP (DS)
    DO 5 I=1,NET
      5 G(I)=GA(I)*AS(I)
      AL(5) = SIMP (DS)
      DO 6 I=1,NET
        6 G(I)=G(I)*AS(I)
        AL(6) = SIMP (DS)
        DO 7 I=1,NET
          7 G(I)=G(I)*AS(I)
          AL(7) = SIMP (DS)
          DO 8 I=1,NET
            8 GA(I)=GA(I)*PHP(I)
            ISIMP = I
            AL(8) = SIMP (DS)
            DO 9 I=1,NET
              9 G(I)=GA(I)*AS(I)
              AL(9) = SIMP (DS)
              DO 10 I=1,NET
                10 G(I)=G(I)*AS(I)
                AL(10) = SIMP (DS)
                DO 11 I=1,NET
                  11 G(I)=G(I)*AS(I)
                  AL(11) = SIMP (DS)
                  DO 12 I=1,NET
                    12 G(I)=G(I)*AS(I)
                    AL(12) = SIMP (DS)
                    DO 13 I=1,NET
                      13 G(I)=G(I)*AS(I)
                      AL(13) = SIMP (DS)
                      DO 14 I=1,NET
                        14 G(I)=G(I)*AS(I)
```

```
AL(14) = SIMP (DS)
DO 15 I=1,NET
15 GA(I)=PHP(I)*PHP(I)*SINE(I)
   ISIMP = 1
AL(42) = SIMP (DS)
DO 16 I=1,NET
16 G(I)=GA(I)*AS(I)
   AL(43) = SIMP (DS)
DO 17 I=1,NET
17 GA(I)=PHP(I)*SINE(I)
   ISIMP = 1
AL(37) = SIMP (DS)
DO 18 I=1,NET
18 G(I)=GA(I)*AS(I)
   AL(38) = SIMP (DS)
DO 19 I=1,NET
19 G(I)=G(I)*AS(I)
   AL(39) = SIMP (DS)
DO 20 I=1,NET
20 G(I)=G(I)*AS(I)
   AL(40) = SIMP (DS)
DO 21 I=1,NET
21 G(I)=G(I)*AS(I)
   AL(41) = SIMP (DS)
DO 22 I=1,NET
22 GA(I)=PHP(I)
   ISIMP = 1
AL(25) = SIMP (DS)
DO 23 I=1,NET
23 G(I)=GA(I)*AS(I)
   AL(26) = SIMP (DS)
DO 24 I=1,NET
24 G(I)=G(I)*AS(I)
   AL(27) = SIMP (DS)
DO 25 I=1,NET
25 G(I)=G(I)*AS(I)
   AL(28) = SIMP (DS)
```

```
DO 26 I=1,NET
26 G(I)=G(I)*AS(I)
AL(29) = SIMP (DS)
DO 27 I=1,NET
27 G(I)=GA(I)*COSINE(I)
AL(30) = SIMP (DS)
DO 28 I=1,NET
28 G(I)=G(I)*AS(I)
AL(31) = SIMP (DS)
DO 29 I=1,NET
29 G(I)=G(I)*AS(I)
AL(32) = SIMP (DS)
DO 30 I=1,NET
30 G(I)=G(I)*AS(I)
AL(33) = SIMP (DS)
DO 31 I=1,NET
31 G(I)=G(I)*AS(I)
AL(34) = SIMP (DS)
DO 32 I=1,NET
32 G(I) = G(I) * AS(I)
AL(35) = SIMP (DS)
DO 155 I = 1,NET
155 G(I) = G(I) * AS(I)
AL(36) = SIMP (DS)
DO 33 I=1,NET
33 GA(I)=1.
ISIMP = 1
AL(15) = SIMP (DS)
DO 34 I=1,NET
34 G(I)=GA(I)*AS(I)
AL(16) = SIMP (DS)
DO 35 I=1,NET
35 G(I)=GA(I)*COSINE(I)
AL(17) = SIMP (DS)
DO 36 I=1,NET
36 G(I)=G(I)*AS(I)
AL(18) = SIMP (DS)
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```
DO 37 I=1,NET
37 G(I)=G(I)*AS(I)
AL(19) = SIMP (DS)
DO 38 I=1,NET
38 G(I)=G(I)*AS(I)
AL(20) = SIMP (DS)
DO 39 I=1,NET
39 G(I)=GA(I)*SINE(I)
AL(21) = SIMP (DS)
DO 40 I=1,NET
40 G(I)=G(I)*AS(I)
AL(22) = SIMP (DS)
DO 41 I=1,NET
41 G(I)=G(I)*AS(I)
AL(23) = SIMP (DS)
DO 42 I=1,NET
42 G(I)=G(I)*AS(I)
AL(24) = SIMP (DS)
DO 43 I=1,NET
43 GA(I)=1./AR(I)
ISIMP = 1
AL(44) = SIMP (DS)
DO 44 I=1,NET
44 G(I)=GA(I)*AS(I)
AL(45) = SIMP (DS)
DO 45 I=1,NET
45 G(I)=G(I)*AS(I)
AL(46) = SIMP (DS)
DO 46 I=1,NET
46 G(I)=G(I)*AS(I)
AL(47) = SIMP (DS)
DO 47 I=1,NET
47 G(I)=G(I)*AS(I)
AL(48) = SIMP (DS)
DO 48 I=1,NET
48 G(I)=GA(I)*COSINE(I)
AL(49) = SIMP (DS)
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DO 49 I=1,NET
49 G(I)=G(I)*AS(I)
AL(50) = SIMP (DS)
DO 50 I=1,NET
50 G(I)=G(I)*AS(I)
AL(51) = SIMP (DS)
DO 51 I=1,NET
51 G(I)=G(I)*AS(I)
AL(52) = SIMP (DS)
DO 52 I=1,NET
52 G(I)=G(I)*AS(I)
AL(53) = SIMP (DS)
DO 53 I=1,NET
53 G(I)=GA(I)*COSINE(I)*COSINE(I)
AL(54) = SIMP (DS)
DO 54 I=1,NET
54 G(I)=G(I)*AS(I)
AL(55) = SIMP (DS)
DO 55 I=1,NET
55 G(I)=G(I)*AS(I)
AL(56) = SIMP (DS)
DO 56 I=1,NET
56 G(I)=G(I)*AS(I)
AL(57) = SIMP (DS)
DO 57 I=1,NET
57 G(I)=G(I)*AS(I)
AL(58) = SIMP (DS)
DO 58 I=1,NET
58 G(I)=G(I)*AS(I)
AL(59) = SIMP (DS)
DO 59 I=1,NET
59 G(I)=G(I)*AS(I)
AL(60) = SIMP (DS)
DO 60 I=1,NET
60 GA(I)=GA(I)*SINE(I)
ISIMP = 1
AL(61) = SIMP (DS)
```

```
DO 61 I=1,NET
61 G(I)=GA(I)*AS(I)
   AL(62) = SIMP (DS)
DO 62 I=1,NET
62 G(I)=G(I)*AS(I)
   AL(63) = SIMP (DS)
DO 63 I=1,NET
63 G(I)=GA(I)*COSINE(I)
   AL(64) = SIMP (DS)
DO 64 I=1,NET
64 G(I)=G(I)*AS(I)
   AL(65) = SIMP (DS)
DO 65 I=1,NET
65 G(I)=G(I)*AS(I)
   AL(66) = SIMP (DS)
DO 66 I=1,NET
66 G(I)=G(I)*AS(I)
   AL(67) = SIMP (DS)
DO 67 I=1,NET
67 G(I)=G(I)*AS(I)
   AL(68) = SIMP (DS)
DO 68 I=1,NET
68 GA(I)=GA(I)*SINE(I)
   ISIMP = 1
   AL(69) = SIMP (DS)
DO 69 I=1,NET
69 G(I)=GA(I)*AS(I)
   AL(70) = SIMP (DS)
DO 70 I=1,NET
70 G(I)=G(I)*AS(I)
   AL(71) = SIMP (DS)
DO 71 I=1,NET
71 G(I)=G(I)*AS(I)
   AL(72) = SIMP (DS)
DO 72 I=1,NET
72 G(I)=G(I)*AS(I)
   AL(73) = SIMP (DS)
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DO 73 I=1,NET
73 GA(I)=GA(I)*PHP(I)
   ISIMP = 1
   AL(86) = SIMP (DS)
DO 74 I=1,NET
74 G(I) = GA(I)*AS(I)
   AL(87) = SIMP (DS)
DO 75 I=1,NET
75 G(I)=G(I)*AS(I)
   AL(88) = SIMP (DS)
DO 76 I=1,NET
76 G(I)=G(I)*AS(I)
   AL(89) = SIMP (DS)
DO 77 I=1,NET
77 GA(I)=PHP(I)*SINE(I)/AR(I)
   ISIMP = 1
   AL(80) = SIMP (DS)
DO 78 I=1,NET
78 G(I) = GA(I) * AS(I)
   AL(81) = SIMP (DS)
DO 156 I = 1,NET
156 G(I) = G(I) * AS(I)
   AL(82) = SIMP (DS)
DO 79 I=1,NET
79 G(I)=G(I)*AS(I)
   AL(83) = SIMP (DS)
DO 80 I=1,NET
80 G(I)=GA(I)*COSINE(I)
   AL(84) = SIMP (DS)
DO 81 I=1,NET
81 G(I)=G(I)*AS(I)
   AL(85) = SIMP (DS)
DO 82 I=1,NET
82 GA(I)=PHP(I)/AR(I)
   ISIMP = 1
   AL(74) = SIMP (DS)
DO 83 I=1,NET
```

```
83 G(I)=GA(I)*AS(I)
   AL(75) = SIMP (DS)
   DO 84 I=1,NET
84 G(I)=G(I)*AS(I)
   AL(76) = SIMP (DS)
   DO 85 I=1,NET
85 G(I)=G(I)*AS(I)
   AL(77) = SIMP (DS)
   DO 86 I=1,NET
86 G(I)=GA(I)*COSINE(I)
   AL(78) = SIMP (DS)
   DO 87 I=1,NET
87 G(I)=G(I)*AS(I)
   AL(79) = SIMP (DS)
   DO 88 I=1,NET
88 GA(I)=GA(I)*PHP(I)
   ISIMP = 1
   AL(90) = SIMP (DS)
   DO 89 I=1,NET
89 G(I)=GA(I)*AS(I)
   AL(91) = SIMP (DS)
   DO 90 I=1,NET
90 G(I)=G(I)*AS(I)
   AL(92) = SIMP (DS)
   DO 91 I=1,NET
91 G(I)=GA(I)*SINE(I)
   AL(93) = SIMP (DS)
   DO 92 I=1,NET
92 G(I)=G(I)*AS(I)
   AL(94) = SIMP (DS)
   DO 93 I=1,NET
93 G(I)=G(I)*AS(I)
   AL(95) = SIMP (DS)
   DO 94 I=1,NET
94 G(I)=G(I)*SINE(I)
   AL(98) = SIMP (DS)
   DO 95 I=1,NET
```



```
95 G(I)=AS(I)*PHP(I)**2*SINE(I)**2/AR(I)
   AL(97) = SIMP (DS)
   DO 96 I=1,NET
96 G(I)=PHP(I)**2*SINE(I)**2/AR(I)
   AL(96) = SIMP (DS)
   DO 97 I=1,NET
97 GA(I)=PHP(I)*SINE(I)/AR(I)**2
   ISIMP = 1
   AL(111) = SIMP (DS)
   DO 98 I=1,NET
98 G(I)=GA(I)*AS(I)
   AL(112) = SIMP (DS)
   DO 99 I=1,NET
99 G(I)=G(I)*AS(I)
   AL(113) = SIMP (DS)
   DO 100 I=1,NET
100 G(I)=G(I)*AS(I)
   AL(114) = SIMP (DS)
   DO 101 I=1,NET
101 G(I)=G(I)*AS(I)
   AL(115) = SIMP (DS)
   DO 102 I=1,NET
102 G(I)=GA(I)*COSINE(I)
   AL(116) = SIMP (DS)
   DO 103 I=1,NET
103 G(I)=G(I)*AS(I)
   AL(117) = SIMP (DS)
   DO 104 I=1,NET
104 G(I)=G(I)*AS(I)
   AL(118) = SIMP (DS)
   DO 105 I=1,NET
105 G(I)=GA(I)*SINE(I)
   AL(119) = SIMP (DS)
   DO 106 I=1,NET
106 G(I)=G(I)*AS(I)
   AL(120) = SIMP (DS)
   DO 107 I=1,NET
```

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107 G(I)=G(I)*AS(I)
    AL(121) = SIMP (DS)
    DO 108 I=1,NET
108 G(I)=G(I)*AS(I)
    AL(122) = SIMP (DS)
    DO 109 I=1,NET
109 G(I)=G(I)*AS(I)
    AL(123) = SIMP (DS)
    DO 110 I=1,NET
110 G(I)=GA(I)*SINE(I)*COSINE(I)
    AL(124) = SIMP (DS)
    DO 111 I=1,NET
111 G(I)=G(I)*AS(I)
    AL(125) = SIMP (DS)
    DO 112 I=1,NET
112 G(I)=G(I)*AS(I)
    AL(126) = SIMP (DS)
    DO 113 I=1,NET
113 GA(I)=SINE(I)/AR(I)**2
    ISIMP = 1
    AL(99) = SIMP (DS)
    DO 114 I=1,NET
114 G(I)=GA(I)*AS(I)
    AL(100) = SIMP (DS)
    DO 115 I=1,NET
115 G(I)=G(I)*AS(I)
    AL(101) = SIMP (DS)
    DO 116 I=1,NET
116 G(I)=G(I)*AS(I)
    AL(102) = SIMP (DS)
    DO 117 I=1,NET
117 G(I)=G(I)*AS(I)
    AL(103) = SIMP (DS)
    DO 118 I=1,NET
118 G(I)=G(I)*AS(I)
    AL(104) = SIMP (DS)
    DO 119 I=1,NET
```

```
119 G(I)=GA(I)*COSINE(I)
    AL(105) = SIMP (DS)
    DO 120 I=1,NET
120 G(I)=G(I)*AS(I)
    AL(106) = SIMP (DS)
    DO 121 I=1,NET
121 G(I)=G(I)*AS(I)
    AL(107) = SIMP (DS)
    DO 122 I=1,NET
122 G(I)=G(I)*AS(I)
    AL(108) = SIMP (DS)
    DO 123 I=1,NET
123 G(I)=GA(I)*COSINE(I)*COSINE(I)
    AL(109) = SIMP (D.F)
    DO 124 I=1,NET
124 G(I)=G(I)*AS(I)
    AL(110) = SIMP (DS)
    DO 125 I=1,NET
125 GA(I)=SINE(I)**2/AR(I)**3
    ISIMP = 1
    AL(142) = SIMP (DS)
    DO 126 I=1,NET
126 G(I)=GA(I)*AS(I)
    AL(143) = SIMP (DS)
    DO 127 I=1,NET
127 G(I)=G(I)*AS(I)
    AL(144) = SIMP (DS)
    DO 128 I=1,NET
128 G(I)=G(I)*AS(I)
    AL(145) = SIMP (DS)
    DO 129 I=1,NET
129 G(I)=G(I)*AS(I)
    AL(146) = SIMP (DS)
    DO 130 I=1,NET
130 G(I)=G(I)*AS(I)
    AL(147) = SIMP (DS)
    DO 131 I=1,NET
```

```
131 G(I)=G(I)*AS(I)
    AL(148) = SIMP (DS)
    DO 132 I=1,NET
132 GA(I)=GA(I)*COSINE(I)
    ISIMP = 1
    AL(149) = SIMP (DS)
    DO 133 I=1,NET
133 G(I)=GA(I)*AS(I)
    AL(150) = SIMP (DS)
    DO 134 I=1,NET
134 G(I)=G(I)*AS(I)
    AL(151) = SIMP (DS)
    DO 135 I=1,NET
135 G(I)=G(I)*AS(I)
    AL(152) = SIMP (DS)
    DO 136 I=1,NET
136 G(I)=G(I)*AS(I)
    AL(153) = SIMP (DS)
    DO 137 I=1,NET
137 G(I)=GA(I)*COSINE(I)
    AL(154) = SIMP (DS)
    DO 138 I=1,NET
138 G(I)=G(I)*AS(I)
    AL(155) = SIMP (DS)
    DO 139 I=1,NET
139 G(I)=G(I)*AS(I)
    AL(156) = SIMP (DS)
    DO 140 I=1,NET
140 GA(I)=COSINE(I)/AR(I)**3
    ISIMP = 1
    AL(134) = SIMP (DS)
    DO 141 I=1,NET
141 G(I)=GA(I)*AS(I)
    AL(135) = SIMP (DS)
    DO 142 I=1,NET
142 G(I)=G(I)*AS(I)
    AL(136) = SIMP (DS)
```

```
DO 143 I=1,NET
143 G(I)=G(I)*AS(I)
   AL(137) = SIMP (DS)
DO 144 I=1,NET
144 G(I)=G(I)*AS(-I)
   AL(138) = SIMP (DS)
DO 145 I=1,NET
145 G(I)=GA(I)*COSINE(I)
   AL(139) = SIMP (DS)
DO 146 I=1,NET
146 G(I)=G(I)*AS(I)
   AL(140) = SIMP (DS)
DO 147 I=1,NET
147 G(I)=G(I)*AS(I)
   AL(141) = SIMP (DS)
DO 148 I=1,NET
148 G(I)=1./AR(I)**3
   AL(127) = SIMP (DS)
DO 149 I=1,NET
149 G(I)=G(I)*AS(I)
   AL(128) = SIMP (DS)
DO 150 I=1,NET
150 G(I)=G(I)*AS(I)
   AL(129) = SIMP (DS)
DO 151 I=1,NET
151 G(I)=G(I)*AS(I)
   AL(130) = SIMP (DS)
DO 152 I=1,NET
152 G(I)=G(I)*AS(I)
   AL(131) = SIMP (DS)
DO 153 I=1,NET
153 G(I)=G(I)*AS(I)
   AL(132) = SIMP (DS)
DO 154 I=1,NET
154 G(I)=G(I)*AS(I)
   AL(133) = SIMP (DS)
DO 157 I=1,NET
```

```
157 G(I) = AR(I)*AS(I)*AS(I)*AS(I)*AS(I)
AL(157) = SIMP (DS)
DO 158 I=1,NET
158 G(I)=AS(I)**2
AL(158) = SIMP (DS)
DO 159 I=1,NET
159 G(I)=G(I)*AS(I)
AL(159) = SIMP (DS)
A1L(1EL)=AL(1)
A2L(1EL)=AL(2)
A3L(1EL)=AL(3)
A4L(1EL)=AL(4)
A5L(1EL)=AL(5)
A6L(1EL)=AL(6)
A7L(1EL)=AL(7)
A15L(1EL)=AL(15)
A16L(1EL)=AL(16)
A17L(1EL)=AL(17)
A18L(1EL)=AL(18)
A19L(1EL)=AL(19)
A20L(1EL)=AL(20)
A21L(1EL)=AL(21)
A22L(1EL)=AL(22)
AL157(1EL)=AL(157)
AL158(1EL)=AL(158)
AL159(1EL)=AL(159)
IF(1SAVE.EQ.0)WRITE(1SCRA3)AL(1),AL(2),AL(3),AL(4),AL(5),AL(6),
1AL(7),AL(15),AL(16),AL(17),AL(18),AL(19),AL(20),AL(21),AL(22),
2AL(157),AL(158),AL(159)
RETURN
END
FUNCTION SIMP(DS)
COMMON /A1/ G(30),GA(30),P(75),R(75),S(75),O(75)
COMMON /A3/ NDP,NET,IXX(12)
COMMON /B5/ISIMP
NET=NET
NET3 = NET
```

0410

```

NET1 = NET3 - 1
NET2 = NET3 - 2
SUM=0.0
IF(ISIMP-1)3,4,3
3 DO 1 I=2,NET1,2
1 SUM = SUM + 4.0*G(I)
DO 2 I=3,NET2,2
2 SUM = SUM + 2.0*G(I)
AG1 = G(I)
AG2 = G(NET3)
GO TO 10
4 DO 11 I=2,NET1,2
11 SUM = SUM + 4.0*GA(I)
DC 22 I=3,NET2,2
22 SUM = SUM + 2.0*GA(I)
AG1 = GA(I)
AG2 = GA(NET3)
10 SIMP = (AG1 + SUM + AG2)*DS/3.0
ISIMP = 0
RETURN
END

```

0470
0480

```

SUBROUTINE INTE(I,J,L,KEY,SIMS)
PI=3.14159265
SIMS=0.
IF(KEY.EQ.0)GO TO 10
IF(KEY.EQ.2)GO TO 20
RETURN
10 K=1
IF(J.EQ.1)K=2
IF(I.EQ.1)K=K+1
IF(L.EQ.1)K=K+1
FK=K
SIMS=FK*PI/2.0
RETURN
20 IF(J.EQ.1.OR.I.EQ.1)RETURN
IF(L.NE.1)GO TO 30
SIMS=PI

```

```
RETURN
30 SIMS=PI/2.0
IF(J+I-L.EQ.1)SIMS=-PI/2.0
RETURN
END
SUBROUTINE CREATL (AM,AN,C1,C2,C,D1,D2,D,G1,G2)
COMMON /B1/ AL(159)
COMMON /B3/ PHPP
COMMON /B4/ F(8,8)
DIMENSION E(8,8)
IFLAG = 0
C THE ABOVE IS A FLAG USED TO DETERMINE WHETHER THE ELEMENTS
C ABOVE OR BELOW THE DIAGONAL ARE BEING CALCULATED
C CALCULATE THE DIAGONAL ELEMENTS OF F
F(1,1) =
A C1*AL(8) + C2*AL(54) - C*AL(30) + 4.*G2*AM*AN*AL(142) + D2*AM*
1 AM*AN*AN*AL(127)
F(2,2) =
A C1*AL(10) + C2*AL(56) - C*AL(32) + D2*AM*AN*AN*AL(129) - D2
1 *(AM*AM+AN*AN)*AL(100) + D2*AL(69) + 4.*G2*AM*AN*AL(144) - 8.*
2 G2*AM*AN*AL(100) + 4.*G2*AM*AN*AL(44)
F(3,3) =
A C1*AL(12) + C2*AL(58) - C*AL(34) + 4.*D1*AL(1) + D2*AM*AN*AN*
1 AN*AL(131) - 2.*D2*(AM*AM+AN*AN)*AL(102) + 4.*D2*AL(71) - D*(
2 AM*AM+AN*AN)*AL(46) + 4.*D*AL(22) + 4.*G2*AM*AN*AL(146) - 16.*
3 G2*AM*AN*AL(102) + 16.*G2*AM*AN*AL(46)
F(4,4) =
A C1*AL(14) + C2*AL(60) - C*AL(36) + 36.*D1*AL(3) + D2*AM*AN*AN*
1 AN*AL(133) - 3.*D2*(AM*AM+AN*AN)*AL(104) - 3.*D*(AM*AM+AN*AN)*
2 AL(48) + 18.*D*AL(24) + 9.*D2*AL(73) + 4.*G2*AM*AN*AL(148) -
3 24.*G2*AM*AN*AL(104) + 36.*G2*AM*AN*AL(48)
F(5,5) =
A C2*AL(69) + G1*AM*AN*AL(44) + D1*PHPP**2 *AL(1) + D2*AL(96) +
1 D*PHPP*AL(37) + G2*AM*AN*AL(90)
F(6,6) =
A C1*AL(1) + C2*AL(71) + C*AL(22) + G1*AM*AN*AL(46) + D1*PHPP**2
1 * AL(3) + 2.*D1*PHPP*AL(5) + D1*AL(8) + D2*AL(98) + D*PHPP*
```



```
2 AL(39) + D*AL(43) + G2*AM*AN*AL(92)
F(7,7) =
A C2*AM*AN*AL(44) + G1*AL(69) + D2*AM*AN*AL(139) + 4.*G2*AL(154)
1 + 4.*G2*AL(124) + G2*AL(96)
F(8,8) =
A C2*AM*AN*AL(46) + G1*AL(71) - 2.*G1*AL(22) + D2*AM*AN*AL(141)
1 + G1*AL(1) + 4.*G2*AL(156) - 4.*G2*AL(110) + 4.*G2*AL(126) +
2 G2*AL(54) - 2.*G2*AL(85) + G2*AL(98)
CALCULATE ELEMENTS ABOVE THE DIAGONAL AND CALL THEM E
C
10 E(1,2) =
A C1*AL(9) + C2*AL(55) - C*AL(31) + D2*AM*AN*AN*AL(128) + 4.*
1 G2*AM*AN*AL(143) - D2*AM*AM*AL(99) - 4.*G2*AM*AN*AL(99)
E(1,3) =
A C1*AL(10) + C2*AL(56) - C*AL(32) + D2*AM*AM*AN*AN*AL(129)
1 + 4.*G2*AM*AN*AL(144) - 2.*D2*AM*AM*AL(100) - D*AM*AM*AL(44) -
2 8.*G2*AM*AN*AL(100)
E(1,4) =
A C1*AL(11) + C2*AL(57) - C*AL(33) + D2*AM*AM*AN*AN*AL(130) - 3.*
1 *D2*AM*AM*AL(101) - 3.*D*AM*AM*AL(45) + 4.*G2*AM*AN*AL(145) -
2 12.*G2*AM*AN*AL(101)
E(1,5) =
A C2*AL(64) - C/2.*AL(37) - D2*AM*AM*AL(111) - D/2.*AM*AM*PHPP*
1 AL(44) - 2.*G2*AM*AN*AL(111)
E(1,6) =
A - C1*AL(4) + C2*AL(65) - C/2.*AL(38) - D2*AM*AM*AL(112) - D/2.*
1 *AM*AM*PHPP*AL(45) - D/2.*AM*AM*AL(74) + C/2.*AL(17) - 2.*G2*
2 AM*AN*AL(112)
E(1,7) =
A C2*AM*AL(49) - C/2.*AN*AL(25) + D2*AM*AM*AN*AL(134) + 4.*G2*AM
1 *AL(149) + 2.*G2*AM*AL(119)
E(1,8) =
A C2*AM*AL(50) - C/2.*AN*AL(26) + D2*AM*AM*AN*AL(135) + 4.*G2*AM
1 *AL(150) - 2.*G2*AM*AL(105) + 2.*G2*AM*AL(120)
E(2,3) =
A C1*AL(11) + C2*AL(57) - C*AL(33) + D2*AM*AM*AN*AN*AL(130) - D2
1 *(2.*AM*AM*AN*AN)*AL(101) + 2.*D2*AL(70) + D*AM*AM*AL(45) + D*
2 AL(21) + 4.*G2*AM*AN*AL(145) - 12.*G2*AM*AN*AL(101) + 8.*G2*AM
```

```

3 *AN*AL(45)
E(2,4) =
A C1*AL(12) + C2*AL(58) - C*AL(34) + D2*AM*AM*AN*AL(131) - D2
1 *(3.*AM*AM+AN*AN)*AL(102) + 3.*D2*AL(71) - 3.*D*AM*AM*AL(46) +
2 3.*D*AL(22) + 4.*G2*AM*AN*AL(146) + 12.*G2*AM*AN*AL(46) - 16.*
3 G2*AM*AN*AL(102)
E(2,5) =
A C2*AL(65) - C/2.*AL(38) - D2*AM*AM*AL(112) + D2*AL(86) - D/2.*
1 AM*AM*PHPP*AL(45) + D/2.*PHPP*AL(21) - 2.*G2*AM*AN*AL(112) +
2 2.*G2*AM*AN*AL(74)
E(2,6) =
A - C1*AL(5) + C2*AL(66) - C/2.*AL(39) + C/2.*AL(18) - D2*AM*AM*
1 AL(113) + D2*AL(87) - D/2.*AM*AM*PHPP*AL(46) + D/2.*PHPP*
2 AL(22) - D/2.*AM*AM*AL(75) + D/2.*AL(37) - 2.*G2*AM*AN*AL(113)
3 + 2.*G2*AM*AN*AL(75)
E(2,7) =
A C2*AN*AL(50) - C/2.*AN*AL(26) + D2*AM*AM*AN*AL(135) - D2*AN*
1 AL(105) + 4.*G2*AM*AL(150) - 4.*G2*AM*AL(105) - 2.*G2*AM*
2 AL(80) + 2.*G2*AM*AL(120)
E(2,8) =
A C2*AN*AL(51) - C/2.*AN*AL(27) + D2*AM*AM*AN*AL(136) - D2*AN*
1 AL(106) + 4.*G2*AM*AL(151) + 2.*G2*AM*AL(121) - 6.*G2*AM*
2 AL(106) + 2.*G2*AM*AL(49) - 2.*G2*AM*AL(81)
E(3,4) =
A C1*AL(13) + C2*AL(59) - C*AL(35) + 12.*D1*AL(2) + D2*AM*AM*AN*
1 AN*AL(132) - D2*(3.*AM*AM+2.*AN*AN)*AL(103) + 6.*D2*AL(72) - D
2 *(AN*AN+3.*AM*AM)*AL(47) + 9.*D*AL(23) + 4.*G2*AM*AN*AL(147) -
3 20.*G2*AM*AN*AL(103) + 24.*G2*AM*AN*AL(47)
E(3,5) =
A C2*AL(66) - C/2.*AL(39) + 2.*D1*PHPP*AL(1) - D2*AM*AM*AL(113)
1 + 2.*D2*AL(87) + D*AL(37) - D/2.*AM*AM*PHPP*AL(46) + D*PHPP*
2 AL(22) - 2.*G2*AM*AN*AL(113) + 4.*G2*AM*AN*AL(75)
E(3,6) =
A - C1*AL(6) + C2*AL(67) - C/2.*AL(40) + 2.*D1*PHPP*AL(2) + 2.*
1 D1*AL(4) - D2*AM*AM*AL(114) + 2.*D2*AL(88) + 2.*D*AL(38) - D/
2 2.*AM*AM*PHPP*AL(47) + D*PHPP*AL(23) - D/2.*AM*AM*AL(76) + C/
3 2.*AL(19) - 2.*G2*AM*AN*AL(114) + 4.*G2*AM*AN*AL(76)

```

E(3,7) =
A C2*AN*AL(51) - C/2.*AN*AL(27) + D2*AM*AM*AN*AL(136) - 2.*D2*AN
1 *AL(106) - D*AN*AL(49) - 4.*G2*AM*AL(81) + 4.*G2*AM*AL(151) +
2 2.*G2*AM*AL(121) - 8.*G2*AM*AL(106)
E(3,8) =
A C2*AN*AL(52) - C/2.*AN*AL(28) + D2*AM*AM*AN*AL(137) - 2.*D2*AN
1 *AL(107) - D*AN*AL(50) + 4.*G2*AM*AL(152) - 10.*G2*AM*AL(107)
2 + 2.*G2*AM*AL(122) + 4.*G2*AM*AL(50) - 4.*G2*AM*AL(82)
E(4,5) =
A C2*AL(67) - C/2.*AL(40) + 6.*D1*PHPP*AL(2) - D2*AM*AM*AL(114)
1 + 3.*D2*AL(88) + 3.*D*AL(38) - D/2.*AM*AM*PHPP*AL(47) + 3./2.*
2 D*PHPP*AL(23) - 2.*G2*AM*AN*AL(114) + 6.*G2*AM*AN*AL(76)
E(4,6) =
A - C1*AL(7) + C2*AL(68) - C/2.*AL(41) + 6.*D1*PHPP*AL(3) + 6.*
1 D1*AL(5) - D2*AM*AM*AL(115) + 3.*D2*AL(89) - D/2.*AM*AM*PHPP*
2 AL(48) + 3./2.*D*PHPP*AL(24) - D/2.*AM*AM*AL(77) + 9./2.*D*
3 AL(39) + C/2.*AL(20) - 2.*G2*AM*AN*AL(115) + 6.*G2*AM*AN*
4 AL(77)
E(4,7) =
A C2*AN*AL(52) - C/2.*AN*AL(28) + D2*AM*AM*AN*AL(137) - 3.*D2*AN
1 *AL(107) - 3.*D*AN*AL(50) + 4.*G2*AM*AL(152) + 2.*G2*AM*
2 AL(122) - 12.*G2*AM*AL(107) - 6.*G2*AM*AL(82)
E(4,8) =
A C2*AN*AL(53) - C/2.*AN*AL(29) + D2*AM*AM*AN*AL(138) - 3.*D2*AN
1 *AL(108) - 3.*D*AN*AL(51) + 4.*G2*AM*AL(153) - 14.*G2*AM*
2 AL(108) + 2.*G2*AM*AL(123) + 6.*G2*AM*AL(51) - 6.*G2*AM*AL(83)
E(5,6) =
A C2*AL(70) + C/2.*AL(21) + G1*AM*AN*AL(45) + D1*PHPP**2 *AL(2)
1 + D1*PHPP*AL(4) + D2*AL(97) + D*PHPP*AL(38) + D/2.*AL(42) + G2
2 *AM*AN*AL(91)
E(5,7) =
A C2*AN*AL(61) + G1*AM*AL(61) - D2*AN*AL(116) - D/2.*AN*PHPP*
1 AL(49) - 2.*G2*AM*AL(116) - G2*AM*AL(93)
E(5,8) =
A C2*AN*AL(62) + G1*AM*AL(62) - G1*AM*AL(15) - D2*AN*AL(117) - D
1 /2.*AN*PHPP*AL(50) - 2.*G2*AM*AL(117) + G2*AM*AL(78) - G2*AM*
2 AL(94)

```
E(6,7) =  
A C2*AN*AL(62) + C/2.*AN*AL(15) + G1*AM*AL(62) -D2*AN*AL(117) -  
ID/2.*AN*PHPP*AL(50)-D/2.*AN*AL(78)-2.*G2*AM*AL(117) - G2*AM*AL(94)  
E(6,8) =  
A C2*AN*AL(63) + C/2.*AN*AL(16) + G1*AM*AL(63) - G1*AM*AL(16) -  
D2*AN*AL(118) - D/2.*AN*PHPP*AL(51) - D/2.*AN*AL(79) - 2.*G2*  
AM*AL(118) + G2*AM*AL(79) - G2*AM*AL(95)  
E(7,8) =  
A C2*AM*AN*AL(45) + G1*AL(70) - G1*AL(21) + D2*AM*AN*AL(140) +  
4.*G2*AL(155) - 2.*G2*AL(109) + 4.*G2*AL(125) - G2*AL(84) + G2  
*AL(97)  
IFLAG = IFLAG + 1  
IF (IFLAG.NE.1) GO TO 20  
C THE ELEMENTS OF F THAT ARE NEEDED ARE THOSE ABOVE THE DIAGONAL  
C THUS F IS IDENTICAL TO E  
DO 15 I=1,7  
II=I+1  
DO 15 J=II,8  
F(I,J) = E(I,J)  
15 CONTINUE  
C INTERCHANGE AM AND AN TO RECALCULATE E FOR THE TERMS OF F  
C BELOW THE DIAGONAL  
TEMP = AM  
AM = AN  
AN = TEMP  
GO TO 10  
C THE F ELEMENTS BELOW THE DIAGONAL ARE SET EQUAL TO E ELEMENTS  
C ABOVE THE DIAGONAL  
20 DO 25 I=2,8  
II = I-1  
DO 25 J=1,II  
F(I,J) = E(J,I)  
25 CONTINUE  
RETURN  
END  
SUBROUTINE MMPLT3  
COMMON /B4/ F(8,8)
```

```
COMMON /B6/ AK(8,8)
COMMON /B8/ CHECK(8,8)
DO 1 I=1,8
DO 1 J=1,8
AK(I,J)=0.0
DO 1 K=1,8
1 AK(I,J)=AK(I,J)+F(I,K)*CHECK(K,J)
DO 2 I=1,8
DO 2 J=1,8
F(I,J)=0.0
DO 2 K=1,8
2 F(I,J)=F(I,J)+CHECK(K,I)*AK(K,J)
DO 3 I=1,8
DO 3 J=1,8
3 AK(I,J)=F(I,J)
RETURN
END
SUBROUTINE FORCES(IEL)
COMMON /A1/ G(30),GA(30), P(75),R(75),S(75),O(75)
COMMON /A2/ ONE(48),TWO(48)
COMMON /A3/ NDF,NET,IXX(12)
COMMON /A7/ NTAPE,ISCR1,ISCR3,NELEMS,NH,IPAGE,PROBLE
COMMON /B5/ISIMP
COMMON /B8/ CHECK(8,8)
COMMON /B9/ IA,IB
COMMON /B15/ IELL,NDPP,IXXX(12),LINEL,ICON,ICOO,NSTOP
COMMON /B19/ IELRES(10),REST(50,8),NEREST
COMMON /B24/ ROI(50),PHY1(50),PHY2(50),ZI(50),Z2(50)
COMMON /C1/ NDELQ
COMMON /C40/ IPRI
COMMON /D10/ ISAVE,IDIJPL
COMMON /D40/ PHPRMI(50),RMDI(50),CPDM(50),SPDM(50),PPPH(50),
IARLC(50),CHEKC(50,8),R02(50),AIL(50),A2L(50),A3L(50),AL157(50),
2A4L(50),A5L(50),A6L(50),A7L(50),A15L(50),A16L(50),A17L(50),A18L(50),
3) ,A19L(50),A20L(50),A21L(50),A22L(50),AL158(50),AL159(50)
COMMON /G1/ APPO,THELO,RC,RL,PR
COMMON /G4/ ONEDD(51,48)
```

```
DIMENSION      Q(8),QQ(8)
INTEGER PROBLE,RERUN,RRUN,APOL,APPO
INTEGER CONS
INTEGER CONST
INTEGER REST
DATA CONST/4HCONS/
DATA APOL/4HAPOL/
XNDELQ=NDELQ
IF(APPO.NE.APOL)GO TO 770
I=IEL
DO 780 J=1,NH
780  IXX(J)=0
      THELO1=THELO/57.2957795
      RA=RC*COS(THELO1)
      RB=RC*SIN(THELO1)
      RLMRC = RL - RC
      DU 703 J=1,3
      P(J)=0.
      R(J)=0.
      S(J)=0.
      O(J)=0.
703  CONTINUE
      RFF=R02(I)
      IF(RFF.GT.(RC+RL))GO TO 403
      IF(RFF.LE.(RC-RL))GO TO 403
      IF(I.NE.1)GO TO 760
      RF=R02(1)/2.
      GO TO 777
760  RF=RFF-(R02(I)-R02(I-1))/2.
777  IF(RLMRC.LT.0.)GO TO 761
      IF(RF.GE.RLMRC)GO TO 761
      R(1) = PR
      NDP = 1
      GO TO 500
761  RR=RA*RA+RF*RF+RB*RB-RL*RL
      RRR=4.*RB*RB*RF*RF-RR*RR
      RRA=4.*RA*RA+4.*RB*RB
```

```
RRB=4.*RR*RA
X3=SQRT(ABS(RRB*RRB+4.*RRA*RRR))
X1=(RRB+X3)/(2.*RRA)
X2=(RRB-X3)/(2.*RRA)
IF(THETA.NE.0.)GO TO 712
Y1=SQRT(ABS(RF*RF-X1*X1))
Y2=-Y1
GO TO 716
712 Y1=(RR-2.*X1*RA)/(2.*RB)
Y2=(RR-2.*X2*RA)/(2.*RB)
716 THE1=ATAN2(Y1,X1)*57.2957795
THE2=ATAN2(Y2,X2)*57.2957795
IF(Y1.GE.0..AND.Y2.GE.0.)GO TO 740
IF(Y1.LE.0..AND.Y2.LE.0.)GO TO 750
R(1)=PR
R(3)=PR
IF(Y1)721,721,723
723 O(2)=THE1
O(3)=360.+THE2
GO TO 724
721 O(2)=THE2
O(3)=360.+THE1
IF(X2.LT.0.)O(2)=180.+THE2
GO TO 724
740 R(2)=PR
O(2)=THE1
O(3)=THE2
IF(X2.LT.0.)O(3)=180.+THE2
GO TO 724
750 R(2)=PR
O(2)=360.+THE1
O(3)=360.+THE2
724 NDP=3
GO TO 500
770 ICO=0
IF(IELEQ.1)GO TO 402
IF(NSTOP.EQ.CONST)GO TO 500
```

```
IF(ICON.EQ.0)GO TO 406
402 READ(5,100)IELL,NDPP,NSTOP,( IXX(I),I=1,NH)
100 FORMAT(2I5,A4,IX,12I1)
406 IF(IEL.NE.0)IELL)GO TO 403
400 ICON=1
NDP = NDPP
DO 401 I=1,NH
  IXX(I) = IXX(I)
401 CONTINUE
GO TO 407
403 ICON=0
NDP = 0
DO 404 I=1,NH
  IXX(I)=0
404 CONTINUE
407 CONTINUE
IF(NDP.NE.0)GO TO 77
P(1)=0.
R(1)=0.
S(1)=0.
O(1)=0.
NDP = 1
GO TO 78
77 CONTINUE
C *****
C *****
C ***** FIRST DATA POINT MUST BE AT 0.0 DEGREES
C *****
C *****
C ***** READ(5,101)(P(I),R(I),S(I),O(I),I=1,NDP)
500 DO 3006 IL=1,NDP
  IPRI=IPRI+1
  O(NDP+1)=360.
  LLITT=0
  IF(IPRI.EQ.1.AND.IL.EQ.1)LINEL=50
  IF(LINEL.LT.50)GO TO 3007
  WRITE(6,3000)IPAGE
```



```
IPAGE = IPAGE +1
LINEL=0
3000 FORMAT(1H1,62X,4HPAGEI3//)
      LLITT=1
      WRITE(6,3001)
3001 FORMAT(51X,30HAPPLIED LOADS ON THE STRUCTURE//)
      LINEL=LINEL + 6
      GO TO 3010
3007 IF(LLITT.EQ.1)GO TO 3020
      IF(IL.NE.1)GO TO 3008
3010 WRITE(6,3002)IEL
3002 FORMAT(60X,12HELEMENT NO. 13//)
      LINEL=LINEL+2
      IF(LLITT.EQ.1)GO TO 3020
      IF(ICOO.EQ.0)GO TO 3008
3020 WRITE(6,3003)
3003 FORMAT(56X,20HPRESSURE COMPONENTS/)
      WRITE(6,3004)
3004 FORMAT(20X,10HMERIDIONAL,20X,6HNORMAL,20X,10HTANGENTIAL,11X,19HFRO
      1M THETA TO THETA,9H(DEGREES)/)
      LINEL=LINEL+3
3008 WRITE(6,3005)P(IL),R(IL),S(IL),O(IL),O(IL+1)
3005 FORMAT(2X,3(18X,F10.3),12X,2(2X,F8.3)/)
      LINEL=LINEL+2
3006 CONTINUE
      ICOO=0
      ICO=1
      DO 50 I=1,NDP
50 O(I)=O(I)/57.29578
101 FORMAT(4F10.0)
      P(NDP+1)=P(I)
      S(NDP+1)=S(I)
      R(NDP+1)=R(I)
      O(NDP+1) = 2.*3.1415926
78 IZ = IA
      INDEX = 0
      CALL FORCE1(IZ,INDEX,I,ICO,IEL)
```

```
INDEX= 4*IA
IZ=IB
CALL FORCE1(IZ,INDEX,0,ICO,IEL)
IAB= (IA+IB)*4
DO 310 IR=1,NEREST
IF(IELRES(IR).NE. IEL)GO TO 310
DO 301 JR=1,4
IF(REST( IEL,JR).EQ.0)GO TO 301
DO 302 JJR=1,NH
KR=JR+(JJR-1)*4
ONE(KR)=0.
302 CONTINUE
301 CONTINUE
310 CONTINUE
DC 480 KK=1,IAB
480 ONEDD( IEL, KK)=ONE(KK)/XNDELQ
DO 20 JACK = 1,IAB
20 ONE(JACK) = TWO(JACK)
RETURN
END
SUBROUTINE FORCE1(IZ,INDE,ICHANG,ICO,IEL)
COMMON /A1/ G(30),GA(30), P(75),R(75),S(75),O(75)
COMMON /A2/ ONE(48),TWO(48)
COMMON /A3/ NDP,NET,IXX(12)
COMMON /A7/ NTAPE,ISCR1,ISCR3,NELEMS,NH,IPAGE,PROBLE
COMMON /B5/ISIMP
COMMON /B8/ CHECK(8,8)
COMMON /B9/ IA,IB
COMMON /B15/ IELL,NDPP,IXX(12),LINEL,ICON,ICOO,NSTOP
COMMON /D40/ PHPRMI(50),RMDI(50),CPDM(50),SPDM(50),PPPH(50),
1ARLC(50),CHEKC(50,8),R02(50),AIL(50),A2L(50),A3L(50),AL157(50),
2A4L(50),A5L(50),A6L(50),A7L(50),A15L(50),A16L(50),A17L(50),A18L(50)
3),A19L(50),A20L(50),A21L(50),A22L(50),AL158(50),AL159(50)
DIMENSION QO(8),Q(8),OP(101),FORCE(8)
DATA NAMEA/1HA/,NAMEB/1HB/
IF(IZ.EQ.0) RETURN
JACK = (INDE /4)+1
```

```
DO 1 IAAA=1, IZ
99 IAA=IAAA-1
DO 2 KKK=1, 8
FORCE(KKK)=0.
2 Q(KKK)=0.
HIA=IAA
DO 3 I=1, NDP
IF(NDP.EQ.1)GO TO 105
X=O(I)*HIA
XX=O(I+1)*HIA
IF(ICHANG.EQ.0)GO TO 12
IF(IAA.NE.0)GO TO 800
PINT=P(I)*O(I+1)-O(I))
SINT=0.
RINT=R(I)*O(I+1)-O(I))
GO TO 101
800 PINT=P(I)*(SIN(XX)-SIN(X))/HIA
SINT=S(I)*(COS(XX)-COS(X))/HIA
RINT=R(I)*(SIN(XX)-SIN(X))/HIA
GO TO 101
12 IF(IAA.NE.0)GO TO 801
PINT=0.
SINT=S(I)*O(I+1)-O(I))
RINT=0.
GO TO 101
801 PINT=-P(I)*(COS(XX)-COS(X))/HIA
SINT=S(I)*(SIN(XX)-SIN(X))/HIA
RINT=-R(I)*(COS(XX)-COS(X))/HIA
GO TO 101
105 IF(IAA.NE.0) GO TO 102
IF ( ICHANG.EQ.0) GO TO 103
PINT = 2.*3.141592 * P(I)
SINT = 0.
RINT = 2.*3.141592 * R(I)
GO TO 101
103 RINT = 0.
SINT = 2.*3.141592 * S(I)
```

```
PINT = 0.
GO TO 101
102 PINT = 0.
RINT = 0.
SINT = 0.
101 Q(1)=Q(1)+RINT*A1L(IEL)
Q(2)=Q(2)+RINT*A2L(IEL)
Q(3)=Q(3)+RINT*A3L(IEL)
Q(4)=Q(4)+RINT*AL157(IEL)
Q(5)=Q(5)+PINT*A1L(IEL)
Q(6)=Q(6)+PINT*A2L(IEL)
Q(7)=Q(7)+SINT*A1L(IEL)
3 Q(8)=Q(8)+SINT*A2L(IEL)
DO 5 IFLAG = 1,8
QQ(IFLAG) = 0.
DO 5 IFL = 1,8
5 QQ(IFLAG) = QQ(IFLAG) + CHECK(IFL,IFLAG)*Q(IFL)
IF(IXX(JACK).EQ.0)GO TO 600
READ(5,500)(FORCE(I),I=1,8)
500 FORMAT(8F10.0)
NAMEH=NAMEB
IF(ICHAN5.EQ.1)NAMEH=NAMEA
IF(LINEL.EQ.0)GO TO 4010
IF(LINEL.LT.50)GO TO 4007
4010 WRITE(6,4000)IPAGE
4000 FORMAT(1H1,62X,4HPAGEI3///)
IPAGE = IPAGE +1
LINEL=0
WRITE(6,4020)
4020 FORMAT(51X,30HAPPLIED LOADS ON THE STRUCTURE//)
LLIT=1
LINEL=LINEL+8
GO TO 4015
4007 IF(ICD.EQ.0.AND.IAAA.EQ.1)GO TO 4015
IF(IAAA.NE.1)GO TO 4016
WRITE(6,4001)
WRITE(6,4003)
```

```
GO TO 4016
4015 WRITE(6,4002)IEL
4002 FORMAT(60X,12HELEMENT NO. I3/)
IF(LLIT.EQ.1)GO TO 4030
IF(IC0.EQ.0)GO TO 4016
4030 WRITE(6,4003)
4003 FORMAT(20X,5HAXIAL,20X,10HTANGENTIAL,19X,6HRADIAL,20X,7HANGULAR,5X
1,12HHARMONIC NO./)
WRITE(6,4001)
4001 FORMAT(1H0,44X,42HCENTRATED LINE LOAD COMPONENTS AT NODES/)
LINEL = LINEL + 3
4016 WRITE(6,4004)(FORCE(I),I=1,4),IAA,NAMEH
WRITE(6,4005)(FORCE(I),I=5,8),IAA,NAMEH
4004 FORMAT(1X,10HFIRST NODE,10X,F10.3,18X,F10.3,17X,F10.3,16X,F10.3,8X
1,12,A2)
4005 FORMAT(1X,11HSECOND NODE,9X,F10.3,18X,F10.3,17X,F10.3,16X,F10.3,8X
1,12,A2)
LINEL = LINEL + 4
IC00=1
600 INDEX=4*IAA + INDE
90 ONE(INDEX+1) = QQ(1) + ONE(INDEX+1) + FORCE(1)
ONE(INDEX+2) = QQ(2) + ONE(INDEX+2) + FORCE(2)
ONE(INDEX+3) = QQ(3) + ONE(INDEX+3) + FORCE(3)
ONE(INDEX+4) = QQ(4) + ONE(INDEX+4) + FORCE(4)
TWO(INDEX+1) = QQ(5) + FORCE(5)
TWO(INDEX+2) = QQ(6) + FORCE(6)
TWO(INDEX+3) = QQ(7) + FORCE(7)
TWO(INDEX+4) = QQ(8) + FORCE(8)
JACK = JACK +1
1 CONTINUE
RETURN
END
SUBROUTINE DISPLA
COMMON /A7/ ITL,ISOL2,ISCR3,NELEMS,NH,IPAGE,PROBLE
COMMON /B9/ IA,IB
COMMON /D10/ ISAVE, IDISPL
COMMON /G4/ ONEDD(51,48)
```

```
COMMON /G5/ ONED(51,48)
DOUBLE PRECISION T(1176),RHS1(48),S(2304),COE(48),RHS2(48)
DOUBLE PRECISION ONED
REWIND ITL
REWIND ISOL2
IORD=4*NH
IDISPL=1
ISIZS = IORD*IORD
ISIZT = (ISIZS+IORD)/2
NELP1=NELEMS+1
DO 910 I=1,NELP1
DO 911 J=1,IORD
ONED(I,J)=ONEDD(I,J)
911 CONTINUE
910 CONTINUE
READ(ITL)(T(I),I=1,ISIZT)
DO 600 I=1,IORD
600 RHS1(I)=ONED(I,I)
DO 1 I=2,IORD
L = I+1
K = I-1
DO 1 J=1,K
JI = J+(I*I-1)/2
JJ = (J*J+J)/2
II = (I*I+I)/2
T(JI) = T(JI)/T(JJ)
T(II) = T(II)-T(JI)*T(JI)*T(JJ)
IF(I-IORD)101,1,101
101 DO 3 M=L,IORD
IM = I+(M*M-M)/2
JM = J+(M*M-M)/2
3 T(IM) = T(IM)-T(JI)*T(JM)
1 RHS1(I) = RHS1(I)-T(JI)*RHS1(J)
DO 4 IELEM=1,NELEMS
READ(ITL)(S(I),I=1,ISIZS)
DO 5 I=2,IORD
K = I-1
```

```
DO 5 J=1,K
  JI = J+(I*I-I)/2
DO 5 M=1,IORD
  MDUM = (M-1)*IORD
  IM = I+MDUM
  JM = J+MDUM
5 S(IM) = S(IM)-T(JI)*S(JM)
  WRITE(ISO2){T(I),I=1,ISIZT}
  NNE=IELEM+1
DO 800 KK=1,IORD
  ONED(IELEM, KK)=RHS1(KK)
DO 10 I=1,IORD
  II = (I+I)/2
10 COE(I) = T(II)
  READ(ITL){T(I),I=1,ISIZT}
DO 601 KK=1,IORD
601 RHS2(KK)=ONED(NNE, KK)
DO 11 I=1,IORD
  II = (I+I)/2
  LEAD = (I-1)*IORD
DO 12 M=1,IORD
  LEADM = (M-1)*IORD
  IM = I+(M*M-M)/2
DO 12 J=1,IORD
  JI = LEAD+J
  IF(M-I) 105,106,105
106 T(II) = T(II)-S(JI)*S(JI)/COE(J)
  S(JI) = S(JI)/COE(J)
GO TO 12
105 JM = LEADM+J
  T(IM) = T(IM)-S(JI)*S(JM)
12 CONTINUE
DO 11 J=1,IORD
  JI = LEAD+J
11 RHS2(I) = RHS2(I)-S(JI)*RHS1(J)
DO 15 I=2,IORD
  L = I+1
```

```
K = I-1
DO 15 J=1,K
  JI = J+(I*I-1)/2
  JJ = (J*J+J)/2
  II = (I*I+I)/2
  T(JI) = T(JI)/T(JJ)
  T(II) = T(II)-T(JI)*T(JI)*T(JJ)
  IF (I-IORD) 109,15,109
109 DO 17 M=L,IORD
  IM = I+(M*M-M)/2
  JM = J+(M*M-M)/2
  17 T(IM) = T(IM)-T(JI)*T(JM)
  15 RHS2(I) = RHS2(I)-T(JI)*RHS2(J)
  DO 18 I=1,IORD
  18 RHS1(I) = RHS2(I)
  4 WRITE(ISOL2)(S(I),I=1,ISIZS)
  WRITE(ISOL2)(T(I),I=1,ISIZT)
DO 801 KK=1,IORD
801 ONED(NELEMS+1, KK)=RHS1(KK)
  REWIND ITL
  BACKSPACE ISOL2
DO 202 IELEM=1,NELP1
  KKK=(IELEM-1)*IORD
DO 803 I=1,IORD
  KKTE=KKK+I
  S(KKTE)=ONED(IELEM,I)
803 CONTINUE
202 CONTINUE
DO 804 IELEM=1,NELP1
  KKK=(IELEM-1)*IORD
DO 820 I=1,IORD
  KKTE=KKK+I
  KE=NELP1-IELEM + 1
  ONED(KE,I)=S(KKTE)
820 CONTINUE
804 CONTINUE
DO 805 I=1,IORD
```



```
805 RHS1(I)=ONED(1,I)
   READ(ISOL2)(T(I),I=1,ISIZT)
   ISOLB = ITL
   WRITE(ISOLB)(T(I),I=1,ISIZT)
   BACKSPACE ISOL2
   KORNER = (IORD*IORD+IORD)/2
   RHS1(IORD) = RHS1(IORD)/T(KORNER)
   IOM1 = IORD-1
DO 21 KK=1,IOM1
   I = IORD-KK
   II = (I*I+I)/2
   RHS1(I) = RHS1(I)/T(II)
   K = I+1
DO 21 J=K,IORD
   IJ = I+(J*J-J)/2
21  RHS1(I) = RHS1(I)-RHS1(J)*T(IJ)
DO 806 I=1,IORD
806  ONED(1,I)=RHS1(I)
DO 23 IELEM=1,NELEMS
DO 24 I=1,IORD
24  RHS2(I) = RHS1(I)
   NNE=IELEM+1
DO 807 KK=1,IORD
807  RHS1(KK)=ONED(NNE,KK)
   BACKSPACE ISOL2
   BACKSPACE ISOL2
   READ(ISOL2)(T(I),I=1,ISIZT)
   READ(ISOL2)(S(I),I=1,ISIZS)
   WRITE(ISOLB)(S(I),I=1,ISIZS)
   WRITE(ISOLB)(T(I),I=1,ISIZT)
   BACKSPACE ISOL2
   BACKSPACE ISOL2
DO 25 KK=1,IORD
   I = IORD-KK+1
   II = (I*I+I)/2
   RHS1(I) = RHS1(I)/T(II)
   K = I+1
```

```
IF(KK-1)107,108,107
107 DO 26 J=K, IORD
    IJ = I+(J-J)/2
26 RHS1(I) = RHS1(I)-RHS1(J)*T(IJ)
108 DO 25 J=1, IORD
    IJ = I+(J-1)*IORD
25 RHS1(I) = RHS1(I)-RHS2(J)*S(IJ)
DO 808 KK=1, IORD
808 ONED(NNE, KK)=RHS1(KK)
23 CONTINUE
    REWIND ISOLB
DO 809 IELEM=1, NELP1
    KKK=(IELEM-1)*IORD
DO 810 I=1, IORD
    KKTE=KKK+I
    S(KKTE)=ONED(IELEM, I)
810 CONTINUE
809 CONTINUE
DO 811 IELEM=1, NELP1
    KKK=(IELEM-1)*IORD
DO 821 I=1, IORD
    KKTE=KKK+I
    KE=NELP1-IELEM + 1
    ONED(KE, I)=S(KKTE)
821 CONTINUE
811 CONTINUE
    REWIND ISOLB
IF(ISAVE.NE.0)RETURN
    REWIND ISOL2
    READ(ITL)(T(I), I=1, ISIZT)
    WRITE(ISCRA3)(T(I), I=1, ISIZT)
DO 400 I=1, NELEMS
    READ(ITL)(S(J), J=1, ISIZS)
    WRITE(ISCRA3)(S(J), J=1, ISIZS)
    READ(ITL)(T(J), J=1, ISIZT)
    WRITE(ISCRA3)(T(J), J=1, ISIZT)
400 CONTINUE
```

```
DO 401 I=1,NELEMS
  READ(ISOL2)(T(J),J=1,ISIZT)
  WRITE(ISCRA3)(T(J),J=1,ISIZT)
  READ(ISOL2)(S(J),J=1,ISIZS)
  WRITE(ISCRA3)(S(J),J=1,ISIZS)
401 CONTINUE
  READ(ISOL2)(T(J),J=1,ISIZT)
  WRITE(ISCRA3)(T(J),J=1,ISIZT)
  REWIND ITL
  REWIND ISOL2
  RETURN
END
SUBROUTINE NONLIN
COMMON /A5/ EM(50),POISSO(50)
COMMON /A7/ ITL,ISOL2,ISCRA3,NELEMS,NH,IPAGE,PROBLE
COMMON /B8/ CHECK(8,6)
COMMON /B9/ IA,IB
COMMON /B14/ XTHETA(10),NUNKTH
COMMON /B18/ NAMES,HS
COMMON /B19/ IELRES(10),REST(50,8),NEREST
COMMON /B30/ NA,NB
COMMON /C1/ NDELQ
COMMON /D10/ ISAVE,IDISPL
COMMON /D40/ PHPRMI(50),RMDI(50),CPDM(50),SPDM(50),PPPH(50),
1ARLC(50),CHEKCC(50,8,8),R02(50),AIL(50),A2L(50),A3L(50),AL157(50),
2A4L(50),A5L(50),A6L(50),A7L(50),A15L(50),A16L(50),A17L(50),A18L(50
3),A19L(50),A20L(50),A21L(50),A22L(50),AL158(50),AL159(50)
COMMON /F1/ ITER,EROR,X,ITLAST,NSTRAI,IPLAST,XSM
COMMON /F2/ QQ2(51,48),QPR(8,10),QQ0(51,48),QQ1(51,48),QP1(48),
1QP2(48),Q1(48),Q2(48),Q(48),QP(8,12),ALG2(8,12),CC1,CC2,CC,CG1,
2ES(10),ET(10),EST(10),E13(10),E23(10),QQ3(48)
COMMON /F4/ QQ2P(51,48)
COMMON /G2/ NCE(50),NAET(50),THETA(50,51),THICKC(50,51)
COMMON /G4/ DELQ(51,48)
COMMON /G5/ ONED(51,48)
COMMON /G9/ TA(50,50)
DOUBLE PRECISION ONED
```

```
INTEGER REST
INTEGER SAND
DATA SAND/4HSAND/
IFO=NA+NB+1
NNODES=NELEMS+1
XNDELQ=NDELQ
NN = 4*NNH
DO 50 I1=1,NDELQ
IDISPL=1
KEY=1
IT=0
XI=I1
IPCF=(XI/XNDELQ)*100.01
IF(I1.EQ.1)GO TO 770
711 CONTINUE
DO 776 I=1,NN
Q1(I)=QQ2P(I,I)
DO 10 IK=1,NN
QPI(IK) = 0.0
10 CONTINUE
DO 40 I2 = 1,NELEMS
PHPMID=PHPRMI(I2)
RMID=RMIDI(I2)
CPMD=CPDM(I2)
SPMD=SPDM(I2)
PHPP=PPPH(I2)
ARCL=ARLC(I2)
DO 4939 ITS1=1,8
DO 4940 ITS2=1,8
CHECK(ITS1,ITS2)=CHECK(I2,ITS1,ITS2)
4940 CONTINUE
4939 CONTINUE
NC=NCE(I2)
NAMET=NAET(I2)
S=ARCL/2.0
E1=EM(I2)
FNUI=POISSO(I2)
```

```
CC1=E1/(1.-FNUI*FNUI)
CC2=CC1
CC=2.*CC1*FNUI
CG1=E1/(2.*(1.+FNUI))
IF(NAMES.NE.SAND)GO TO 301
CC1=2.*CC1
CC2=CC1
CC=2.*CC
CG1=2.*CG1
301 CONTINUE
DO 777 I=1,NM
777 Q2(I)=QQ2P(I2+1,I)
KA=0
DO 12 I3=1,8
DO 12 I4=1,NH
QP(I3,I4) = 0.0
QPR(I3,I4) = 0.0
12 CONTINUE
13 DO 21 I3 = 1,NH
DO 14 K=1,4
KA = KA + 1
Q(K) = Q1(KA)
Q(K+4) = Q2(KA)
14 CONTINUE
DO 20 J = 1,8
SUM = 0.0
DO 16 M=1,8
SUM = SUM + CHECK(J,M)*Q(M)
16 CONTINUE
18 ALG2(J,I3) = SUM
20 CONTINUE
FI=I3-1
IF(I3.GT.IA)FI=IA-I3+1
C1=ALG2(1,I3)+ALG2(2,I3)*S+ALG2(3,I3)*S**2+ALG2(4,I3)*S**3
C2=ALG2(5,I3)+ALG2(6,I3)*S
C3=ALG2(7,I3)+ALG2(8,I3)*S
ES(I3)=ALG2(6,I3)-PHPMID*C1
```

```

ET(I3)=(FI*C3+C2*SPMD+C1*CPMD)/RMID
EST(I3)=-{(FI*C2+C3*SPMD)/RMID+ALG2(8,I3)}
E13(I3)=ALG2(2,I3)+2.*ALG2(3,I3)*S+3.*ALG2(4,I3)*S**2
1+C2*PHPMID
E23(I3)=-{(FI*C1+C3*CPMD)/RMID}
21 CONTINUE
DO 29 I4 = 1,2
IF (I4.NE.1) GO TO 24
IF (IA.EQ.0) GO TO 29
24 DO 28 I5=1,4
IF (IA.NE.0) GO TO 26
IF (I5.NE.4) GO TO 28
26 CALL HARM(I5,I4,I2)
IF (IB.EQ.0) GO TO 30
28 CONTINUE
29 CONTINUE
30 DO 32 II = 1,8
DO 32 JJ=1,NH
SUM = 0.0
DO 33 LL=1,8
33 SUM=SUM + CHECK(LL,II)*QPR(LL,JJ)
QP(II,JJ) = SUM
32 CONTINUE
JA=0
DO 34 II=1,NH
DO 34 JJ=1,4
JA = JA + 1
QP1(JA)=-QP{JJ,II}+QP1(JA) + XI*DEIQ(I2,JA)
QP2(JA) = -QP{JJ+4,II}
34 CONTINUE
DO 510 IR=1,NEREST
IF(IELRES(IR).NE.I2)GO TO 510
DO 501 JR=1,4
IF(REST(I2,JR).EQ.0)GO TO 501
DO 502 JJR=1,NH
KR=JR+(JJR-1)*4
QP1(KR)=0.

```

```
502 CONTINUE
501 CONTINUE
510 CONTINUE
   DO 904 I=1,NN
904  ONED(I2,I)=QP1(I)
      IF (I2.NE.NELEMS) GO TO 36
   DO 35 JA = 1,NN
      QP2(JA)=QP2(JA)+XI*DELO(I2+1,JA)
35  CONTINUE
      DO 810 IR=1,NEREST
      IF(IELRES(IR).NE.NELEMS)GO TO 810
   DJ 801 JR=5,8
      IF(REST(NELEMS,JR).EQ.0)GO TO 801
   DO 802 JJR=1,NH
      KR=JR+(JJR-1)*4-4
      QP2(KR)=0.
802  CONTINUE
801  CONTINUE
810  CONTINUE
   DO 905 I=1,NN
905  ONED(NELEMS+1,I)=QP2(I)
      GO TO 40
36  DO 38 JA = 1,NN
      Q1(JA)=Q2(JA)
      QP1(JA) = QP2(JA)
38  CONTINUE
40  CONTINUE
      CALL DISPL1
770 DO 900 I=1,NNODES
      DO 901 J=1,NN
      QQ2(I,J)=ONED(I,J)
901  CONTINUE
900  CONTINUE
      CALL EXTRAP(I1,I,NN,KEY,NNODES,IPCF)
      IF(KEY.EQ.1)GO TO 711
      IF(I1.EQ.NDELO.AND. IPLAST.EQ.0)GO TO 50
      CALL STRESS(I1,IPCF,I)
```

```
50 CONTINUE
RETURN
END
SUBROUTINE HARM(I SUM, L FLAG, I2)
COMMON /B9/ IA, IB
COMMON /B14/ XTHER, A(10), NUNKTH
COMMON /B30/ NA, NB
DIMENSION JS(4), ID(4), JD(4), LD(4)
ID(1)=0
JD(1)=0
LD(1)=0
ID(2)=0
JD(2)=1
LD(2)=1
ID(3)=1
JD(3)=1
LD(3)=0
ID(4)=1
JD(4)=0
LD(4)=1
GO TO (10, 11, 15, 19), ISUM
10 IM = IA
JM = IA
KM = 0
KKM = 0
GO TO 26
11 IM = IA
JM = IB
KM = 0
KKM = IA
DO 14 I3 = 1, 4
IF(JD(I3)) I3, I2, I3
12 JD(I3) = 1
GO TO 14
13 JD(I3) = 0
14 CONTINUE
GO TO 26
```



```
15 IM = IB
    JM = IA
    KM = IA
    KKM = 0
    DO 18 I3 = 1,4
    IF(ID(I3)) 17,16,17
16 ID(I3) = 1
    GO TO 18
17 ID(I3) = 0
18 CONTINUE
    GO TO 26
19 IM = IB
    JM = IB
    KM = IA
    KKM = IA
    DO 25 I3 = 1,4
    IF(ID(I3)) 21,20,21
20 ID(I3) = 1
    GO TO 22
21 ID(I3) = 0
22 IF(JD(I3)) 24,23,24
23 JD(I3) = 1
    GO TO 25
24 JD(I3) = 0
25 CONTINUE
26 IF (LFLAG.EQ.1) GO TO 30
    LM = IB
    KKKM = IA
    DO 29 I3 = 1,4
    IF(LD(I3)) 28,27,28
27 LD(I3) = 1
    GO TO 29
28 LD(I3) = 0
29 CONTINUE
    GO TO 31
30 LM = IA
    KKKM = 0
```

```

31 DO 35 I3 = 1,4
   IS(I3) = ID(I3) + JD(I3) + LD(I3)
35 CONTINUE
CALL QPRIME(IM, JM, LM, IS, ID, JD, LD, KM, KKM, KKKM, I2)
RETURN
END
SUBROUTINE QPRIME(IM, JM, LM, IS, ID, JD, LD, KM, KKM, KKKM, I2)
COMMON /A7/ NTAPE, ISCRA1, ISCRA3, NELEMS, NH, IPAGE, PROBLE
COMMON /B9/ IA, IB
COMMON /B14/ XTHETA(10), NUNKTH
COMMON /B30/ NA, NB
COMMON /D40/ PHPRMI(50), RMDI(50), CPDM(50), SPD(50), PPPH(50),
1ARLC(50), CHEKC(50, 8, 8), R02(50), AIL(50), A2L(50), A3L(50), AL157(50),
2A4L(50), A5L(50), A6L(50), A7L(50), A15L(50), A16L(50), A17L(50), A18L(50)
3), A19L(50), A20L(50), A21L(50), A22L(50), AL158(50), AL159(50)
COMMON /F2/ QQ2(51, 48), QPR(8, 10), Q00(51, 48), QQ1(51, 48), QP1(48),
1QP2(48), Q1(48), Q2(48), Q(48), QP(8, 12), ALG2(8, 12), CCL, CC2, CC, GGL,
2ES(10), ET(10), EST(10), E13(10), E23(10), QQ3(48)
COMMON /G4/ DELQ(51, 48)
COMMON /G9/ TA(50, 50)
DIMENSION IS(4), ID(4), JD(4), LD(4)
DIMENSION AL(I59)
AL(1) = A1L(I2)
AL(2) = A2L(I2)
AL(3) = A3L(I2)
AL(4) = A4L(I2)
AL(5) = A5L(I2)
AL(6) = A6L(I2)
AL(7) = A7L(I2)
AL(15) = A15L(I2)
AL(16) = A16L(I2)
AL(17) = A17L(I2)
AL(18) = A18L(I2)
AL(19) = A19L(I2)
AL(20) = A20L(I2)
AL(21) = A21L(I2)
AL(22) = A22L(I2)

```

```

AL(157)=AL157(I2)
AL(158)=AL158(I2)
AL(159)=AL159(I2)
DO 99 L=1,LM
IF (LM.EQ.1A) HL=(L-1)
IF (LM.EQ.1B) HL=-1
DO 99 I=1,IM
DO 99 J=1,JM
IF ((J+I).NE.(L+1)).AND.(ABS(J-I).NE.(L-1)) GO TO 99
IKM = I + KM
JKM = J + KKM
LKM = L + KKKM
IDI=I-1
JDI=J-1
LDI=L-1
CALL INTRL(IDI, JDI, LDI, IS(1), ID(1), JD(1), LD(1), SIMP1, I2)
CALL INTRL(IDI, JDI, LDI, IS(2), ID(2), JD(2), LD(2), SIMP2, I2)
CALL INTRL(IDI, JDI, LDI, IS(3), ID(3), JD(3), LD(3), SIMP3, I2)
CALL INTRL(IDI, JDI, LDI, IS(4), ID(4), JD(4), LD(4), SIMP4, I2)
QPR(1,LKM) = -E13(IKM)*E13(JKM)*CC1*SIMP1*AL(4)/2. - ET(IKM)
1*E23(JKM)*CC2*SIMP2*HL*AL(15) + E23(IKM)*E23(JKM)*CC2*SIMP3*AL(17)
2/2. - ES(IKM)*E23(JKM)*CC*SIMP2*HL*AL(15)/2. - E23(IKM)*E23(JKM)
3*CC*SIMP3*AL(4)/4. + E13(IKM)*E13(JKM)*CC*SIMP1*AL(17)/4.
4-EST(IKM)*E13(JKM)*GG1*SIMP4*HL*AL(15) + QPR(1,LKM)
QPR(2,LKM) = ES(IKM)*E13(JKM)*CC1*SIMP1*AL(1) - E13(IKM)*E13(JKM)
1*CC1*SIMP1*AL(5)/2. - ET(IKM)*E23(JKM)*CC2*SIMP2*HL*AL(16)
2+E23(IKM)*E23(JKM)*CC2*SIMP3*AL(18)/2. - ES(IKM)*E23(JKM)*CC*SIMP2
3*HL*AL(16)/2. - E23(IKM)*E23(JKM)*CC*SIMP3*AL(5)/4. + ET(IKM)
4+E13(JKM)*CC*SIMP1*AL(1)/2. + E13(IKM)*E13(JKM)*CC*SIMP1*AL(18)/4.
5-EST(IKM)*E13(JKM)*GG1*SIMP4*HL*AL(16) + EST(IKM)*E23(JKM)*GG1
6*SIMP3*AL(1) + QPR(2,LKM)
QPR(3,LKM) = ES(IKM)*E13(JKM)*2.*CC1*SIMP1*AL(2) - E13(IKM)
1*E13(JKM)*CC1*SIMP1*AL(6)/2. - ET(IKM)*E23(JKM)*CC2*SIMP2*HL
2*AL(158) + E23(IKM)*E23(JKM)*CC2*SIMP3*AL(19)/2. - ES(IKM)
3+E23(JKM)*CC*SIMP2*HL*AL(158)/2. - E23(IKM)*E23(JKM)*CC*SIMP3
4*AL(158) + ET(IKM)*E13(JKM)*CC*SIMP1*AL(2) - E13(IKM)*E13(JKM)
5*CC*SIMP1*AL(19)/4. - EST(IKM)*E13(JKM)*GG1*SIMP4*HL*AL(158)

```

```

6+EST(IKM)*E23(JKM)*2.*GGI*SIMP3*AL(2) + QPR(3,LKM)
QPR(4,LKM) = ES(IKM)*E13(JKM)*3.*CC1*SIMP1*AL(3) - E13(IKM)
1+E13(JKM)*CC1*SIMP1*AL(7)/2. - ET(IKM)*E23(JKM)*CC2*SIMP2*HL
2*AL(159) + E23(IKM)*E23(JKM)*CC2*SIMP3*AL(20)/2. - ES(IKM)
3+E23(JKM)*CC*SIMP2*HL*AL(159)/2. - E23(IKM)*E23(JKM)*CC*SIMP3
4*AL(7)/4. + ET(IKM)*E13(JKM)*CC*SIMP1*AL(3)*3./2. + E13(IKM)
5+E13(JKM)*CC*SIMP1*AL(20)/4. - EST(IKM)*E13(JKM)*GGI*SIMP4*HL
6*AL(159) + EST(IKM)*E23(JKM)*3.*GGI*SIMP3*AL(3) + QPR(4,LKM)
QPR(5,LKM) = ES(IKM)*E13(JKM)*CC1*SIMP1*AL(4) + E23(IKM)*E23(JKM)
1*CC2*SIMP3*AL(21)/2. + ET(IKM)*E13(JKM)*CC*SIMP1*AL(4)/2.
2+E13(IKM)*E13(JKM)*CC*SIMP1*AL(21)/4. + EST(IKM)*E23(JKM)*GGI
3*SIMP3*AL(4) - E23(IKM)*E13(JKM)*GGI*SIMP4*HL*AL(15) + QPR(5,LKM)
QPR(6,LKM) = ES(IKM)*E13(JKM)*CC1*SIMP1*AL(5) + E13(IKM)*E13(JKM)
1*CC1*SIMP1*AL(1)/2. + E23(IKM)*E23(JKM)*CC2*SIMP3*AL(22)/2.
2+E23(IKM)*E23(JKM)*CC*SIMP3*AL(1)/4. + ET(IKM)*E13(JKM)*CC*SIMP1
3*AL(5)/2. + E13(IKM)*E13(JKM)*CC*SIMP1*AL(22)/4. + EST(IKM)
4+E23(JKM)*GGI*SIMP3*AL(5) - E23(IKM)*E13(JKM)*GGI*SIMP4*HL*AL(16)
5+QPR(6,LKM)
QPR(7,LKM) = -ET(IKM)*E23(JKM)*CC2*SIMP2*AL(17) + E23(IKM)
1+E23(JKM)*CC2*SIMP3*HL*AL(15)/2. - ES(IKM)*E23(JKM)*CC*SIMP2
2*AL(17)/2. + E13(IKM)*E13(JKM)*CC*SIMP1*HL*AL(15)/4. - EST(IKM)
3+E13(JKM)*GGI*SIMP4*AL(17) - E23(IKM)*E13(JKM)*GGI*SIMP4*AL(21)
4+QPR(7,LKM)
QPR(8,LKM) = -ET(IKM)*E23(JKM)*CC2*SIMP2*AL(18) + E23(IKM)
1+E23(JKM)*CC2*SIMP3*HL*AL(16)/2. - ES(IKM)*E23(JKM)*CC*SIMP2
2*AL(18)/2. + E13(IKM)*E13(JKM)*CC*SIMP1*HL*AL(16)/4. - EST(IKM)
3+E13(JKM)*GGI*SIMP4*AL(18) + E23(IKM)*E13(JKM)*GGI*SIMP4*AL(1)
4-E23(IKM)*E13(JKM)*GGI*SIMP4*AL(22) + QPR(8,LKM)
99 CONTINUE
RETURN
END
SUBROUTINE INTRL(I,J,L,IS,JD,LD,HL)
COMMON /B30/ NA,NB
COMMON /G9/ TA(50,50)
HI1=0.
HI2=0.
HI3=0.

```

```
IF(IS.EQ.1)GO TO 65
IF (IS.EQ.0) GO TO 10
IF(IS=2)65,15,65
KEY=3
IF(LD.EQ.0)GO TO 12
IF (ID) 30,35,30
IA=L
LAI=I
JA=J
GO TO 11
JA=L
LAI=J
IA=I
GO TO 11
KEY=I
IA=I
JA=J
LAI=L
LA2=0
HI1=TA(M,1)*VAL(IA,JA,LA1,LA2,KEY)
IF(NB.EQ.0)GO TO 55
DO 40 K=1,NA
LA2=K
40 HI2=HI2+TA(M,K+1)*VAL(IA,JA,LA1,LA2,KEY)
55 IF (IS.EQ.0 .OR. IS.EQ.2) GO TO 45
65 IF(NB.EQ.0)GO TO 45
IF(IS.EQ.3)GO TO 70
KEY=3
IF(LD.EQ.0)GO TO 75
JA=L
LAI=J
LA2=I
GO TO 90
75 IF(JD)80,85,80
85 LA2=J
LAI=L
JA=I
```

```
GO TO 90
70 KEY=5
80 JA=J
   LA1=L
   LA2=I
90 DO 50 K=1,NB
   IA=K
   KS=K+NA
50 HI3=HI3+TA(M,KS +1)*VAL(JA,JA,LA1,LA2,KEY)
45 HI=HI1+HI2+HI3
   RETURN
   END
FUNCTION VAL(I,J,L,LA,KEY)
PI=3.14159265
VAL=0.0
GO TO (10,16,10,16,10),KEY
16 RETURN
10 IPJ=IABS(I+J)
   IMJ=IABS(I-J)
   LPLA=IABS(L+LA)
   LMLA=IABS(L-LA)
   IF(IPJ.EQ.LPLA .OR.
11 IMJ.EQ.LMLA)GO TO 11
   RETURN
   IF (IPJ-LPLA) 20,25,20
25 IF (IPJ) 21,26,21
26 VAL=PI/2.
   GO TO 27
21 VAL=PI/4.
27 IF (KEY.EQ.3) VAL=-VAL
20 IF (IPJ-LMLA) 30,35,30
35 IF (IPJ) 31,36,31
36 FI1=-PI/2.
   GO TO 37
31 FI1=-PI/4.
37 IF (KEY.EQ.1) FI1=-FI1
   VAL=VAL+FI1
```

```
30 IF (IMJ-LPLA) 40,45,40
45 IF (IMJ) 41,46,41
46 FII=PI/2.
   GO TO 47
41 FII=PI/4.
47 IF (KEY.EQ.5) FII=-FII
   VAL=VAL+FII
40 IF (IMJ.NE.LMLA) RETURN
55 IF (IMJ) 51,56,51
56 VAL=VAL+PI/2.
   RETURN
51 VAL=VAL+PI/4.
   RETURN
   END
SUBROUTINE EXTRAP(I1,IT,NN,KEY,NNODES,IPCF)
COMMON /A7/ ITL,ISOL2,ISCR3,NELEMS,NH,IPAGE,PROBLE
COMMON /C1/ NDELQ
COMMON /B30/ NA,NB
COMMON /B14/ XTHETA(10),NUNKTH
COMMON /B18/ NAMES,HS
COMMON /F1/ ITER,ERROR,X,ITLAST,NSTRAI,IPLAST,XSM
COMMON /F2/ QQ2(51,48),QPR(8,10),QQ0(51,48),QQ1(51,48),QP1(48),
IQP2(48),Q1(48),Q2(48),Q(48),QP(8,12),ALG2(8,12),CC1,CC2,CC,CG1,
2ES(10),ET(10),EST(10),E13(10),E23(10),QQ3(48)
COMMON /F4/ QQ2P(51,48)
COMMON /G4/ DELQ(51,48)
IPS=0
IT=IT+1
IF(ITER)20,16,19
19 IF(IT.GT.ITER)GO TO 15
IF(I1.EQ.1.AND.IT.EQ.1)GO TO 9
DO 1 INODE=1,NNODES
DO 1 I=1,NN
IF(ABS(QQ2(INODE,I))-XSM)1,1,18
18 CONTINUE
ARG=ABS(QQ2(INODE,I)-QQ2P(INODE,I))
IF(ARG-ERROR)1,1,9
```

```

1 CONTINUE
  IPS=1
  GO TO 16
15 IT=IT-1
  WRITE(6,500)II,ERROR,IT
500 FORMAT (//14H INCREMENT NO.,I4/51H SOLUTION DOES NOT CONVERGE TO S
  1PECIFIED ACCURACY (F6.3,8H) AFTER,I4,11H ITERATIONS/11H . . . ST
  20P)
  STOP
20 IF(IT+ITER)9,9,16
16 IF(II.EQ.NDELQ.AND.ITER.EQ.0.AND.IT.LE.ITLAST)GO TO 9
  KEY=0
  IF(II.EQ.NDELQ)RETURN
  IF(II.GT.1)GO TO 5
  DO 4 INODE=1,NNODES
  DO 2 I=1,NN
    QQ1(INODE,I)=QQ2(INODE,I)
  2 QQ0(INODE,I)=0.0
  DO 3 I=1,NN
    QQ2P(INODE,I)=(1.0 + X)*QQ2(INODE,I)
  3 CONTINUE
  RETURN
5 DO 7 INODE=1,NNODES
  II=0
  DO 6 I=1,NN
  II=II+1
  QQ3(II)=(1.0+1.5*X+0.5*X**2)*QQ2(INODE,I)+(2.0*X*X**2)*QQ1(INODE,
  II)+0.5*(X+X**2)*QQ0(INODE,II)
  QQ0(INODE,II)=QQ1(INODE,II)
  QQ1(INODE,II)=QQ2(INODE,II)
  6 QQ2P(INODE,I)=QQ3(II)
  7 CONTINUE
  RETURN
9 KEY=1
  IF(IPS.EQ.1)CALL STRESS(II,IPUF,IV)
  DO 10 INODE=1,NNODES
  DO 10 J=1,NN

```



```

10 QQ2P(INODE,J)=QQ2(INODE,J)
RETURN
END
SUBROUTINE DISPL1
COMMON /A7/ ITL, ISOL2, ISCRAB, NELEMS, NH, IPAGE, PROBLE
COMMON /B9/ IA, IB
COMMON /D10/ ISAVE, IDISPL
COMMON /G4/ ONEDD(51,48)
COMMON /G5/ ONED(51,48)
DOUBLE PRECISION T(1176), RHS1(48), RHS2(48), S(2304)
DOUBLE PRECISION ONED
IORD=4*NH
ISIZS = IORD*IORD
ISIZT = (ISIZS+IORD)/2
NELP1 = NELEMS+1
IF(IDISPL.NE.0)GO TO 500
REWIND ISOL2
REWIND ITL
READ(ISCRAB)(T(I), I=1, ISIZT)
WRITE(ITL)(T(I), I=1, ISIZT)
DO 400 I=1, NELEMS
READ(ISCRAB)(S(J), J=1, ISIZS)
WRITE(ITL)(S(J), J=1, ISIZS)
READ(ISCRAB)(T(J), J=1, ISIZT)
WRITE(ITL)(T(J), J=1, ISIZT)
400 CONTINUE
DO 401 I=1, NELEMS
READ(ISCRAB)(T(J), J=1, ISIZT)
WRITE(ISOL2)(T(J), J=1, ISIZT)
READ(ISCRAB)(S(J), J=1, ISIZS)
WRITE(ISOL2)(S(J), J=1, ISIZS)
401 CONTINUE
READ(ISCRAB)(T(J), J=1, ISIZT)
WRITE(ISOL2)(T(J), J=1, ISIZT)
REWIND ISOL2
REWIND ITL
500 READ(ISOL2)(T(I), I=1, ISIZT)

```

```
DO 600 I=1, IORD
600 RHS1(I)=ONED(I, I)
DO 1 I=2, IORD
K = I-1
DO 1 J=1, K
JI = J*(I+I-1)/2
1 RHS1(I) = RHS1(I)-T(JI)*RHS1(J)
DO 3 IELEM=1, NELEMS
READ(ISOL2)(S(I), I=1, ISIZS)
NNE=IELEM+1
DO 800 KK=1, IORD
ONED(IELEM, KK)=RHS1(KK)
800 READ(ISOL2)(T(I), I=1, ISIZT)
DO 601 KK=1, IORD
RHS2(KK)=ONED(NNE, KK)
601 DO 4 I=1, IORD
LEAD = (I-1)*IORD
DO 4 J=1, IORD
JI = LEAD+J
4 RHS2(I) = RHS2(I)-S(JI)*RHS1(J)
DO 6 I=2, IORD
K = I-1
DO 6 J=1, K
JI = J*(I+I-1)/2
6 RHS2(I) = RHS2(I)-T(JI)*RHS2(J)
DO 3 I=1, IORD
3 RHS1(I) = RHS2(I)
DO 801 KK=1, IORD
801 ONED(NELEMS+1, KK)=RHS1(KK)
DO 202 IELEM=1, NELP1
KKK=(IELEM-1)*IORD
DO 803 I=1, IORD
KKTE=KKK+I
S(KKTE)=ONED(IELEM, I)
803 CONTINUE
202 CONTINUE
DO 804 IELEM=1, NELP1
```

```
KKK=(IELEM-1)*IORD
DO 820 I=1, IORD
  KKTE=KKK+I
  KE=NELPI-IELEM + 1
  ONED(KE, I)=S(KKTE)
820 CONTINUE
804 CONTINUE
DO 805 I=1, IORD
  KORSI(I)=ONED(1, I)
  REWIND ISOL2
  READ(ITL)(T(I), I=1, ISIZT)
  KORNER = (IORD*IORD+IORD)/2
  RHSI(IORD) = RHSI(IORD)/T(KORNER)
  IOM1 = IORD-1
DO 11 KK=1, IOM1
  I = IORD-KK
  II = (I+I)/2
  RHSI(I) = RHSI(I)/T(II)
  K = I+1
DO 11 J=K, IORD
  IJ = I+((J-J)/2)
  11 RHSI(I) = RHSI(I)-RHSI(J)*T(IJ)
806 ONED(1, I)=RHSI(I)
DO 13 IELEM=1, NELEMS
DO 14 I=1, IORD
  14 RHS2(I) = RHSI(I)
  NNE=IELEM+1
DO 807 KK=1, IORD
  RHSI(KK)=ONED(NNE, KK)
  READ(ITL)(S(I), I=1, ISIZS)
  READ(ITL)(T(I), I=1, ISIZT)
DO 15 KK=1, IORD
  I = IORD-KK+1
  II = (I+I)/2
  RHSI(I) = RHSI(I)/T(II)
  K = I+1
```

```
IF (KK-1) 101,102,101
101 DO 16 J=K, IORD
  IJ = I+(J-J-1)/2
16 RHS1(I) = RHS1(I)-RHS1(J)*T(IJ)
102 DO 15 J=1, IORD
  IJ = I+(J-1)*IORD
15 RHS1(I) = RHS1(I)-RHS2(J)*S(IJ)
DO 808 KK=1, IORD
808 ONED(NNE, KK)=RHS1(KK)
13 CONTINUE
DO 809 IELEM=1, NELP1
  KKK=(IELEM-1)*IORD
DO 810 I=1, IORD
  KKTE=KKK+I
  S(KKTE)=ONED(IELEM, I)
810 CONTINUE
809 CONTINUE
DO 811 IELEM=1, NELP1
  KKK=(IELEM-1)*IORD
DO 821 I=1, IORD
  KKTE=KKK+I
  KE=NELP1-IELEM + 1
  ONED(KE, I)=S(KKTE)
821 CONTINUE
811 CONTINUE
  IDISPL=IDISPL+1
  REWIND ITL
  RETURN
END
SUBROUTINE STRESS(I1, IPCF, ITIT)
COMMON /A5/ EM(50), POISSO(50)
COMMON /A7/ NTAPE, ISCRAI, ISCRAB, NELEMS, NH, IPAGE, PROBLE
COMMON /B9/ IA, IB
COMMON /B10/ C(8,8), PHPMID, RMID, CPMD, SPMD, PPHP, ARCL
COMMON /B11/ ST(50,10,6), STR(50,10,8), F(10), CAPA(6)
COMMON /B14/ XTHETA(10), NUNKTH
COMMON /B18/ NAMES, HS
```

```
COMMON /B19/ IELRES(10),REST(50,8),NEREST
COMMON /B22/ Q3(51,48)
COMMON /B30/ NA,NB
COMMON /C1/ NDELQ
COMMON /D10/ ISAVE,IDISPL
COMMON /D40/ PHPRMI(50),RMDI(50),CPDM(50),SPDM(50),PPPH(50),
1ARLC(50),CHEKC(50,8,8),R02(50),A1L(50),A2L(50),A3L(50),A157(50),
2A4L(50),A5L(50),A6L(50),A7L(50),A15L(50),A16L(50),A17L(50),A18L(50
3),A19L(50),A20L(50),A21L(50),A22L(50),A158(50),A159(50)
COMMON /G2/ NCE(50),NAET(50),THETE(50,51),THICKC(50,51)
COMMON /G5/ ONED(51,48)
COMMON /F2/ Q2(51,48),QPR(8,10),QQ0(51,48),QQ1(51,48),QP1(48),
1QP2(48),Q1(48),Q2(48),Q(48),QP(8,12),ALG2(8,12),CC1,CC2,CC,CG1,
2ES(10),ET(10),EST(10),E13(10),E23(10),QQ3(48)
COMMON /F1/ ITER,EROR,X,ITLAST,MSTRAI,IPLAST,XSM
DOUBLE PRECISION ONED
DIMENSION DRE(4)
REWIND ISCR A3
C NN-TOTAL NO. OF Q'S FOR ONE ELEMENT
CALL SOLV
AXT=0.
TANT=0.
RADT=0.
ANGT=0.
DO 100 II=1,NUNKTH
WRITE(6,200)IPAGE
IPAGE = IPAGE +1
WRITE(6,501)
501 FORMAT(50X,32HDISPLACEMENT COMPONENTS OF NODES)
WRITE(6,900)II,IPCF
900 FORMAT(42X,14HINCREMENT NO. I2,5H *** I4,22H PERCENT OF TOTAL LOAD
1)
WRITE(6,901)ITIT
901 FORMAT(55X,24HITERATION NO. I2)
WRITE(6,503)XTHETA(II)
WRITE(6,502)
502 FORMAT(23X,8HNODE NO. I0X,5HAXIAL I12X,10HTANGENTIAL I1X,6HRADIAL,
```

```
112X,7HANGULAR/)
503 FORMAT(55X,6HTHETA=F8.3,3X,7HDEGREES/,
NNODES=NELEMS+1
DO 10 IJ=1,NNODES
DO 102 IN=1,4
DRE(IN)=0.
102 CONTINUE
104 INE=IJ
105 IF(IA.EQ.0)GO TO 106
DO 107 IG=1,IA
ID=(IG-1)*4 + 1
IT=IG - 1
TI=IT
TH=TI*XTHETA(II)/57.2957795
DRE(1)=DRE(1) + Q3(INE, ID)*COS(TH)
DRE(2)=DRE(2) + Q3(INE, ID+1)*SIN(TH)
DRE(3)=DRE(3) + Q3(INE, ID+2)*COS(TH)
DRE(4)=DRE(4) + Q3(INE, ID+3)*COS(TH)
107 CONTINUE
106 IF(IB.EQ.0)GO TO 108
DO 109 IG=1,IB
ID=(IG-1)*4+1+4*IA
IT=IG - 1
TI=IT
TH=TI*XTHETA(II)/57.2957795
DRE(1)=DRE(1)+Q3(INE, ID)*SIN(TH)
DRE(2)=DRE(2)+Q3(INE, ID+1)*COS(TH)
DRE(3)=DRE(3)+Q3(INE, ID+2)*SIN(TH)
DRE(4)=DRE(4)+Q3(INE, ID+3)*SIN(TH)
109 CONTINUE
108 IF(ABS(DRE(1)).LT.ABS(AXT))GO TO 170
AXT=DRE(1)
W1=XTHETA(II)
I2=INE
170 IF(ABS(DRE(2)).LT.ABS(TANT))GO TO 171
TANT=DRE(2)
W3=XTHETA(II)
```

```
I4=INE
171 IF(ABS(DRE(3)).LT.ABS(RADT))GO TO 172
    RADT=DRE(3)
    W5=XTHETA(II)
    I6=INE
172 IF(ABS(DRE(4)).LT.ABS(ANGT))GO TO 173
    ANGT=DRE(4)
    W7=XTHETA(II)
    I8=INE
173 CONTINUE
    WRITE(6,110)INE,(DRE(IK),IK=1,4)
110 FORMAT(26X,12,4X,4(4X,E15.8))
10 CONTINUE
100 CONTINUE
    IF(NSTRAI.NE.0)GO TO 480
    DO 210 I=1,NUNKTH
    WRITE(6,200)IPAGE
    IPAGE = IPAGE + 1
200 FORMAT(1H1,62X,4HPAGEI3//)
    WRITE(6,201)
201 FORMAT(37X,58HSTRAINS AND CURVATURES AT THE CENTER OF EACH SHELL E
    LEMENT)
    WRITE(6,900)II,IPCF
    WRITE(6,901)ITII
    WRITE(6,202)XTHETA(I)
202 FORMAT(55X,6HTHETA=F8.3,3X,7HDEGREES/)
    WRITE(6,204)
204 FORMAT(4X,11HELEMENT NO.,10X,4HE(S),13X,8HE(THETA),10X,10HE(S-1)HEI
    1A),12X,4HK(S),13X,8HK(THETA),10X,10HK(S-THETA)//)
    DO 211 J=1,NELEMS
205 WRITE(6,203)J,(ST(J,I,K),K=1,6)
203 FORMAT(8X,12,5X,6(4X,E15.8))
211 CONTINUE
210 CONTINUE
480 DO 310 I=1,NUNKTH
    WRITE(6,200)IPAGE
    IPAGE = IPAGE + 1
```

```
WRITE(6,301)
301 FORMAT(39X,53HSTRESS RESULTANTS AT THE CENTER OF EACH SHELL ELEMEN
IT)
WRITE(6,900)I1,IPCF
WRITE(6,901)ITIT
WRITE(6,202)XTHETA(I)
WRITE(6,304)
304 FORMAT(1X,11HELEMENT NO.,6X,4HN(S),9X,8HN(THETA),6X,10HN(S-THETA),
18X,4HM(S),9X,8HM(THETA),6X,10HM(S-THETA),8X,4HQ(S),9X,8HQ(THETA))//
2)
DO 311 J=1,NELEMS
305 WRITE(6,208)J,(STR(J,I,K),K=1,8)
208 FORMAT(5X,12,4X,8(2X,E13.6))
311 CONTINUE
310 CONTINUE
IF(INSTRAI.NE.0)RETURN
WRITE(6,200)IPAGE
IPAGE = IPAGE + 1
WRITE(6,150)
150 FORMAT(35X,62HMAXIMUM COMPUTED NODE DISPLACEMENT COMPONENTS OF THE
1 STRUCTURE////)
WRITE(6,900)I1,IPCF
WRITE(6,901)ITIT
WRITE(6,151)
151 FORMAT(1H0,22X,5HAXIAL,20X,10HTANGENTIAL,19X,6HRADIAL,20X,
17HANGULAR////)
WRITE(6,152)AXT,TANT,RADT,ANGT
152 FORMAT(6X,4(12X,E15.8)////)
WRITE(6,153)
153 FORMAT(4(27H
AT)////)
WRITE(6,154)
154 FORMAT(1H0,5X,4(13X,14HTHETA NODE)////)
WRITE(6,155)W1,I2,W3,I4,W5,I6,W7,I8
155 FORMAT(1H0,4X,4(12X,F8.3,5X,12)////)
SN = 0.
TN = 0.
TSN= 0.
```



```
SM = 0.  
TM = 0.  
TSM = 0.  
QS=0.  
QST=0.  
DO 180 I=1,NELEMS  
DO 181 J=1,NUNKTH  
IF(ABS(STR(I,J,1)).LT.ABS(SN))GO TO 190  
SN=STR(I,J,1)  
W1=XTHETA(J)  
I2=I  
190 IF(ABS(STR(I,J,2)).LT.ABS(TM))GO TO 191  
TN=STR(I,J,2)  
W3=XTHETA(J)  
I4=I  
191 IF(ABS(STR(I,J,3)).LT.ABS(TSN))GO TO 192  
TSN=STR(I,J,3)  
W5=XTHETA(J)  
I6=I  
192 IF(ABS(STR(I,J,4)).LT.ABS(SM))GO TO 193  
SM=STR(I,J,4)  
W7=XTHETA(J)  
I8=I  
193 IF(ABS(STR(I,J,5)).LT.ABS(TM))GO TO 194  
TM=STR(I,J,5)  
W9=XTHETA(J)  
I10=I  
194 IF(ABS(STR(I,J,6)).LT.ABS(TSM))GO TO 195  
TSM=STR(I,J,6)  
W11=XTHETA(J)  
I12=I  
195 IF(ABS(STR(I,J,7)).LT.ABS(QS))GO TO 196  
QS=STR(I,J,7)  
W13=XTHETA(J)  
I14=I  
196 IF(ABS(STR(I,J,8)).LT.ABS(QST))GO TO 197  
QST=STR(I,J,8)
```

```
W15=XTHETA(J)
I16=I
197 CONTINUE
181 CONTINUE
180 CONTINUE
WRITE(6,156)
156 FORMAT(1H0,34X,61HMAXIMUM COMPUTED STRESS RESULTANT COMPONENTS OF
1THE STRUCTURE////)
WRITE(6,157)
157 FORMAT(8X,4HN(S),10X,8HN(THETA),7X,10HN(S-THETA),9X,4HM(S),10X,
18HM(THETA),7X,10HM(S-THETA),9X,4HQ(S),10X,8HO(THETA)////)
WRITE(6,158)SN,TN,TSN,SM,TM,TSM,QS,QST
158 FORMAT(8(3X,E13.6)////)
WRITE(6,159)
159 FORMAT(1H0,8X,2HAT,7(14X,2HAT)////)
WRITE(6,160)
160 FORMAT(8(3X,13HELEMENT THETA)////)
WRITE(6,161)I2,W1,I4,W3,I6,W5,I8,W7,I10,W9,I12,W11,I14,W13,I16,W15
161 FORMAT(1H0,8(4X,12,2X,F8.3)////)
RETURN
END
SUBROUTINE SOLV
COMMON /A5/ EM(50),POISSO(50)
COMMON /A7/ NTAPE,ISCRAL,ISCRAB,NELEMS,NH,IPAGE,PROBLE
COMMON /B9/ IA,IB
COMMON /B10/ C(6,8),PHPMID,RMID,CPMD,SPMD,PHPP,ARCL
COMMON /B11/ ST(50,10,6),STR(50,10,8),T(10),CAPA(6)
COMMON /B14/ XTHETA(10),NUNKTH
COMMON /B19/ IELRES(10),REST(50,8),NEREST
COMMON /B18/ NAMES,HS
COMMON /B22/ Q3(51,48)
COMMON /B30/ NA,NB
COMMON /D40/ PHPRMI(50),RMDI(50),CPDM(50),SPDM(50),PPPH(50),
1ARLC(50),CHEKC(50,8,8),R02(50),A1L(50),A2L(50),A3L(50),AL157(50),
2A4L(50),A5L(50),A6L(50),A7L(50),A15L(50),A16L(50),A17L(50),A18L(50)
3),A19L(50),A20L(50),A21L(50),A22L(50),AL15R(50),AL159(50)
COMMON /F2/ QQ2(51,48),QPR(8,10),QQO(51,48),QO1(51,48),QP1(48),
```

```
1QP2(48),Q7(48),Q8(48),R(48),R(48),QP(8,12),ALG2(8,12),CC1,CC2,CC,CG1,
2ES(10),ET(10),EST(10),E13(10),E23(10),QQ3(48)
COMMON /G2/ NCE(50),NAET(50),THETE(50,51),THICKC(50,51)
COMMON /G5/ ONED(51,48)
DOUBLE PRECISION ONED
DIMENSION AB(12,8)
DIMENSION THETA(51),THIC(51)
DIMENSION Q1(48),Q2(48),Q(8)
INTEGER REST
NN = 4*(IA + IB)
IFO=NA+NB+1
DO 300 I=1,NN
300 Q1(I)=ONED(1,I)
DO 800 II=1,NN
800 Q3(1,II)=Q1(II)
DO 10 IJ=1,NELEMS
DO 301 I=1,NN
301 Q2(I)=ONED(IJ+1,I)
DO 801 II=1,NN
Q3(IJ+1,II)=Q2(II)
801 CONTINUE
C KA-COUNTER FOR ADDRESSING Q'S IN THE ARRAYS Q1 AND Q2
C IAB-TOTAL NO. OF HARMONICS FOR ONE NODE
PHPMID=PHPRMI(IJ)
RMID=RMDI(IJ)
CPMD=CPDM(IJ)
SPMD=SPDM(IJ)
PHPP=PPPH(IJ)
ARCL=ARLC(IJ)
DO 4939 ITS1=1,8
DO 4940 ITS2=1,8
C(ITS1,ITS2)=CHEKC(IJ,ITS1,ITS2)
4940 CONTINUE
4939 CONTINUE
NC=NCE(IJ)
NAMET=NAET(IJ)
DO 4937 ITH=1,NC
```

```
THETA(ITH)=THETE(IJ,ITH)
THIC(ITH)=THICKC(IJ,ITH)
4937 CONTINUE
KA=1
IAB=IA+IB
C COMPUTE ALPHAS FOR ALL HARMONICS AT ONE NODE
DO15 I=1,IAB
C PUT Q1'S AND Q2'S FOR ONE HARMONIC INTO COLUMN MATRIX
DO 20 K=1,4
Q(K)=Q1(KA)
Q(K+4)=Q2(KA)
KA=KA+1
20 CONTINUE
C MULTIPLY GIVEN 8X8 TRANSFORMATION MATRIX C BY COLUMN MATRIX OF ALPHAS
C FOR THE FIRST HARMONIC AND STORE IN ARRAY AB
DO 25 J=1,8
SUM=0.
DO 30 M=1,8
SUM=C(J,M)*Q(M)+SUM
30 CONTINUE
AB(I,J)=SUM
25 CONTINUE
15 CONTINUE
C DO COMPUTATION FOR N NUMBER OF THETAS PER ELEMENT
DO 35 KJ=1,NUNKTH
IF(NC.NE.1)GO TO 100
T(KJ) = THIC(1)
GO TO 101
100 CONTINUE
DO 65 J=1,NC
TH1=THETA(J)*57.29577795
TH2=THETA(J+1)*57.29577795
IF(XTHETA(KJ).GE.TH1.AND.XTHETA(KJ).LE.TH2)GO TO 12
65 CONTINUE
12 T(KJ)=THIC(J)
C COMPUTE STRAIN RESULTANTS AND RETURN IN ARRAY ST
101 CONTINUE
```

```
CALL STRAIN(AB,KJ,IAB,IJ)
C COMPUTE STRESS RESULTANTS AND RETURN IN ARRAY STR(I,J,K) WHERE I=ELEMENT
C COMPUTE CAPAS-PARTIAL DERIVATIVES OF THE RADIUS OF CURVATURE
C NO.,J=THETA NO.,K=COLUMN MATRIX 1-8 GIVING VALUES OF NS,NTHETA,NSTHETA,
C MS,MTHETA,MSTHETA,QS,QTHETA RESPECTIVELY
CALL STRESR(IJ,KJ)
35 CONTINUE
C MOVE Q'S FOR NODE P+1 TO NODE P
DO 40 L=1,NN
Q1(L)=Q2(L)
40 CONTINUE
10 CONTINUE
RETURN
END
SUBROUTINE STRAIN(AB,KJ,IAB,IJ)
COMMON /A5/ EM(50),POISSO(50)
COMMON /B9/ IA,IB
COMMON /B10/ C(8,8),PHPMID,RMID,CPMD,SPMD,PHPP,ARCL
COMMON /B11/ ST(50,10,6),STR(50,10,8),T(10),CAPA(6)
COMMON /B14/ XTHETA(10),NUNKTH
COMMON /B19/ IELRES(10),REST(50,8),NEREST
DIMENSION AB(12,8),U(40)
S=ARCL/2.
DO 45 I=1,40
U(I)=0.
45 CONTINUE
C TEST FOR A HARMONICS, IF NO, GO TO TEST FOR B HARMONICS
IF(IA.EQ.0) GO TO 110
C COMPUTE US-OR U(1),VTHETA-U(2),UTHEAT-U(3),VS-U(4),WS-U(5),WSS-U(6),
C WTHETA-U(7),MTHETATHETA-U(8),U-U(9),V-V(10),W-U(11),WSTHETA-U(12),
C WSSS-U(13),WSSSTHETA-U(14),MTHETATHETAS-U(15),MTHETATHETATHETA-U(16),
C UTHETATHETA-U(17),USTHETA-U(18),VTHETATHETA-U(19),VSTHETA-U(20) IE
C U,V,W AND THEIR PARTIAL DERIVATIVES FOR A HARMONICS
DO 50 I=1,IA
AI=I-1
X=AI*XTHETA(KJ)
X=X/57.2957795
```

```
U(1)=AB(I,6)*COS(X)+U(1)
U(2)=AI*(AB(I,7)+AB(I,8)*S)*COS(X)+U(2)
U(3)=-((AI*(AB(I,5)+AB(I,6)*S)*SIN(X))+U(3)
U(4)=AB(I,8)*SIN(X)+U(4)
U(5)=(AB(I,2)+2.*AB(I,3)*S+3.*AB(I,4)*S**2)*COS(X)+U(5)
U(6)=(2.*AB(I,3)+6.*AB(I,4)*S)*COS(X)+U(6)
U(7)=-((AI*(AB(I,1)+AB(I,2)*S+AB(I,3)*S**2+AB(I,4)*S**3)*SIN(X))
1+U(7)
U(8)=-((AI**2)*(AB(I,1)+AB(I,2)*S+AB(I,3)*S**2+AB(I,4)*S**3)*
1COS(X))+U(8)
U(9)=(AB(I,5)+AB(I,6)*S)*COS(X)+U(9)
U(10)=(AB(I,7)+AB(I,8)*S)*SIN(X)+U(10)
U(11)=(AB(I,1)+AB(I,2)*S+AB(I,3)*S**2+AB(I,4)*S**3)*COS(X)+U(11)
U(12)=-AI*(AB(I,2)+2.*AB(I,3)*S+3.*AB(I,4)*S**2)*SIN(X)+U(12)
U(13)=6.*AB(I,4)*COS(X)+U(13)
U(14)=-((AI*(2.*AB(I,3)+6.*AB(I,4)*S)*SIN(X))+U(14)
U(15)=-((AI**2)*(AB(I,2)+2.*AB(I,3)*S+3.*AB(I,4)*S**2)*COS(X))+
1U(15)
1U(16)=(AI**3)*(AB(I,1)+AB(I,2)*S+AB(I,3)*S**2+AB(I,4)*S**3)*SIN(X)
+U(16)
U(17)=-((AI**2)*(AB(I,5)+AB(I,6)*S)*COS(X))+U(17)
U(18)=-AI*AB(I,6)*SIN(X)+U(18)
U(19)=-((AI**2)*(AB(I,7)+AB(I,8)*S)*SIN(X))+U(19)
U(20)=AI*AB(I,8)*COS(X)+U(20)
50 CONTINUE
C CHECK FOR B HARMONICS
110 IF(IAB.EQ.1A) GO TO 120
K=IA+1
C COMPUTE US,VTHETA,ETC. AS ABOVE BUT FOR B HARMONICS
DO 55 J=K,IAB
AI=J-IA-1
Y=AI*XTHEA(KJ)
Y=Y/57.2957795
U(21)=AB(J,6)*SIN(Y)+U(21)
U(22)=-((AI*(AB(J,7)+AB(J,8)*S)*SIN(Y))+U(22)
U(23)=AI*(AB(J,5)+AB(J,6)*S)*COS(Y)+U(23)
U(24)=AB(J,8)*COS(Y)+U(24)
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U(25)=(AB(J,2)+2.*AB(J,3)*S+3.*AB(J,4)*S**2)*SIN(Y)+U(25)
U(26)=(AB(J,3)*2.+6.*AB(J,4)*S)*SIN(Y)+U(26)
U(27)=AI*(AB(J,1)+AB(J,2)*S+AB(J,3)*S**2+AB(J,4)*S**3)*COS(Y)+
  U(27)
U(28)=-((AI**2)*(AB(J,1)+AB(J,2)*S+AB(J,3)*S**2+AB(J,4)*S**3)*
  SIN(Y))+U(28)
U(29)=(AB(J,5)+AB(J,6)*S)*SIN(Y)+U(29)
U(30)=(AB(J,7)+AB(J,8)*S)*COS(Y)+U(30)
U(31)=(AB(J,1)+AB(J,2)*S+AB(J,3)*S**2+AB(J,4)*S**3)*SIN(Y)+U(31)
U(32)=AI*(AB(J,2)+2.*AB(J,3)*S+3.*AB(J,4)*S**2)*COS(Y)+U(32)
U(33)=6.*AB(J,4)*SIN(Y)+U(33)
U(34)=AI*(2.*AB(J,3)+6.*AB(J,4)*S)*COS(Y)+U(34)
U(35)=-((AI**2)*(AB(J,2)+2.*AB(J,3)*S+3.*AB(J,4)*S**2)*SIN(Y))+
  U(35)
1
U(36)=-((AI**3)*(AB(J,1)+AB(J,2)*S+AB(J,3)*S**2+AB(J,4)*S**3)*
  COS(Y))+U(36)
1
U(37)=-((AI**2)*(AB(J,5)+AB(J,6)*S)*SIN(Y))+U(37)
U(38)=AI*AB(J,6)*COS(Y)+U(38)
U(39)=-((AI**2)*(AB(J,7)+AB(J,8)*S)*COS(Y))+U(39)
U(40)=-AI*AB(J,8)*SIN(Y)+U(40)
55 CONTINUE
C ADD THE A AND B HARMONICS FOR AN ELEMENT
120 DO 60 I=1,20
  U(I)=U(I)+U(I+20)
60 CONTINUE
C USING THE DISPLACEMENT FUNCTIONS AND THEIR PARTIALS, COMPUTE THE NORMAL
C STRAINS AND SHEAR STRAIN FOR AN ELEMENT
ES=U(1)-PHPMID*U(11)
ETHEA=1./RMID*(U(2)+U(9)*SPMD+U(11)*CPMD)
ESTHE=1./RMID*U(3)-U(10)/RMID*SPMD+U(4)
E13=U(5)+U(9)*PHPMID
E23=1./RMID*(U(7)-U(10)*CPMD)
C COMPUTE STRAIN ENERGY USING EQNS. FOR THE STRAINS.
C COMPUTE 3 CURVATURE CHANGES OF SHELL SURFACE.
C ALL EQNS. FOR ONE ANGLE THETA OF ONE ELEMENT.
ST(IJ,KJ,I) =
1 ES+5*E13**2

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```
ST(IJ,KJ,2) =
1  ETHETA+.5*E23**2
ST(IJ,KJ,3) =
1  ESTHE+E13*E23
ST(IJ,KJ,4) =
1  -(U(6)+U(9))*PHPP+PHPMID*U(1)}
ST(IJ,KJ,5) =
1  -1./RMID*(1./RMID*U(8)+U(5))*SPMD+PHPMID*U(9)*SPMD-1./RMID*
*U(2)*CPMD}
ST(IJ,KJ,6) =
1  1./RMID*(2./RMID*U(7))*SPMD-2./RMID*U(10)*SPMD*CPMD-2.*U(12)
*-PHPMID*U(3)+CPMD*U(4)-U(10)*SPMD*PHPMID)
CAPA(1)=-{U(13)+2.*U(1)*PHPP}
CAPA(2)=-{U(14)+U(3)*PHPP +U(18)*PHPMID}
CAPA(3)={1./RMID**2)*(-U(15)+{2./RMID}*SPMD*U(8)-RMID*SPMD*U(6)-
RMID*PHPMID*CPMD*U(5)+SPMD**2*U(5)-PHPP *RMID*SPMD*U(9)
+SPMD**2*PHPMID*U(9)-RMID*PHPMID*SPMD*U(1)-PHPMID**2*RMID
*CPMD*U(9)+CPMD*U(20)-(2./RMID)*CPMD*SPMD*U(2)-PHPMID*
SPMD*U(2)}
CAPA(4)={1./RMID**2)*(-U(16)-RMID*SPMD*U(12)-RMID*PHPMID*SPMD*
U(3)+CPMD*U(19)}
CAPA(5)={1./RMID**2)*(-{4./RMID}*SPMD**2*U(7)+2.*SPMD*U(12)+2.*
PHPMID*CPMD*U(7)+{4./RMID}*SPMD**2*CPMD*U(10)-2.*SPMD*
CPMD*U(4)-2.*CPMD**2*PHPMID*U(10)+2.*SPMD**2*PHPMID*
U(10)+2.*SPMD*U(12)-2.*RMID*U(14)-RMID*PHPP*U(3)+PHPMID
*SPMD*U(3)-PHPMID*RMID*U(18)-PHPMID*RMID*SPMD*U(4)-SPMD
*CPMD*U(4)-RMID*SPMD*U(4)*PHPMID-PHPMID**2*RMID*CPMD*
U(10)+SPMD**2*PHPMID*U(10)-RMID*SPMD*PHPP*U(10)}
CAPA(6)={1./RMID)*{(2./RMID)*SPMD*U(8)-(2./RMID)*SPMD*CPMD*U(2)
-2.*U(15)-PHPMID*U(17)+CPMD*U(20)-SPMD*PHPMID*U(2)}
1  RETURN
END
SUBROUTINE STRESR(IJ,KJ)
COMMON /A5/ EM(50),POISSO(50)
COMMON /B9/ IA,IB
COMMON /B10/ C(8,8),PHPMID,RMID,CPMD,SPMD,PHPP,ARCL
COMMON /B11/ ST(50,10,6),STR(50,10,8),T(10),CAPA(6)
```


COMMON /B14/ XTHETA(10), NUNKTH
COMMON /B18/ NAMES,HS
COMMON /B19/ IELRES(10), REST(50,8), NEREST
INTEGER SAND

DATA SAND/4HSAND/
EE1 = EM(IJ)
FNU1 = POISSO(IJ)
FNU2 = FNU1

IF(NAMES,NE,SAND)GO TO 2
A=2.*(EE1*T(KJ))/(1.-FNU1*FNU2)
CXX=(EE1*T(KJ)*HS**2)/(2.*(1.-FNU1*FNU2))
GO TO 3

2 A=(EE1*T(KJ))/(1.-FNU1*FNU2)
CXX=(EE1*T(KJ)**3)/(12.*(1.-FNU1*FNU2))

3 B=A

D=CXX

G = EE1/(2.0*(1.0 + FNU1))

C FOLLOWING ARE PARTIALS OF MS,MTHETA,MSTHETA

PMSS=CXX*CAPA(1)+FNU1*CXX*CAPA(3)

PMTT=FNU2*D*CAPA(2)+D*CAPA(4)

IF(NAMES,NE,SAND)GO TO 4

PMSTS=G*T(KJ)*HS**2*CAPA(5)/2.

PMSTT=G*T(KJ)*HS**2*CAPA(6)/2.

GO TO 5

4 PMSTS=G*(T(KJ)**3/12.)*CAPA(5)

PMSTT=G*(T(KJ)**3/12.)*CAPA(6)

C COMPUTE STRESS RESULTANTS

5 STR(IJ,KJ,1) = A*ST(IJ,KJ,1) + A*FNU1*ST(IJ,KJ,2)

STR(IJ,KJ,2) = FNU2*B*ST(IJ,KJ,1) + B*ST(IJ,KJ,2)

STR(IJ,KJ,4)=CXX*ST(IJ,KJ,4)+FNU1*CXX*ST(IJ,KJ,5)

STR(IJ,KJ,5) = FNU2*D*ST(IJ,KJ,4) + D*ST(IJ,KJ,5)

STR(IJ,KJ,7)=(1./RMID)*(RMID*PMSS+PMSTT+(STR(IJ,KJ,4)*

STR(IJ,KJ,5))*SPMD)

1 IF(NAMES,NE,SAND)GO TO 6

STR(IJ,KJ,3)=2.*G*T(KJ)*ST(IJ,KJ,3)

STR(IJ,KJ,6)=G*T(KJ)*HS**2*ST(IJ,KJ,6)/2.

STR(IJ,KJ,8)=(1./RMID)*(RMID*PMSTS+PMTT+2.*SPMD*STR(IJ,KJ,6))

```
RETURN  
6 STR(IJ,KJ,3) = G*T(KJ)*ST(IJ,KJ,3)  
  STR(IJ,KJ,6) = (T(KJ)**3/12.)*ST(IJ,KJ,6)*G  
  STR(IJ,KJ,8) = (1./RMID)*(RMID*PMSTS+PMTT+2.*SPMD*STR(IJ,KJ,6))  
RETURN  
END
```