THREE ISOPARAMETRIC SOLID ELEMENTS FOR NASTRAN
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## SUMMARY

Linear, quadratic, and cubic isoparametric hexahedral solid elements have been added to the element library of NASTRAN. These elements are available for static, dynamic, buckling, and heat-transfer analyses. Because the isoparametric element matrices are generated by direct numerical integration over the volume of the element, variations in material properties, temperatures, and stresses within the elements are represented in the computations. In order to compare the accuracy of the new elements, three similar models of a slender cantilever were developed, one for each element. All elements performed well. As expected, however, the linear element model yielded excellent results only when shear behavior predominated. In contrast, the results obtained from the quadratic and cubic element models were excellent in both shear and bending.

## INTRODUCTION

New aerospace vehicle concepts, such as the Space Shuttle, have added impetus for the continued updating oi NASTRAN with the best state-of-the-art finite element technology. In response to this need, the three-dimensional family of linear, quadratic, and cubic isoparametric hexahedral solid elements were developed for and installed in NASTRAN. These three new elements significanty improve NASTRAN's capability to solve any three-dimensional solid problem requiring static, dynamic, buckling, and/or heat-transfor analysis.

## THEORETICAL BACKGROUND

Hexahedron solid isoparametric elements may be used to analyze any threedimensional continuum composed of isotropic or anisotropic materials. Examples include thick inserts in rocket engine nozzles, thermal protection system insulations, soil structure interaction problems, and geometrically complex thick-walled mechanical components such as pumps, valves, etc. These solid elements have only three degrees of freedom at each grid point (the three displacement components), and they may be combined with all other nonaxisymmetric NASTRAN elements.

The isoparametric solid elements were first presented by Irons, Ergatoudis and Zienkiewicz [Refs. 1 to 4]. Isoparametric solid elements employing either eight, twenty or thirty-two grid points have been found to be suitable to solve most problems (Figure 1). These elements correspond to assuming a linear, quadratic, and cubic variation of displacement, respectively. Clough [Ref. 5] conducted an evaluation of three-dimensional solid elements and showed that the isoparametric elements were superior to other solid elements. He further
pointed out that the choice of which isoparametric element is best to use depends on the type of problem being solved. For problems involving shear and bending type deformations, the higher order elements are preferred over the linear elements which should be used for problems in which shear stresses predominate. It is for this reason that all three isoparametric elements have been incorporated into NASTRAN.

The governing equations for isoparametric elements are based on minimum energy principles. The derivation of these equations assumes a displacement function over the element which depends on grid point displacements only. The governing equations are obtained by minimizing the Potential Energy which is evaluated in terms of these displacement functions.

## Jisplacement Functions

The name isoparametric is derived from the fact that the interpolating or shape functions used to represent the deformation of the element are also used to represent the geometry of the element. This choice insures that the element displacement functions satisfy the criteria necessary for convergence of the finite element analysis [Ref. 4]. Referring to the curvilinear coordinates ( $\xi, \eta, \zeta$ ) shown in Figure 1 , the rectangular basic $x, y, z$ coordinates at any point in the ement are obtained from the NASTRAN basic coordinates at each of the n grid points by:

$$
\left\{\begin{array}{l}
x  \tag{1}\\
y \\
z
\end{array}\right\}=\sum_{i=1}^{n} N_{i}(\xi, \eta, \zeta)\left\{\left.\left._{j}^{x}\right|_{z} ^{x}\right|_{i}\right.
$$

where the $N_{1}(\xi, \eta, \zeta)$ are shape functions which depend on the number of grid points used to define the element geometry. The $N_{i}$ functions are either linear quadratic, or cubic, and correspond to employing two, three, or four grid points respectively, along each edge of the element. This choice insures that there are no geometric gaps between grid points. Expressions for the shape functions may be found in Reference 6.

The deformation of the element is represented with the identical interpolating functions used to define the geo: cry; that is:

$$
\left\{\begin{array}{l}
u  \tag{2}\\
v \\
w
\end{array}\right\}=\sum_{i=1}^{n} N_{i}(\xi, n, \zeta)\left\{\left._{v}^{u}\right|_{i} ^{u}\right.
$$

where $u, v$ and $w$ are displacements along the $x, y$ and $z$ basic coordinate axes. The displacement functions $N_{i}(\xi, \eta, \zeta)$ satisfy the required convergence criterion of adequately representing a constant strain state, and insure interelement compatibility along the complete element boundary [Ref. 4].

## Strain-Displacement Reiations

Equation (2) may be used in the well-known strain-displacement relations for a three-dimensional continuum [Ref. 7] to define the strain vector $\{\varepsilon\}$ in terms of the grid point displacements:

In order to evaluate the strain matrix [C], the derivatives of the shape functions $N_{i}$ with respect to $x, y$, and $z$ must be calculated. Since $N_{i}$ is defined in terms of $\xi, n$ and $\zeta$, it is necessary to use the relation that

$$
\left\{\begin{array}{l}
\partial N_{1}  \tag{5}\\
\frac{\partial x}{\partial N_{1}} \\
\frac{\partial y}{\partial N_{1}} \\
\frac{\partial z}{\partial z}
\end{array}\right\}=[J]^{-1}\left\{\begin{array}{l}
\partial N_{1} \\
\partial \xi \\
\partial N_{1} \\
\frac{\partial n}{\partial N_{1}} \\
\frac{\partial N_{1}}{\partial \zeta}
\end{array}\right\}
$$

where [J] is the Jacobian matrix. It is easily evaluated by noting that

$$
[J]=\left[\begin{array}{ccc}
\frac{\partial x}{\partial \xi} & \frac{\partial y}{\partial \xi} & \frac{\partial z}{\partial \xi}  \tag{6}\\
\frac{\partial x}{\partial n} & \frac{\partial y}{\partial n} & \frac{\partial z}{\partial n} \\
\frac{\partial x}{\partial \zeta} & \frac{\partial y}{\partial \zeta} & \frac{\partial z}{\partial \zeta}
\end{array}\right]=\left[\begin{array}{llll}
\frac{\partial N_{1}}{\partial \xi}, & \frac{\partial N_{2}}{\partial \xi_{2}}, & \cdots & \frac{\partial N_{n}}{\partial \xi} \\
\frac{\partial N_{1}}{}, & \frac{\partial N_{2}}{\partial n}, & \frac{\partial N_{n}}{\partial n}, & \cdots \\
\frac{\partial n}{} \\
\frac{\partial N_{1}}{\partial \zeta}, & \frac{\partial N_{2}}{\partial \zeta}, & \cdots & \frac{\partial N_{n}}{\partial \zeta}
\end{array}\right]\left[\begin{array}{lll}
x_{1} & y_{1} & z_{1} \\
& & \\
x_{2} & y_{2} & z_{2} \\
\vdots & \vdots & \vdots \\
x_{n} & y_{n} & z_{n}
\end{array}\right]
$$

where the subscript; $i, 2, \ldots n$ denote the $n$ grid points of an element.

## Stress-Strain Relations

The stress-strain relations for a general elastic material are

$$
\begin{equation*}
\{\sigma\}=\left[G_{e}\right]\left\{\varepsilon-\varepsilon_{t}\right\} \tag{7}
\end{equation*}
$$

where $\{\sigma\}$ is the $6 \times 1$ stress vector in the basic coordinate system, $\left[G_{e}\right]$ is a $6 \times 6$ symmetric clastic material matrix, and $\left\{\varepsilon_{t}\right\}$ is the $6 x l$ thermal strain vector. This thermal strain vector is defined as

$$
\begin{equation*}
\left\{\varepsilon_{t}\right\}=\left\{\alpha_{e}\right\} \cdot \sum_{i=1}^{n} N_{i}(\xi, n, \zeta) T_{i} \tag{8}
\end{equation*}
$$

where $\left\{\alpha_{e}\right\}$ is a vector of 6 thermal expansion coefficients, and $T_{i}$ is the temperature at the $i^{\text {th }}$ element grid point.

## Stiffness, Mass, and Load Matrices

The stiffness, mass, and load matrices for the isoparametric element are derived by application of the Principle of Virtual Wori. These element matrices, relative to the basic coordinate system, are given by

$$
\begin{align*}
& {\left[K_{e e}\right]=\int_{-1}^{+1} \int_{-1}^{+1} \int_{-1}^{+1}[C]^{T}\left[G_{e}\right][C]|J| d \xi d n d \zeta}  \tag{9}\\
& {\left[M_{e e}\right]=\int_{-1}^{+1} \int_{-1}^{+1} \int_{-1}^{+1} \rho[N]^{T}[N]|J| d \xi d n d \zeta} \tag{10}
\end{align*}
$$

$$
\begin{align*}
& \left\{P_{e}^{t}\right\}=\int_{-1}^{+1} \int_{-1}^{+1} \int_{-1}^{+1}[C]^{T}\left[G_{e}\right]\left\{\alpha_{e}\right\}|J|\left(\sum_{i=1}^{n} N_{1} T_{i}\right) d \xi d \eta d \zeta  \tag{11}\\
& \left\{P_{e}^{P}\right\}=-\int_{-1}^{+1} \int_{-1}^{+1} P_{-\zeta}[N(\xi, \eta,-1)]^{T}|J|\left\{J_{\zeta}^{-1}\right\} d \xi d \eta \\
& -\int_{-1}^{+1} \int_{-i}^{+1} P_{-n}[N(\xi,-1, \zeta)]^{T}|J|\left\{J_{n}^{-1}\right\} d \xi d \zeta \\
& +\int_{-1}^{+1} \int_{-1}^{+1} P_{+\zeta}[N(\xi, n,+1)]^{T}|J|\left\{J_{\zeta}^{-1}\right\} d \xi d \eta \tag{12}
\end{align*}
$$

where [ $K_{e e}$ ] is the element stiffness matrix in the basic coordinate system, [ $M_{e e}$ ] is the mass matrix, $\left\{P_{e}{ }^{t}\right\}$ is the thermal load vector, and $\left\{P_{e}{ }^{P}\right\}$ is the pressure load vector derived from surface pressures on each of the six faces of the solid element. $|J|$ is the determinant of the Jacobian matrix, and [ $N$ ] is a matrix of the isoparametric shape function defined by

$$
[N]=\left[\begin{array}{lll:lll:l}
N_{1} & 0 & 0 & N_{2} & 0 & 0 &  \tag{13}\\
0 & N_{1} & 0 & 0 & N_{2} & 0 & \cdots \\
0 & 0 & N_{1} & 0 & 0 & N_{2} & N_{n} \\
0 & 0 \\
0 & N_{n} & 0 \\
0 & 0 & N_{n}
\end{array}\right]
$$

$P_{-\zeta}$ is the uniform normal pressure (positive outward) applied to the face of the element where $\zeta=-1 ; \quad P_{-\eta}$ is the pressure applied to the face where $\eta=-1$, etc.; and $\left\{J_{\xi}^{-1}\right\},\left\{J_{\eta}^{-1}\right\}$, and $\left\{J_{\zeta}^{-1}\right\}$ are the first, second, and third columss, respectively, of the inverse of the Jacobian matrix. Products like $|J|\left\{J_{\xi}^{-1}\right\}$ in the expression for pressure load are equivalent to a vector of direction cosines multiplied by a surface area scaling factor relating the curviinear coordinates to the basic coordinate system.

The integrals in equations (9) to (12) are evaluated numerically by using the method of Gaussian Quadrature [Ref. 8]. In the above equations, therefore, [C], $|J|$, and [ $N$ ] must be evaluated at each interior point used for numerical integration. $\left[G_{e}\right],\left\{\alpha_{e}\right\}$, and $p$ can also be evaluated at each integration point. Thus, variations in these quantities are allowed because of, say, temperaturedependent m?terial.

The computations for the iscparametric elements are carried out in the basic coordinate system. If the global coordinate system at any grid point is different from the basic system, the final matrices and vectors are transformed into that global system.

## Stress Recovery

The equation for calculating element stresses at any interior point of an isoparametric element may be obtained by combining equations (3), (7), and (8) a follows:

$$
\begin{equation*}
\{\sigma\}=\left[G_{e}\right]\left([C]\left\{u_{e}\right\}-\left\{\alpha_{e}\right\} \cdot\left(\sum_{i=1}^{n} N_{1} T_{i}\right)\right) \tag{14}
\end{equation*}
$$

[C] and $N_{1}$ are functions of the element curvilinear coordinates $\xi, \eta, \zeta$ evaluated at the point at which stresses are desired.

## Differential Stiffness Matrix

The differential stiffness matrix for the isoparametric solid element is derived by adding the energy of an inital stress state to the potential energy function. This additional energy is derived in Reference 9 and is given by

$$
\begin{align*}
& W_{p}=\frac{1}{2} \int_{v}\left[\omega_{x}^{2}\left(\sigma_{y}+\sigma_{z}\right)+\omega_{y}^{2}\left(\sigma_{x}+\sigma_{z}\right)+\omega_{z}^{2}\left(\sigma_{x}+\sigma_{y}\right)\right. \\
& \left.-2 \omega_{x} \omega_{y} \tau_{x y}-2 \omega_{y} \omega_{z} \tau_{y z}-2 \omega_{z} \omega_{x} \tau_{z x}\right] d V \tag{15}
\end{align*}
$$

where the rotations are given by the relations

$$
\begin{align*}
& \omega_{x}=\frac{1}{2}\left(\frac{\partial w}{\partial y}-\frac{\partial v}{\partial z}\right) \\
& \omega_{y}=\frac{1}{2}\left(\frac{\partial u}{\partial z}-\frac{\partial w}{\partial x}\right)  \tag{16}\\
& \omega_{z}=\frac{1}{2}\left(\frac{\partial v}{\partial x}-\frac{\partial u}{\partial y}\right)
\end{align*}
$$

These rotations may be expressed in terms of the grid point displacements by using equation (1):

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$$
\left[\overline{\mathrm{C}}_{1}\right]=\frac{1}{2}\left[\begin{array}{ccc}
0 & -\frac{\partial N_{1}}{\partial z} & \frac{\partial N_{1}}{\partial y}  \tag{18}\\
\frac{\partial N_{1}}{\partial z} & 0 & -\frac{\partial N_{i}}{\partial x} \\
-\frac{\partial N_{i}}{\partial y} & \frac{\partial N_{1}}{\partial x} & 0
\end{array}\right]
$$

Substituting equation (17) into equation (15) and adding this function to the otential energy expression yields the differential stiffness matrix:

$$
\left.\mathrm{k}_{\mathrm{ee}}^{\mathrm{d}}\right]=\int_{-1}^{+1} \int_{-1}^{+1} \int_{-1}^{+1}[\overline{\mathrm{C}}]^{T}\left[\begin{array}{ccc}
\sigma_{y}+\sigma_{z} & -\tau_{x y} & -\tau_{z x}  \tag{19}\\
-\tau_{x y} & \sigma_{x}+\sigma_{z} & -\tau_{z y} \\
& {[\overline{\mathrm{C}}]|\mathrm{J}| \mathrm{d} \xi \mathrm{~d} \mid \mathrm{d} \zeta, ~}
\end{array}\right]
$$

As with the structural stiffness matrix, this integral is evaluated using the nethod of Gaussian Quadrature. The differential stiffness is computed in the basic coordinate system and then transformed, as required, to the NASTRAN blobal system.

## IMPLEMENTATION

Many existing NASTRAN functional modules and subroutines were modified to mplement the isoparametric snlid elements. Several new subroutines were also added. These modules and brief descriptions of the changes tu each are listed In Table 1. The detailed description of these changes presented in Reference 10 tan be used to augment the NASTRAN Programer's Manual instructions, Section 6.8, fo assist in the installation of other new elements of similar complexity. Many of the changes are those normally required when implementing new elements. However, in this case, changes were also required in the PLDT module (to plot three-dirensional elements), the GP3 module (to process a new external pressure load). ad in various other modules to accommodate the large space requirements of the 32-grid-point cubic element.

It should be noted that the isoparametric elements were installed in functional modules SMA1, SMA2, and DSMG1 on an interim basis only. The element natrix subroutines were designed specifically for the new Element Matrix Generator module and will be made available with Level 16 of NASTRAN.

## EVALUATION

A slender cantilever beam model was chosen to evaluate the performance of the three new isoparametric solid elements in NASTRAN. This model was chosen for two reasons: (1) theoretical solutions are well known, and (2) solid finite
elements characteristically do not perform well when used to model structures which exhibit predominant bending behavior.

Three models were prepared as shown in Figures 2. 3, and 4, one with each of the three elements: IHEX1, IHEX2, and IHEX3, the linear, quadratic, and cubic elements, respectively. All three beam models had a length (L) of 3.66 m (144 in.) and a uniform rectangular cross section with depth (D) of 0.61 m ( 24 in. ) and width ( $W$ ) of 0.30 m ( 12 in. ). The same uniform material properties shown in Table 2 were used for all three models. Static, normal modes, and buckling analyses were performed for each of the beam models.

## Statics

For the static analyses, all deg ees of freedom at the base of the beam ( $2=0$ ) were completely fixed. All three models were subjected to the same four loading conditions:

$$
\begin{aligned}
& \text { 1. Linear thermal gradient (Y-direction) } \\
& T=322.04 K \text { at } Y=0\left(120^{\circ} F \text { at } Y=0\right) \\
& T=188.71 K \text { at } Y=0.6 l_{m}\left(-120^{\circ} F \text { at } Y=24 \mathrm{in} .\right)
\end{aligned}
$$

2. Uniform temperature rise $\Delta T=55.56 \mathrm{~K}\left(100^{\circ} \mathrm{F}\right)$
3. Compressive axial pressure (Z-direction)

$$
P_{Z}=-2.954 \times 10^{8} \mathrm{~N} / \mathrm{m}^{2} \text { at } Z=3.66 \mathrm{~m}(-42837 \mathrm{psi} \text { at } Z=144 \mathrm{in.})
$$

4. Transverse pressure ( $Y$-direction)

$$
\mathrm{P}_{\mathrm{Y}}=6.895 \times 10^{5} \mathrm{~N} / \mathrm{m}^{2} \text { at } \mathrm{Y}=0(100 \mathrm{psi} \text { at } Y=0)
$$

The results for the tip displacements are sumarized in Table 3, where the computed solutions are compared with the theoretical solutions. The maximum error for the linear IHEXI element was $10.3 \%$ for the transverse pressure load. For the quadratic and cubic elements, IHEX2 and IHEX3, the maximum errors of $4.5 \%$ and $3.5 \%$, respectively, occurred in the solutions for the themal gradient load. For the transverse pressure load, the errors were $1.6 \%$ for the IHEX2 element model, and $1.1 \%$ for IHEX3. Thus, the higher order isoparametric sclid elements perform very well when used to model the bending behavior of this beam.

## Normal Modes

In the normal mode analyses, the same single point constraints were applied to all three models in the following manner: All 2 components of displacement in the plane $2=0$ and all $Y$ components along the liue $2=0$, $Y=0.30 \mathrm{~m}(12 \mathrm{in}$.$) , were fixed. For the IHEXI and IHEX3 models only, all X$ components along the line $Z=0, X=0$, were fixed. For the IHEX2 model cnly, the $X$ components along the line $Z=0, X=-0.15 m(6 \mathrm{in}$.$) , were fixed. This$ system of constraints was chosen to allow dilatation at the base of the beam. The particular set of constraints used for the IHEX2 model has the additional advantage of symatry.

The inverse power method was used to extract the first three normal modes of each model. The results for the natural frequencies are summarized in Table 4. The computed natural frequencles for the IHEX2 and IHEX3 $\pi$ iels are within 3.0 per cent of the theoretical solution. The natural frequency for the IHEXI model is $2.7 \%$ off for bending in the Y-direction, but it is off by $18.3 \%$ and $15.8 \%$ for the two bending modes about the X -dircction. These er: s are probably caused by an insufficient number of elements through the width of the beam in the $X$-direction. Using a smaller mesh size with more IHEXI elements would improve these results at the expense of increased computer costs. This problein, therefore, serves to demonstrate even more clearly the superiority of the IHEX2 and IHEX3 elements over the THEZi element for modeling the bending behavior of structures.

All the computed mode shapes for ali three rodels showed excellent correlation with the theoretical solution [Ref. 11]. Comparative plots of the mode shapes are not included in this paper because there would be no visible distinction between computed and theoretical solutions.

## Bucking

Each of the three beam models was used to compute the critical buckling load for axial pressure. The same system of constraints used to compute normal modes was used to compute the axial pressure buckling load. The applied pressure on the end of the beam was $-2.954 \times 10^{\circ} \mathrm{N} / \mathrm{m}^{2}(42,837 \mathrm{psi})$. This amounts to a tetal applied force of $-5.406 \times 10^{7} \mathrm{~N}\left(-1.234 \times 10^{7} 1 \mathrm{~b}\right)$, which is equal to the theoretical critical load for buckling in the $X$-direction. Therefore, the fundamental eigenvalue for buckling should have been unity.

Again, the inverse power method was used to extract the three lowest buckling modes. The resuits for the buckling eigenvalues $\lambda$ are fresented in Table 5. The IHEX2 and IHEX3 elemant results are excellent. They are within $0.7 \%$ of the theoretical solution. The eigenvalue for the IAEXI element modri is in error by leis than $10 \%$ for buckling in the $Y$-direction. Rowever, it is off by more than $40 \%$ for both buckling modes in the X-direction. This situation is siminr to that of the normal mode problem for the IHEX: element model. Again, it is probably due to the lack of an adequate number of elements through the width of the beam in the $x$-direction.
as was the case for thi: normal modes problem, the mode shapes computed by NASTRAN for buckling were very close to the theoretical shapes. Thus, no plots comparing computed shapes with theoretical shapes are included in this paper.

## concluding kmarks

All three isoparametric solid elements produced good results for static, nurmal mode, and buckling analyses. As expected, the linear element results showed that it is beat used when shear betavior predominates. The superiority of the quadratic and cubic elements was confirmed by the excellent resui is obtained in both the bending and the shear behavior of a cantilever beam wodel. Therefore, the implementation of these three isoparametric solid elemer's, which provide for variations in both material propertice and stressec throughout the element, does greatly enhance the total modeiing capability of NASTRAN.

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TABLE 1. FUNCTIONAL MODULE MODIFICATIONS TO IMPLEMENT ISOPARAMETRIC SOLID ELEMENTS

| IFP | - New Bulk Data cards were added |
| :---: | :---: |
| GP2 | - Array sizes were increased to accommodate elements with 32 grid points |
| PLTSET | - Array sizes were increased to accommodate elements with 32 grid points |
| PLDT | - Capability for plotting solid elements was implemented |
| GP3 | - Processing of the isoparametric element pressure card was implemented |
| TAl | - Capability to appenc grid point temperatures to EST/ECPT entries was implemented |
| SMAI | - Stiffness and conductance matrix generation for the new elements was inplemented |
| SMA2 | - Mass and capacitance matrix generation for the new elements was implemented |
| SSG1 | - Load vector generation for thermal and pressure loads on the new elements was implemented |
| DSMGI | - Differential stiffness matrix generation for the new elements was implemented |
| SDR2 | - Stress calculations for individual grid points of the new elements was implemented |
| ¢FP | - Stress printout formats for the new elements were implemented |

table 2. material properties of the cantilever beam models

| Symbol | Description | Value (SI) | Value (English) |
| :--- | :--- | :--- | :--- |
| E | Young's modulus | $2.068 \times 10^{11} \mathrm{~N} / \mathrm{m}^{2}$ | $30 \times 10^{6} \mathrm{lb} / \mathrm{in}^{2}$ |
| $\nu$ | Poisson's ratio | 0.3 | 0.3 |
| $\alpha$ | Coef. of thermal expansion | $2.570 \times 10^{-5} \frac{\mathrm{~m}}{\mathrm{~m}-\sigma^{\circ} \mathrm{K}}$ | $1.428 \times 10^{-5} \frac{\mathrm{in}}{\mathrm{in}-{ }^{\circ} \mathrm{F}}$ |
| $\rho$ | Mass density | $20.86 \mathrm{~kg} / \mathrm{m}^{3}$ | $7.535 \times 10^{-4} \mathrm{lb} / \mathrm{in}^{3}$ |

table 3. COMPARISON OF TIP DEFLECTIONS FOR NASTRAN AND THEORETICAL SOLUTIONS FOR FOUR STATIC LOADING CONDITIONS

| Load Case | Description | NASTRAN Solutions |  |  |  |  |  | Theoretical Solution* Tip Defl., cm |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | IHEX1 Model |  | IHEX2 Mode1 |  | IHEX3 Model |  |  |
|  |  | $\underset{\mathrm{cm}}{\text { Def1., }}$ | $\begin{gathered} \text { Error } \\ \% \end{gathered}$ | $\begin{gathered} \text { Defl., } \\ \mathrm{cm} \end{gathered}$ | $\begin{gathered} \text { Erroz } \\ \% \end{gathered}$ | Defl., <br> cm | $\begin{gathered} \text { Error, } \\ \% \end{gathered}$ |  |
| 1 | Thermal Gradient | 3.668 | 2.5 | 3.932 | 4.5 | 3.894 | 3.5 | 3.762 |
| 2 | Uniform Temperature | . 5367 | 2.8 | . 5344 | 2.3 | . 5304 | 1.6 | . 5222 |
| 3 | Axial Compression | -. 5179 | 0.8 | -. 5187 | 0.7 | -. 5199 | 0.4 | -. 5222 |
| 4 | Transverse Pressure | . 3612 | 10.3 | . 3965 | 1.6 | . 3985 | 1.1 | . 4028 |
| *Theoretical Solutions |  |  |  |  |  |  |  |  |
| Load Case 1 Load C |  | ase 2 | Load Case 3 |  |  | Load Case 4 |  |  |
| $\delta_{Y}$ | $\frac{a \Delta T L^{2}}{2 D} \quad \delta_{\%}=0$ | $\delta_{z}=a \Delta T L$ | $\delta_{Z}=\frac{P_{Z} L}{E}$ |  |  | $\delta_{Y}=\frac{3 P_{Y} L^{4}}{2 E D^{3}}\left[1+\frac{4 D^{2}}{5 L^{2}}\right]$ |  |  |
|  | $3.762 \mathrm{~cm} \quad \delta_{Y}=.5$ | $\delta_{Y}=.5222 \mathrm{~cm}$ | $\delta_{Y}=-.5222 \mathrm{~cm}$ |  |  | $\delta_{Y}=.4028 \mathrm{~cm}$ |  |  |

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TABLE 4. COMPARISON OF NATURAL FREQUENCIES FOR NASTRAN AND THEORETICAL SOLUTIONS

| Mode | Description | NASTRAN Solutions |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | IHEXI Model |  | IHEX2 | model | IHEX3 | Model |  |
|  |  | Freq., cps | $\begin{gathered} \text { Error, } \\ \% \end{gathered}$ | Freq., cps | $\begin{gathered} \text { Error, } \\ \% \end{gathered}$ | Freq., cps | $\begin{gathered} \text { Error } \\ \% \end{gathered}$ |  |
| 1 | First Bending Mode in the X -Direction | 22.0 | 18.3 | 18.6 | 0 | 18.6 | 0 | 18.6 |
| 2 | First Be iding Mode in the $Y$-Direction | 38.3 | 2.7 | 36.5 | 2.1 | 36.5 | 2.1 | 37.3 |
| 3 | Second Rending Mode in the X -Direction | 135.3 | 15.8 | 114.3 | 2.1 | 113.3 | 3.0 | 116.8 |

TABLE 5. COMPARISON OF BUCKLING EIGENVALUES FOR NASTRAN AND THEORETICAL SOLUTIONS

| Mode | Description | NASTRAN Solutions |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | IHEX1 Model |  | IHEX2 Model |  | IHEX3 Model |  |  |
|  |  | $\lambda$ | $\begin{gathered} \text { Error, } \\ \% \end{gathered}$ | $\lambda$ | $\begin{array}{\|c\|} \hline \text { Error, } \\ \% \end{array}$ | $\lambda$ | $\underset{\%}{\text { Error, }}$ |  |
| 1 | X-Direction | 1.406 | 40.6 | 1.002 | . 2 | 1.001 | . 1 | 1.0 |
| 2 | Y-Direction | 4.391 | 9.8 | 3.981 | . 5 | 3.979 | . 5 | 4.0 |
| 3 | X-Direction | 12.809 | 42.3 | 9.037 | . 4 | 8.934 | . 7 | 9.0 |


(a) Linear.

(b) Quadratic.
(c) Cubic


FIGURE 1. THREE ISOPARAMETRIC ELEMENTS


FIGURE 2. IHEXI MODEL -- 216 ELEMENTS AND 364 GRID POINTS MATRIX ORDER $(\mathrm{g}-\mathrm{SET})=1092$, SEMI-BANDWIDTH $=102$.


FIGURE 3. IHEX2 MODEL -- 36 ELEMENTS AND 275 GRID POINTS MATRIX ORDER $(\mathrm{g}-\mathrm{SET})=825$, SEMI-BANDWIDTH $=156$.


FIGURE 4. IHEX3 MODEL -- 8 ELEMENTS AND 148 GRID POINTS MATRIX ORDER $(\mathrm{g}-\mathrm{SET})=444$, SEMI-BANDWIDTH $=132$

