Structural Health Monitoring Using High-Density Fiber Optic Strain Sensor and Inverse Finite Element Methods

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Abstract

In an effort to mitigate accidents due to system and component failure, NASA’s Aviation Safety has partnered with industry, academia, and other governmental organizations to develop real-time, on-board monitoring capabilities and system performance models for early detection of airframe structure degradation. NASA Langley is investigating a structural health monitoring capability that uses a distributed fiber optic strain system and an inverse finite element method for measuring and modeling structural deformations.

This report describes the constituent systems that enable this structural monitoring function and discusses results from laboratory tests using the fiber strain sensor system and the inverse finite element method to demonstrate structural deformation estimation on an instrumented test article.

Introduction

NASA Langley Research Center has developed instrumentation based upon principles of Optical Frequency-Domain Reflectometry (OFDR) for the provision of large-scale, densely distributed strain sensors using optical fiber embedded with Bragg gratings. Fiber Optic Bragg Grating technology enables the distribution of thousands of sensors that are immune to moisture and electromagnetic interference and have negligible weight penalty. The theory and development of this technology, called FOSS (Fiber Optic Strain System) is described in [1]. The algorithms and methods for deriving strain from an OFDR measurement system are described in [2].

At NASA Langley, this technology provides a key component for research and development relevant to comprehensive aerospace vehicle structural health monitoring (SHM). It has been lab tested in various applications. Notably, FOSS was used during structural testing of an advanced composite transport wing box [3]. An integrated prototype system has been developed that includes hardware and software for the acquisition of data from an optical network and conversion of the data into strain measurements.

To enable a comprehensive structural health capability, an inverse finite element method (iFEM) has been developed that uses the measured strain data to compute the full-field displacements and, subsequently, internal loads experienced by the structure in real time. The technique is applicable to thin and moderately thick beams, plates, shells, and built-up structures. The mathematical foundation for iFEM is described in [5].

This report briefly describes the FOSS system and the inverse finite element method and demonstrates the capability of the approach using a simple structural test. The structural test was conducted on an instrumented beam test article subjected to tip deflection. The strain measurements were used by the inverse finite element model to reconstruct the deformed shape of the beam. Details of the configuration of the beam test article and the experimental results are described. The report concludes with a discussion of issues and potential areas of future research.
Sensor System

The FOSS system is based upon an optical frequency-domain reflectometry (OFDR) technique that allows the reading of thousands of Bragg grating sensors distributed along a single fiber. The technique entails writing diffraction gratings with equivalent periods into a fiber which is part of a network that includes a broadband reflector that, when interrogated with a swept-wavelength laser source, modulates each sensor by a unique frequency. Each grating’s reflected wavelength will vary according to the physical stresses experienced by the sensor, and a series of specialized digital signal processes are then applied to resolve a micro-strain commensurate with the tension or compression of the fiber [2].

Inverse Finite Element Method

Efficient and robust computational mechanics structural analysis capabilities and tools are required to (1) optimize the selection of the type of sensors as well as the sensor locations, and to (2) process, in real time, raw sensor data from hundreds or thousands of multiple types of sensors, to obtain reliable knowledge of structural conditions. Such technologies, therefore, are critical to enabling an effective SHM system.

One current area of structural analysis being pursued is real-time reconstruction of full-field structural displacements and internal loads. This is seen as an enabling technology for structural health monitoring and actuation and control systems feedback for the next generation of aerospace vehicles. To facilitate such capabilities, load-carrying structural members need to be instrumented with a network of strain sensors, e.g., fiber optic Bragg-grating sensors.

Reconstruction of a displacement vector at every material point of the structure from a set of discrete strain measurements constitutes an inverse mathematical problem. Inverse problems are ill posed in the sense that they do not necessarily satisfy conditions of existence, uniqueness, and stability. For this class of mathematical problems (which use experimentally measured data known only approximately and containing random error), general methods for constructing approximate solutions that are stable under small changes in the measured data have been developed using the fundamental concept of a regularizing operator [4].

To address the modeling of complex aerospace structures densely instrumented with fiber optic strain sensors, Tessler and Spangler [5, 6] developed a novel variational principle that is well suited for this class of “reconstruction” inverse problems. The variational principle is used as a basis for formulating a computationally efficient inverse Finite Element Method (iFEM). The method computes the full-field displacements and internal loads using strain data from in-situ strain sensors. The formulation is based upon the minimization of a least squares functional that uses the complete set of strain measures corresponding to a first-order shear deformation theory. The error functional uses the least-squares-difference terms comprised of the strains that are expressed in terms of the displacements and the corresponding strains that are measured experimentally. All strain-displacement relations are enforced explicitly whereas the analytical and measured strains are matched in the least-squares sense. A penalty parameter controlled “regularization” term enforces physical constraints imposed on the transverse shear strains. By virtue of these assumptions, all strain compatibility relations are explicitly satisfied.

The methodology for reconstructing the displacements does not require elastic or inertial material properties. Thus, it is equally applicable for static and dynamic loadings, as well as a wide range of elastic materials. Once the displacements are reconstructed, the internal loads are readily computed using strain-displacement and material constitutive (stress-strain) relations.

A constant-strain inverse shell element, labeled iMIN3, was developed and implemented into NASA’s COMET-AR [7] finite element code. The element has three nodes and conventional shell-like six degrees-of-freedom at each node, i.e., three displacements and three
rotations. The displacement variables are interpolated using the lowest-order, C0-continuous anisoparametric shape functions adopted from a related Mindlin theory plate formulation [8].

**Test Article**

To validate the capability for predicting structural deformations using FOSS strain measurements, an aluminum beam test article was instrumented using both a FOSS fiber and thirteen foil gauges. The iFEM model estimated deflection based only on the FOSS strain values. The foil strain gauges were used strictly for independent validation of the FOSS measurements. The Instrumented Test Article (ITA) is an aluminum bar (7076 AL) which measures 1219.2 mm x 63 mm x 6 mm. The fiber and foils, mounted on the top surface, were positioned according to the diagram in Figure 1.

The FOSS fiber contains 333 Fiber Optics Bragg Gratings (FOBG) that measure axial strain along the length of the ITA. The single fiber provides for two rows of FOBG sensors by looping back near the last foil strain gauge (Figure 2). It is attached to the optical circuitry through a fusion splice that is protected by a 2 mm diameter cylindrical sleeve that is about 50 mm in length. The sleeve is depicted in Figure 3 and Figure 4.

![Figure 1: Dimensions and sensor layout for ITA.](image1.png)

![Figure 2: FOSS fiber looping back at the free end of ITA.](image2.png)

![Figure 3: Fiber bond site relative to mount screws and foil 1.](image3.png)

![Figure 4: Gratings number 330-332. Grating number 332 is the last grating.](image4.png)
Each grating is approximately 4 mm long having centerlines spaced approximately 6.35 mm along the fiber. The first grating, labeled grating 0, is located inside the fusion splice sleeve. The sensing end of the fiber is suspended 1 mm above the ITA surface as it exits the sleeve, and is bonded to the ITA surface approximately 20 mm before grating 9. Figure 3 shows the approximate location of grating 9 relative to the fusion sleeve and foil gauge 1. The inset in Figure 3 shows the suspended segment of the test fiber exiting the fusion sleeve at one end and bonded to the ITA surface on the other. The gratings in this suspended section were omitted from the test analysis since they were not bonded to the ITA surface.

Grating locations were determined using a thermal-based technique that proceeds by first locating 4 individual gratings (small segments of fiber were heated to induce localized strain), then calculating the location of remaining gratings through linear regression. Figure 4 shows the markings indicating approximate locations of the last three gratings in the fiber.

Thirteen single axis foil gauges were bonded along the centerline of the bar and oriented to measure strain along the length of the ITA. The centers of the active element in each foil are spaced 76.20 mm apart as illustrated in Figure 1. The first foil gauge is bonded at 146.05 mm from the fixed end of the bar as seen in Figure 3. Foil 13 is located near the loop in the FOSS fiber as depicted in Figure 2. The foils have a nominal resistance of 350 ohms at room temperature. The data acquisition system for the foil gauges was also calibrated at room temperature using a Fluke 702 Documenting Process Calibrator as a resistance source.

The ITA was mounted on an optical table in a cantilevered configuration as depicted in Figure 5. The fixed end of the ITA was attached using four posts as shown in Figure 6. The mount screws were not actually screwed into the posts, but rather were locked in place by the collar-screw located near the top of each post. Figure 7 shows the top view of the mounting assembly where the mount screws holding down the ITA are located. It also shows where the mount screws are located relative to the fiber and foil gauges.
ITA Measurements

Strain measurements were taken with the free end (tip) of ITA subjected to various deflection conditions. For each imposed deflection condition, strain values were acquired from both the FOSS system and the foil gauges. Data were recorded with the beam under its own weight (Figure 5), and also with the tip depressed (downward deflection, Figure 8), and raised (upward deflection, Figure 9) at various magnitudes. The imposed deflections were achieved by actuating a screw lever mounted on the optical table at the free end of the bar. The lever contacts the ITA at 9 mm inboard of the end of the ITA. The screw lever is shown in Figure 10.

Table 1: ITA test cases

<table>
<thead>
<tr>
<th>Load case</th>
<th>Direction of Loading</th>
<th>Imposed Deflection (W_o) (mm)</th>
</tr>
</thead>
<tbody>
<tr>
<td>7201</td>
<td>upward</td>
<td>59.22</td>
</tr>
<tr>
<td>7170</td>
<td>upward</td>
<td>46.52</td>
</tr>
<tr>
<td>7134</td>
<td>upward</td>
<td>33.82</td>
</tr>
<tr>
<td>7085</td>
<td>upward</td>
<td>21.12</td>
</tr>
<tr>
<td>7045</td>
<td>upward</td>
<td>8.42</td>
</tr>
<tr>
<td>6925</td>
<td>upward</td>
<td>-4.28</td>
</tr>
<tr>
<td>7401</td>
<td>own weight</td>
<td>-17.08</td>
</tr>
<tr>
<td>6783</td>
<td>own weight</td>
<td>-17.31</td>
</tr>
<tr>
<td>7441</td>
<td>downward</td>
<td>-23.33</td>
</tr>
<tr>
<td>7481</td>
<td>downward</td>
<td>-36.03</td>
</tr>
<tr>
<td>7525</td>
<td>downward</td>
<td>-48.73</td>
</tr>
<tr>
<td>7556</td>
<td>downward</td>
<td>-61.43</td>
</tr>
<tr>
<td>7588</td>
<td>downward</td>
<td>-74.13</td>
</tr>
</tbody>
</table>

Three load cases were selected from Table 1 for analysis. The cases are representative of the range of deflections induced on the ITA. Load case 7201 is the extreme upward case. 7401 was selected for the gravity induced effect. 7481 was selected for the downward case. For these cases, the FOSS surface strains were used as input to the inverse finite element method to compute deflections along the entire length of the ITA. Comparisons were made between the foil-gauge strains and those measured by the FOSS system. The following sections describe the analysis performed for the selected load cases.

Strain Validation and Verification

To ascertain the accuracy and correctness of the FOSS measured strains, The FOSS measured strains from the selected load cases were compared to the foil-gauge strains and strains obtained by analysis. Because the foil gauge data was acquired simultaneously with the FOSS
measurements, the foil sensors provide verification of FOSS strain measurements. To obtain analytical validation, the measured strains were compared with those computed using a finite element (FEM) model of the ITA as well as classical (Bernoulli-Euler) beam theory.

An ABAQUS FEM model of the ITA was developed using a three-node, triangular shell element. The mesh consists of 800 elements, 446 nodes and 2,676 degrees-of-freedom. Due to the complexity of the mounting apparatus, a detailed model of the mounting was not undertaken. Instead, the ITA was modeled as a cantilevered beam clamped at the location of the rightmost mounting screws, as depicted in Figure 6. This is the last attachment point before the cantilever. Figure 11 illustrates the top view of the ITA showing the alignment of the mesh start location with the rightmost set of screw holes corresponding to the cantilever attachment point. This model was used to compute the deflections corresponding to the loading conditions summarized in Table 1.

![Figure 11: FEM plate model of ITA.](image)

The ITA was also analyzed as a cantilever beam using classical beam theory. There are essentially two load conditions of interest in this experiment. One describes the effects of the beam under its own weight (distributed load). The other describes the effect of the beam under a point load.

For the beam under its own weight, the gravitational (uniform) load per unit length, \( q = \rho \cdot A \cdot g \), was computed as a product of the material density (\( \rho \)), cross sectional area (\( A \)), and gravitational acceleration (\( g \)). For this loading, the beam deflection, \( w \), assumed positive in the upward direction, is readily obtained by integrating the moment-curvature relations subject to the clamped boundary conditions at \( x = 0 \). The resulting deflection expression is given by:

\[
 w = -\frac{q x^2}{24EI} \left( x^2 - 4Lx + 6L^2 \right) \quad (1)
\]

where \( L \) is the cantilevered beam length; \( E \) is the modulus of elasticity; \( I = 2hr^3 / 3 \) is the moment of inertia (\( t \) is half of the beam thickness, and \( b \) is the beam width); and \( x \) denotes the beam’s axial coordinate whose origin is at the clamped end. The corresponding axial strains on the top (+) and bottom (-) beam surfaces, \( \epsilon_x^\pm \), are obtained as

\[
 \epsilon_x^\pm = \pm \frac{qt}{2EI} (L-x)^2 \quad (2)
\]

For a beam under a vertical concentrated force, \( P_a \), applied by the screw lever near the tip at \( x = a \), where \( P_a \) is assumed positive in the downward direction, the deflection expressions have the form:

\[
 w = \begin{cases} 
 \frac{P_a}{6EI} x^2 (x - 3a) & 0 \leq x \leq a \\
 \frac{P_a}{6EI} a^2 (a - 3x) & a < x \leq L 
\end{cases} \quad (3)
\]

The axial strains corresponding to this loading have the form:

\[
 \epsilon_x^\pm = \begin{cases} 
 \pm \frac{tP_a}{EI} (a - x) & 0 \leq x \leq a \\
 0 & a < x \leq L 
\end{cases} \quad (4)
\]

For load case 7401, the beam theory deflections and strains were computed using Equation (1) and (2), respectively.

For load cases 7481 and 7201, the above solutions for the distributed and concentrated loads were superposed to account for the total loading. The resulting strain and deflection for these cases are given by:

\[
 w = -\frac{x^2}{24EI} \left[ g \left( x^2 - 4xL + 6L^2 \right) + 4P_a (3a-x) \right] \quad (5)
\]
\[ \varepsilon_x^+ = \pm \frac{t}{2EI} \left[ q(L-x)^2 + 2aP_a \left( \frac{x}{a} - 1 \right) \right] \] (6)

In these cases, the concentrated load \( P_a \) is expressed in terms of the prescribed deflection, \( W_a \), imposed by the screw lever at \( x = a \), and the uniform loading \( q \) as:

\[ P_a = -\frac{3EIW_a}{a^3} - \frac{q}{8a} (a^2 - 4aL + 6L^2) \] (7)

Note that \( W_a \) is assumed positive in the upward direction; refer to Table 1 containing the loading cases and \( W_a \) measurements.

Table 2 and Table 3 summarize the pertinent dimensions and material parameters.

### Table 2: ITA dimensional parameters

<table>
<thead>
<tr>
<th>Parameter</th>
<th>Value</th>
</tr>
</thead>
<tbody>
<tr>
<td>Total bar length, mm</td>
<td>1219.20</td>
</tr>
<tr>
<td>Width of beam (b, mm)</td>
<td>63.50</td>
</tr>
<tr>
<td>Thickness of beam (2t, mm)</td>
<td>6.00</td>
</tr>
<tr>
<td>Moment of inertia (I, mm^4)</td>
<td>1143.00</td>
</tr>
<tr>
<td>Offset to cantilever, mm</td>
<td>222.25</td>
</tr>
<tr>
<td>Offset, tip to lever contact, mm</td>
<td>9.00</td>
</tr>
<tr>
<td>Test section length (L, mm)</td>
<td>996.95</td>
</tr>
<tr>
<td>Distance from cantilever to lever contact (a, mm)</td>
<td>987.95</td>
</tr>
</tbody>
</table>

### Table 3: ITA material parameters (7076 AL)

<table>
<thead>
<tr>
<th>Parameter</th>
<th>Value</th>
</tr>
</thead>
<tbody>
<tr>
<td>Elastic modulus ( (E, \text{N/mm}^2) )</td>
<td>6.700E+04</td>
</tr>
<tr>
<td>Density ( (\rho, \text{N s}^2/\text{mm}^4) )</td>
<td>2.84E-06</td>
</tr>
<tr>
<td>Poisson’s ratio</td>
<td>0.34</td>
</tr>
<tr>
<td>Uniform load due to gravity per unit length ( (q, \text{N/mm}) )</td>
<td>10.6112</td>
</tr>
</tbody>
</table>

In Figure 12, the top-surface axial strains measured experimentally by the FOSS and foil sensors for the beam under its own weight (load case 7401) are compared against the corresponding strains computed using beam theory and the FEM model. The beam theory strains were computed using Equation (2). Note that both the FEM and beam theory results are in close agreement, and predict higher strains than the FOSS and foil data. Additionally, both the FOSS and foil data show compressive strains at the free end of the beam. This effect is not present in the FEM and beam theory predictions.

The potential causes for the discrepancy between the experimental and analytical strains can be related to the heating and cooling of the laboratory and the strain condition of the ITA while the baseline was taken. A regression analysis performed on the foil and fiber strain data showed that the top surface at the beam’s tip was undergoing a small compressive strain (-20 μstrain) under gravitational loading only, which is unphysical.

![Figure 12: Gravitational load (load case 7401): Comparison of FOSS and foil axial strain measurements to beam theory and FEM results.](image)

To account for the discrepancy between experimental and analytical strains, the experimental strains were shifted +20 μstrain, to satisfy, in the average sense, the physically required zero-strain condition at the beam’s tip. Figure 13 depicts the strain data of Figure 12 once the +20 μstrain adjustment to the FOSS and foil strains was introduced.
The strain comparisons for two other loading conditions, corresponding to the imposed $W_d$ deflection downward (load case 7481) and upward (load case 7201), are depicted in Figure 14 and Figure 15. Note that the FOSS and foil data in these cases also have the $+20 \mu$ strain adjustment consistent with the gravitational load case discussed previously. The beam theory strains were computed using Equation (6) containing the contribution of both the concentrated and gravitational loads.

**iFEM Comparison**

After the validation of the FOSS strain measurements, these strains were input into the inverse FEM (iFEM) algorithm to predict the deflections. The iFEM deflections were then compared to the measured deflections as well as those of the FEM analysis and classical beam theory. The resulting deflections are plotted in Figure 17, Figure 18, and Figure 19, below and shown in Table 4 and Table 5.

The iFEM model of the ITA employed a three-node, triangular inverse shell element implemented into a user-defined routine in the ABAQUS finite element code. The iFEM model consists of nine such elements, having 14 nodes and 66 degrees-of-freedom. Like the FEM model, only the portion of the ITA beyond the clamped edge is included in the iFEM mesh. Figure 16 depicts the iFEM mesh overlaid on the top view of the ITA.

Note that the iFEM mesh is of much lower fidelity than the FEM mesh, with the latter consisting of 800 elements, corresponding to 446 nodes and 2,676 degrees-of-freedom. Although higher-fidelity iFEM meshes were also examined, no appreciable response differences were observed. Hence, the nine-element mesh was
selected because of its simplicity and superior computational efficiency. The practical implication is that, without sacrificing accuracy, ultra-fast runtimes can be achieved – a necessary condition for real-time applications.

The computed deflections are shown in Figure 17, Figure 18, and Figure 19, for the load cases 7401 (gravity load), 7481 (downward point load), and 7201 (upward point load), respectively. In all three loading cases, the FEM results agree closely with those of beam theory. Both the beam theory computations and FEM results are slightly higher than the iFEM and experimental values. Note that in this test the deflection along the ITA was measured at only five locations. Although the measurements are somewhat sparse, they give an adequate sense of the deflection shape that compares well with the analytic predictions.

The screw lever gave the most accurate displacement measurement available. Table 4 gives the value at the screw lever contact point for the measured versus the analytical deflections. Table 5 gives the difference in those deflections. As seen in Table 5, the iFEM predicted deflections are within 5% of the experimental values. This correspondence between the measured and analytical deflections is consistent with the experimental measurement errors involved, and the simplified boundary conditions employed in the analysis.

Table 4: Deflection at screw lever.

<table>
<thead>
<tr>
<th>Load Case</th>
<th>Exper. (mm)</th>
<th>iFEM (mm)</th>
<th>FEM (mm)</th>
<th>Beam Theory (mm)</th>
</tr>
</thead>
<tbody>
<tr>
<td>7401</td>
<td>-17.1</td>
<td>-16.308</td>
<td>-16.920</td>
<td>-16.960</td>
</tr>
<tr>
<td>7481</td>
<td>-36.0</td>
<td>-34.492</td>
<td>-36.130</td>
<td>-36.184</td>
</tr>
<tr>
<td>7201</td>
<td>59.2</td>
<td>57.490</td>
<td>59.960</td>
<td>59.493</td>
</tr>
</tbody>
</table>

Table 5: Percentage difference between experimental and analytical deflections at screw lever.

<table>
<thead>
<tr>
<th>Load Case</th>
<th>iFEM (mm)</th>
<th>FEM (mm)</th>
<th>Beam Theory</th>
</tr>
</thead>
<tbody>
<tr>
<td></td>
<td>%</td>
<td>%</td>
<td></td>
</tr>
<tr>
<td>7401</td>
<td>-0.79</td>
<td>-0.18</td>
<td>-0.14</td>
</tr>
<tr>
<td>7481</td>
<td>-1.54</td>
<td>0.10</td>
<td>0.15</td>
</tr>
<tr>
<td>7201</td>
<td>1.73</td>
<td>-0.74</td>
<td>-0.27</td>
</tr>
</tbody>
</table>

Figure 17: Displacement comparison for Load Case 7401

Figure 18: Displacement comparison for Load Case 7481

Figure 19: Displacement comparison for Load Case 7201.
Summary

Both FOSS and iFEM technologies were applied to reconstruct the full-field structural deformations in an aluminum beam instrumented in a structures laboratory. The beam test article, clamped at one end and subjected to an imposed near-tip deflection at the other, was examined. The FOSS capability enabled sufficiently accurate, though somewhat noisy, axial strains to be measured. These strain measurements were subsequently used in an iFEM model, resulting in an accurate reconstruction of the deformed shape of the beam. Although not discussed in the report, the reconstructed deformed shape can be readily interrogated to produce continuous strains and stresses defined everywhere in the beam. This type of structural response information, when computed in real time, may constitute an important database to monitor structural conditions of load bearing structures in civil, automotive, and aerospace applications.

The beam deflection results obtained with the iFEM models, utilizing experimentally measured strains, were found to be in excellent agreement with the experimental measurements, the classical beam theory, and predictions achieved by the forward FEM. A particularly appealing aspect of the iFEM technology is that relatively simple meshes, computed very fast, are required to achieve results of high accuracy. This is contrasted with the forward FEM models that often require a significantly higher mesh density, and thus greater number of degrees-of-freedom, to obtain comparable results.

Naturally, under real-time structural monitoring conditions, the knowledge of external loads is almost never available, and hence the application of the forward FEM is generally not feasible. Conversely, the strains measured by a comprehensive FOSS network and the numerical processing of these strains with the computationally efficient iFEM technology have the potential for reconstructing the important structural conditions including displacements, strains, stresses, and strain-stress based failure criteria. It may also be possible, with the use of the FOSS-iFEM technology, to enable reconstruction of the external loads acting on the structure.

References


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**Abstract:**
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